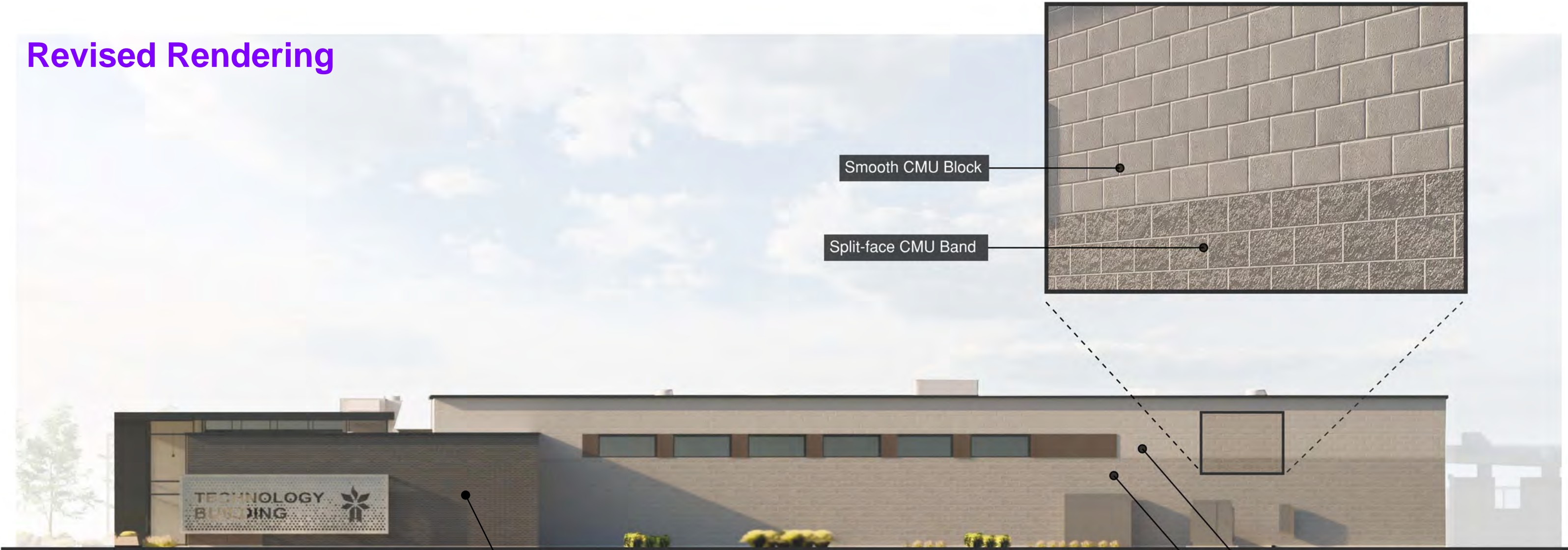


DAVIS TECHNICAL COLLEGE - WELDING TECHNOLOGY BUILDING MASONRY BID PACKAGE

Revised Rendering



Smooth CMU Block

Split-face CMU Band

4x4x16 Emperor Brick
Interstate Brick - Black Midnight
Or approved Equal

8" Smooth Face CMU
Sunroc - Cream

8" Split Face CMU
Sunroc - Tumbleweed

Or approved Equal

DAVIS TECHNICAL COLLEGE WELDING TECHNOLOGY BUILDING

355 SOUTH 650 EAST, KAYSVILLE, UT 84037

CRSA

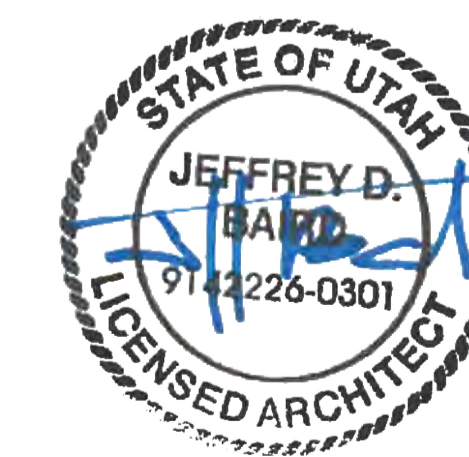
PROJECT **24-038**

BID PACKAGE #1 2024-08-26

REVISIONS
NO. DATE DESCRIPTION



**DAVIS TECHNICAL COLLEGE
WELDING TECHNOLOGY BUILDING**
355 SOUTH 650 EAST
KAYSVILLE, UT 84037



COVER SHEET
GI000.1



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GRAMOLL CONSTRUCTION
KEN ROMNEY
801-295-2341
KEN.ROMNEY@GRAMOLL.COM



ARCHITECT
CRSA
KATHY WHEADON
801-355-5915
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Consulting Structural Engineers

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NARRATIVE

BID PACKAGE #1

SITE & STRUCTURE - Major structural and site elements.

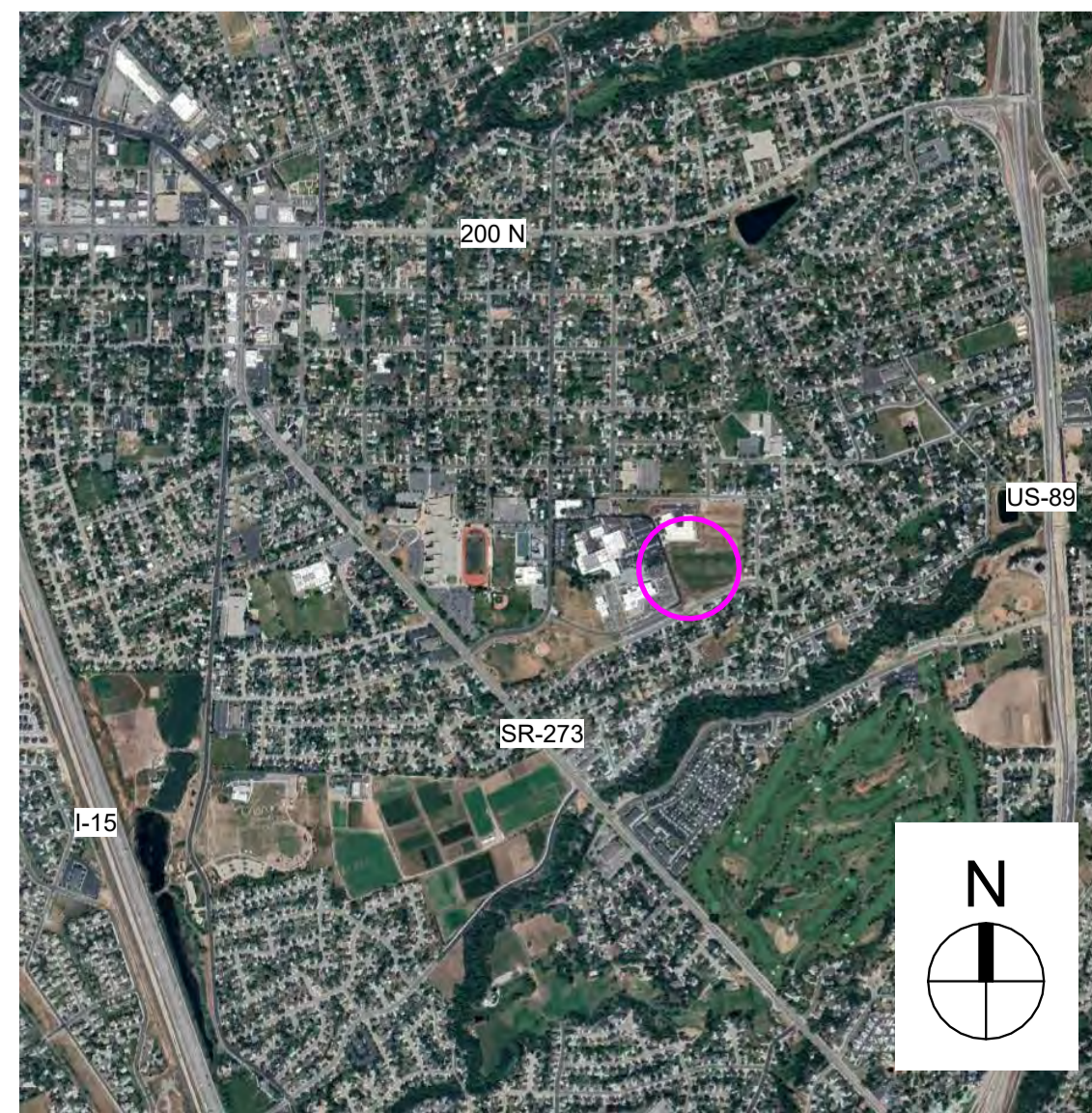
BID PACKAGE #2

CORE & SHELL - Exterior envelope and major MEP systems.

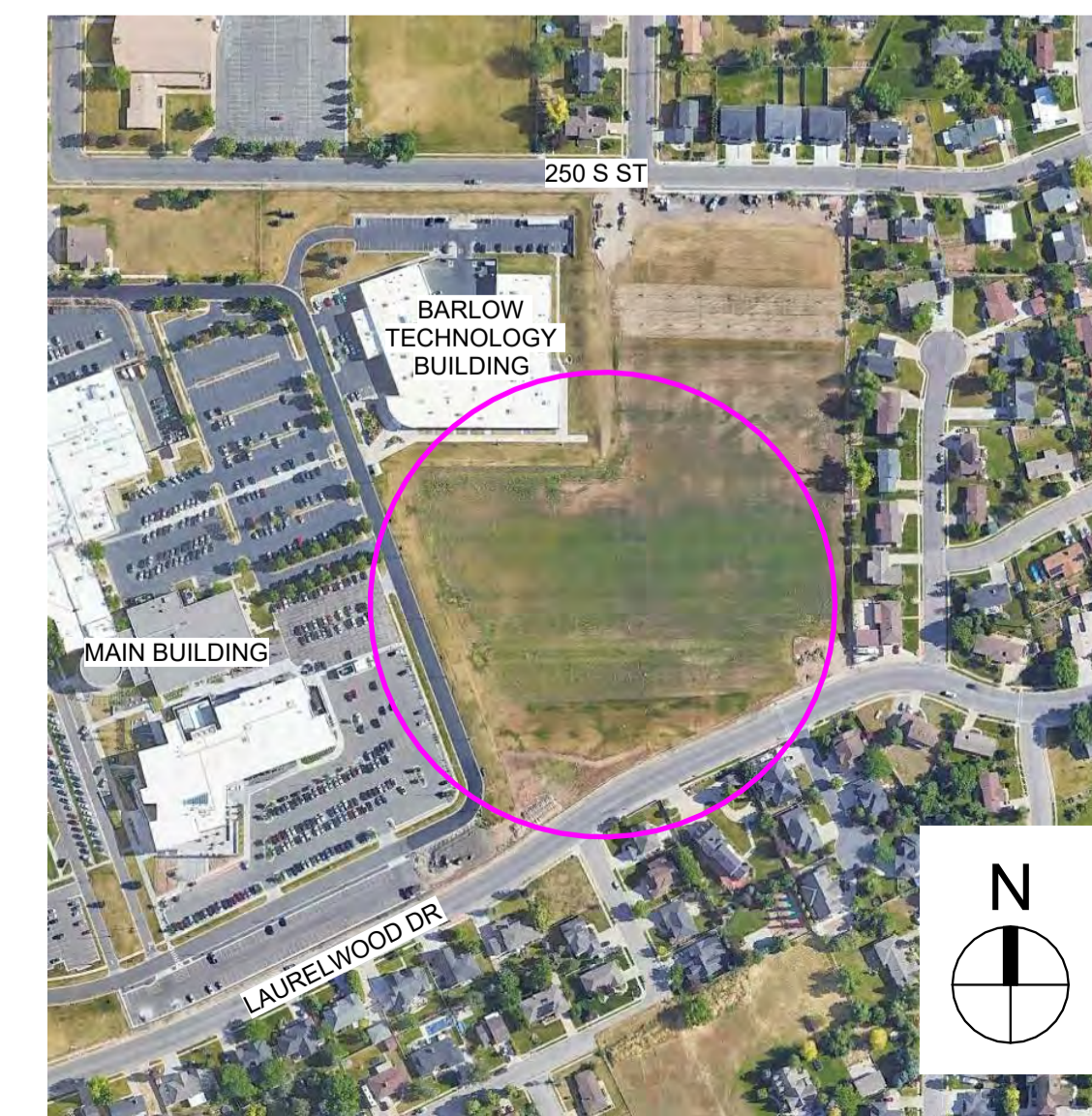
BID PACKAGE #3

INTERIOR FINISHES - Interior elements, including finishes and millwork.

LOCATION MAP



VICINITY MAP



GENERAL

- GI000.1 COVER SHEET
- GI001.1 SHEET INDEX
- GI002.1 CODE SUMMARY DFCM
- GI003.1 SPECIAL INSPECTIONS
- GI101.1 LEVEL 1 CODE PLAN

CIVIL

- CG400.1 GRADING PLAN
- CU300.1 UTILITY PLAN

ARCHITECTURAL SITE

- AS101.1 ARCHITECTURAL SITE PLAN

ARCHITECTURAL

- AE100.1 DIMENSION CONTROL PLAN
- AE101.1 LEVEL 1 FLOOR PLAN
- AE171.1 LEVEL 1 REFLECTED CEILING PLAN
- AE191.1 ROOF PLAN
- AE201.1 BUILDING ELEVATIONS

STRUCTURAL

- SE001.1 GENERAL STRUCTURAL NOTES
- SE002.1 GENERAL STRUCTURAL NOTES
- SE003.1 GENERAL STRUCTURAL NOTES
- SE101.1 FOOTING AND FOUNDATION PLAN
- SE102.1 LOW ROOF FRAMING PLAN
- SE103.1 HIGH ROOF FRAMING PLAN
- SE201.1 ELEVATIONS
- SE211.1 BUILDING ELEVATIONS
- SE301.1 BUILDING SECTIONS
- SE501.1 FOOTING AND FOUNDATION DETAILS
- SE502.1 FOOTING AND FOUNDATION DETAILS
- SE701.1 ROOF FRAMING DETAILS
- SE702.1 ROOF FRAMING DETAILS
- SE703.1 ROOF FRAMING DETAILS
- SE704.1 STEEL STUD FRAMING DETAILS
- SE801.1 CONCRETE SCHEDULES
- SE802.1 STEEL SCHEDULES
- SE803.1 MASONRY SCHEDULES
- SE804.1 STEEL STUD SCHEDULES
- SE805.1 DIAPHRAGM SCHEDULE

ELECTRICAL

- EE001.1 SHEET INDEX, AND ABBREVIATIONS
- ES101.1 ELECTRICAL SITE PLAN
- ES501.1 ELECTRICAL DETAILS
- EP601.1 ONE-LINE DIAGRAM



PROJECT **24-038**

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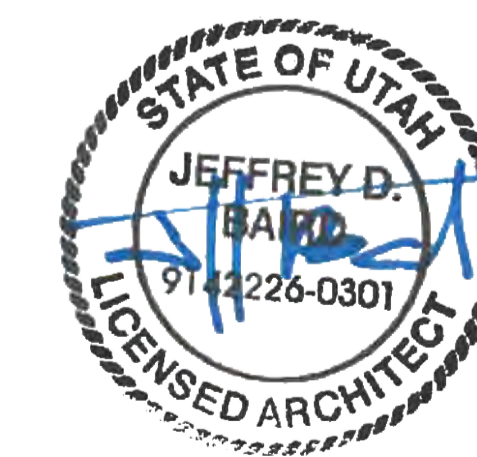
REVISIONS

NO.	DATE	DESCRIPTION
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**DAVIS TECHNICAL COLLEGE
WELDING TECHNOLOGY BUILDING**
355 SOUTH 650 EAST
KAYSVILLE, UT 84037



DAVISTECH
DAVIS TECHNICAL COLLEGE



SHEET INDEX
GI001.1

DFCM GUIDELINES FOR SEISMIC RESTRAINT OF NONSTRUCTURAL COMPONENTS

1. General Comments: a) These guidelines shall apply to all nonstructural components installed in newly constructed buildings... 2. Checklist Requirements: a) All DFCM projects shall have the "Nonstructural Component Checklist" clearly shown on the front of the construction plans... 3. Submittal Requirements: a) The seismic restraint requirements for nonstructural components may be provided with the original construction documents... 4. Construction Documents: a) The construction documents must include seismic restraint details providing specific information relating to the materials, type, size, and locations of anchorages... 5. Seismic Restraint Design Requirements: a) Per IBC 1813.1, the seismic restraint of nonstructural components shall meet the requirements of ASCE 7... 6. Deferred Submittals: a) Deferred submittals of seismic restraint of nonstructural components must be submitted to the DFCM Building Official a minimum of two weeks prior to the planned installation...

DFCM SUBMITTAL NOTES

- 1. DEFERRED SUBMITTALS FOR SEISMIC RESTRAINT OF NONSTRUCTURAL COMPONENTS MUST BE SUBMITTED TO THE DFCM BUILDING OFFICIAL A MINIMUM OF TWO WEEKS PRIOR TO THE PLANNED INSTALLATION... 2. IF SEISMIC RESTRAINTS OF NON-STRUCTURAL COMPONENTS ARE INSTALLED PRIOR TO RECEIVING DFCM APPROVAL THEY SHALL NOT BE COVERED OR CONCEALED UNTIL RECEIVING BOTH PLAN REVIEW AND INSPECTION APPROVAL... 3. THE REQUIREMENTS FOR SEISMIC RESTRAINT OF NONSTRUCTURAL COMPONENTS CANNOT BE SATISFIED BY A GENERAL REFERENCE TO DESIGN MANUALS... 4. SUBMITTALS MUST INCLUDE DETAILS OF THE PROPOSED SEISMIC RESTRAINT OF NONSTRUCTURAL COMPONENTS... REQUIREMENTS FOR OCCUPANCY 1. A CODE INSPECTION REPORT RECOMMENDING THAT A CERTIFICATE OF OCCUPANCY BE ISSUED... 2. FINAL REPORT FROM THE SPECIAL INSPECTION AGENCY... 3. CERTIFICATE OF FIRE CLEARANCE FROM THE STATE FIRE MARSHALL... 4. FINAL APPROVAL FROM THE STATE ELEVATOR INSPECTOR, IF APPLICABLE... 5. FINAL APPROVAL FROM THE STATE BOILER INSPECTOR, IF APPLICABLE... 6. REPORT OF THE DISINFECTION OF THE POTABLE WATER SYSTEM, IPC 610... 7. A CERTIFICATE OF COMPLIANCE FROM THE APPROVED FABRICATOR, IF A APPLICABLE, IBC 1704.2.2... 8. A STAMPED AND SIGNED FINAL REPORT FROM THE STRUCTURAL ENGINEER WHEN STRUCTURAL OBSERVATION IS REQUIRED BY IBC 1710... 9. FINAL REPORT FROM THE SPECIAL INSPECTOR AND THE MECHANICAL ENGINEER WHEN SMOKE CONTROL IS REQUIRED... 10. THE NFRC CERTIFICATE TO SHOW COMPLIANCE WITH THE FENESTRATION REQUIREMENTS OF THE INTERNATIONAL ENERGY CODE.

NONSTRUCTURAL COMPONENT CHECKLIST

Table with 5 columns: ITEM DESCRIPTION, NOT REQUIRED, ON CONST. DOCUMENTS, DEFERRED SUBMITTAL, COMMENTS. Rows include ARCHITECTURAL COMPONENTS (Interior Nonstructural Walls & Partitions, Cantilever Elements, etc.), MEP COMPONENTS (Fire Sprinklers, Mechanical Equipments, etc.), and other structural elements.

CODE ANALYSIS

APPLICABLE CODES

Table with 3 columns: Code Name, Year, Code Name, Year. Includes International Building Code 2021, National Electrical Code 2020, International Mechanical Code 2021, etc.

- A. Occupancy and Group: B Change in Use: Yes No X Mixed Occupancy: Yes No X Special Use and Occupancy (e.g. High Rise, Covered Mall): N/A B. Seismic Design Category: D Design Wind Speed: 105 mph C. Type of Construction (circle one): I A, II A, III A, IV HT, V A, VI B D. Fire Resistance Rating Requirements for the Exterior Walls based on the fire separation distance (in hours): North: 0 South: 0 East: 0 West: 0 E. Mixed Occupancies: Nonseparated Uses: F. Sprinklers: Required: Provided: Type of Sprinkler System: FULL NFPA-13

G. Number of Stories: 1 Building Height: 20'-0" H. Actual Area per Floor (square feet): 17,052

I. Tabular Area: 36,000 J. Area Modifications: N/A

- a) N/A b) Sum of the Ratio Calculations for Mixed Occupancies: Actual Area <= 1 Allowable Area c) Total Allowable Area for: 1) One Story: N/A 2) Two Story: A (2) N/A 3) Three Story: A (3) N/A d) Unlimited Area Building: Yes No X Code Section: TABLE 507.4

K. Fire Resistance Rating Requirements for Building Elements (hours).

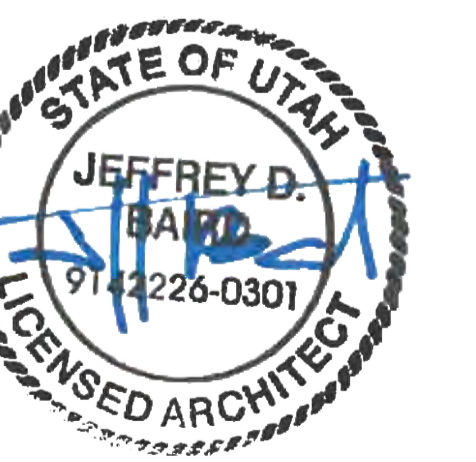
Table with 6 columns: Element, Hours, Assembly Listing, Element, Hours, Assembly Listing. Rows include Exterior Bearing Walls, Interior Bearing Walls, Exterior Non-Bearing Walls, Structural Frame, Partitions - Permanent, Fire Barriers.

L. Design Occupant Load: 147 Stair Occupant Load: N/A Exit Width Required: 30" Stair Width Required: N/A Exit Width Provided: 144" Stair Width Provided: N/A

- M. Minimum Number of Required Plumbing Facilities: a) Water Closets - Required 4 Total Provided: 5 b) Lavatories - Required 3 Total Provided: 4 c) Bath Tubs or Showers: Required: 0 Provided: 0 d) Drinking Fountains: 2 Service Sinks: 1 Provided Level 1: (DF) 2, Level 1: (SS) 1

FOOTNOTES: 1) In case of conflict with the U.S. Department of Justice Federal Registers Parts I through V - ADA Guidelines and specific reference to the International Building Code Accessibility Chapters, the more restrictive requirement shall govern. 2) Additional Code Information shall be provided at the discretion of the Building Official for Complex Buildings. Including, but not limited to: a) High Rise Requirements. b) Atriums. c) Performance Based Criteria. d) Means or Egress Analysis. e) Fire Assembly Locator Sheet. f) Exterior and Interior Accessibility Route. g) Fire Stopping, Including Tested Design Number.

DAVIS TECHNICAL COLLEGE WELDING TECHNOLOGY BUILDING 355 SOUTH 650 EAST KAYSVILLE, UT 84037



CODE SUMMARY DFCM G1002.1

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Special Inspection, Material Testing & Structural Observation
Items Required by Chapter 17 of the 2021 IBC

Indicate items requiring special inspection, structural testing, or structural observations by checking the appropriate box. All items not requiring inspection/testing should be removed from the form. For items requiring continuous inspection, a special inspector must be present onsite during the performance of that task. In most cases "periodic" inspections shall be performed prior to commencing the task, intermittently during the task, and at the completion of the task.

Table with columns: Fabricator Name, Fabrication plant location, Required Inspections, and checkboxes for various inspection types.

STRUCTURAL STEEL (IBC 1705.2.1, 1705.12.1 & 1705.13.1)

Please refer to drawing SE003.1 for further information re: Structural Steel.

Table with columns: Item, Proposed Frequency, and Name of Structural Observer. Includes sections for Prior to Welding, Welding, and During Welding.

Table with columns: Item, Proposed Frequency, and Name of Structural Observer. Includes sections for Masonry Construction and Masonry Construction Begins.

Table with columns: Item, Proposed Frequency, and Name of Structural Observer. Includes sections for Helical Pile Foundations and Storage Racks.

Table with columns: Item, Proposed Frequency, and Name of Structural Observer. Includes sections for Sprayed Fire-Resistant Materials and Mechanical & Electrical Components.

Table with columns: Item, Proposed Frequency, and Name of Structural Observer. Includes sections for Mastic and Intumescent Fire-Resistant Coatings and Exterior Insulation and Finish Systems (EIFS).

Table with columns: Item, Proposed Frequency, and Name of Structural Observer. Includes sections for Fire-Resistant Penetrations and Joints, Smoke Control, and Architectural Components.

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GENERAL NOTES

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- DO NOT SCALE DRAWINGS
- ITEMS HALF-TONED SHOWN FOR REFERENCE ONLY.



PROJECT **24-038**

BID PACKAGE #1 2024-08-26

REVISIONS		
NO.	DATE	DESCRIPTION

CODE PLAN LEGEND

ASSEMBLY GROUP A - SMALL ASSEMBLY SPACE (OCCUPANT LOAD < 50) - FIXED SEATS	
BUSINESS AREAS (150 GROSS)	
ACCESSORY STORAGE AREAS, MECHANICAL EQUIPMENT ROOM (300 GROSS)	
COMMON PATH OF TRAVEL	
TOTAL PATH OF TRAVEL	

KEYNOTES

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TOTAL PATH
80' - 1 29/32"

CLASSROOM
108
1128 SF
41 SEATS

TOTAL PATH
172' - 2 9/16"

BUSINESS AREAS
14642 SF
@ 150
98 OCC.

TOTAL PATH
82' - 10 11/16"

TOTAL PATH
148' - 3 3/16"

STORAGE
117
1017 SF
@ 300
4 OCC.

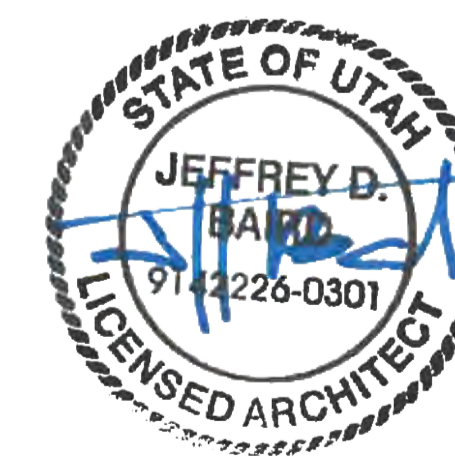
COMMON PATH
16' - 2 27/32"

COMPRESSOR
119
1017 SF
@ 300
4 OCC.

TOTAL PATH
130' - 4 29/32"

LEVEL 1 FLOOR PLAN
1/8" = 1'-0"

**DAVIS TECHNICAL COLLEGE
WELDING TECHNOLOGY BUILDING**
355 SOUTH 650 EAST
KAYSVILLE, UT 84037



LEVEL 1 CODE
PLAN
G1101.1

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8/27/2024 12:27:56 AM

EXISTING BARLOW BUILDING

BUILDING
FF=4444.00

GENERAL GRADING NOTES

1. HANDICAP PARKING AREA SHALL NOT EXCEED 2% IN ANY DIRECTION. THE PERPENDICULAR CROSS SLOPE TO PARKING STALL IN OTHER AREAS OF THE PARKING LOT SHALL NOT EXCEED 0% IN SLOPE AND SLOPE SHALL NOT EXCEED 0% IN ANY DIRECTION FOR PARKING AREAS.
2. ALL WALKWAYS SHALL NOT EXCEED 5% SLOPE 2% MAX. FROM BUILDING OR STAIR RISERS FOR 6" MIN. REFER TO PLAN AT ALL DOORWAYS TO THE BUILDING ALSO SLOPE 2% MAX FOR 5' AT THE END OF THE 1:12 SLOPE OF ALL H.C. RAMPS. ALL STEPS AND RAMPS ARE DETAILED ON THE ARCHITECTURAL SITE PLANS.
3. SITE CLEARING, SUBGRADE PREPARATION, EXCAVATION, AND BACKFILL WILL BE IN ACCORDANCE WITH THE REQUIREMENTS OUTLINED IN THE GEOTECHNICAL REPORT. SITE PAVEMENT THICKNESS WILL ALSO IN ACCORDANCE WITH THE GEOTECHNICAL REPORT. USE MINIMUM PAVEMENT THICKNESS OUTLINE IN NOTES 4 AND 5 IF GEOTECHNICAL REPORT HAS LESS STRINGENT REQUIREMENTS.
4. CONCRETE DRIVEWAY TO BE CONSTRUCTED PER APWA STANDARD PLAN 225. ALL OTHER CONCRETE PAVEMENT FOR VEHICLES SHALL BE A MINIMUM OF 6" OF CONCRETE (4500 psi) OVER 4" OF BASE COURSE.
5. ALL ASPHALT PAVING TO BE 4" OF ASPHALT (1 1/2" OF 1/2" MIX OVER 2 1/2" OF 3/4" MIX) OVER 8" OF BASE COURSE.
6. ALL CONCRETE AND ASPHALT PAVEMENT TO MEET REQUIREMENTS OF THE APWA SPECIFICATIONS. BASE COURSE TO MEET UDOT SPECIFICATIONS (1 1/2" GRADATION).

CRSA

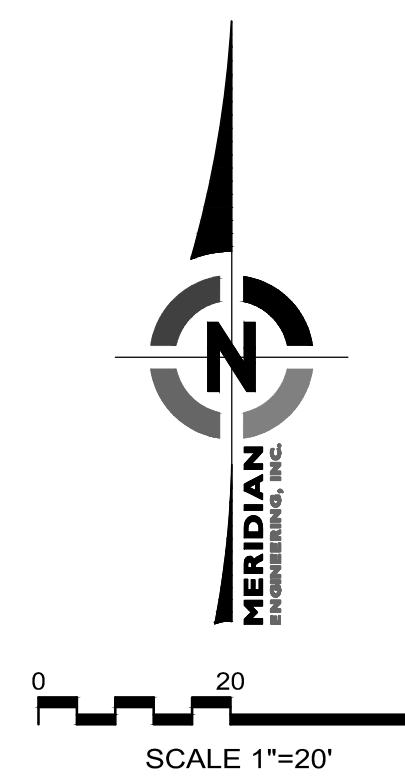
PROJECT **24-038**

BID PACKAGE #1 2024-08-26

REVISIONS

NO.	DATE	DESCRIPTION

**DAVIS TECHNICAL COLLEGE
WELDING TECHNOLOGY BUILDING**
550 EAST 300 SOUTH,
KAYSVILLE, UT 84037



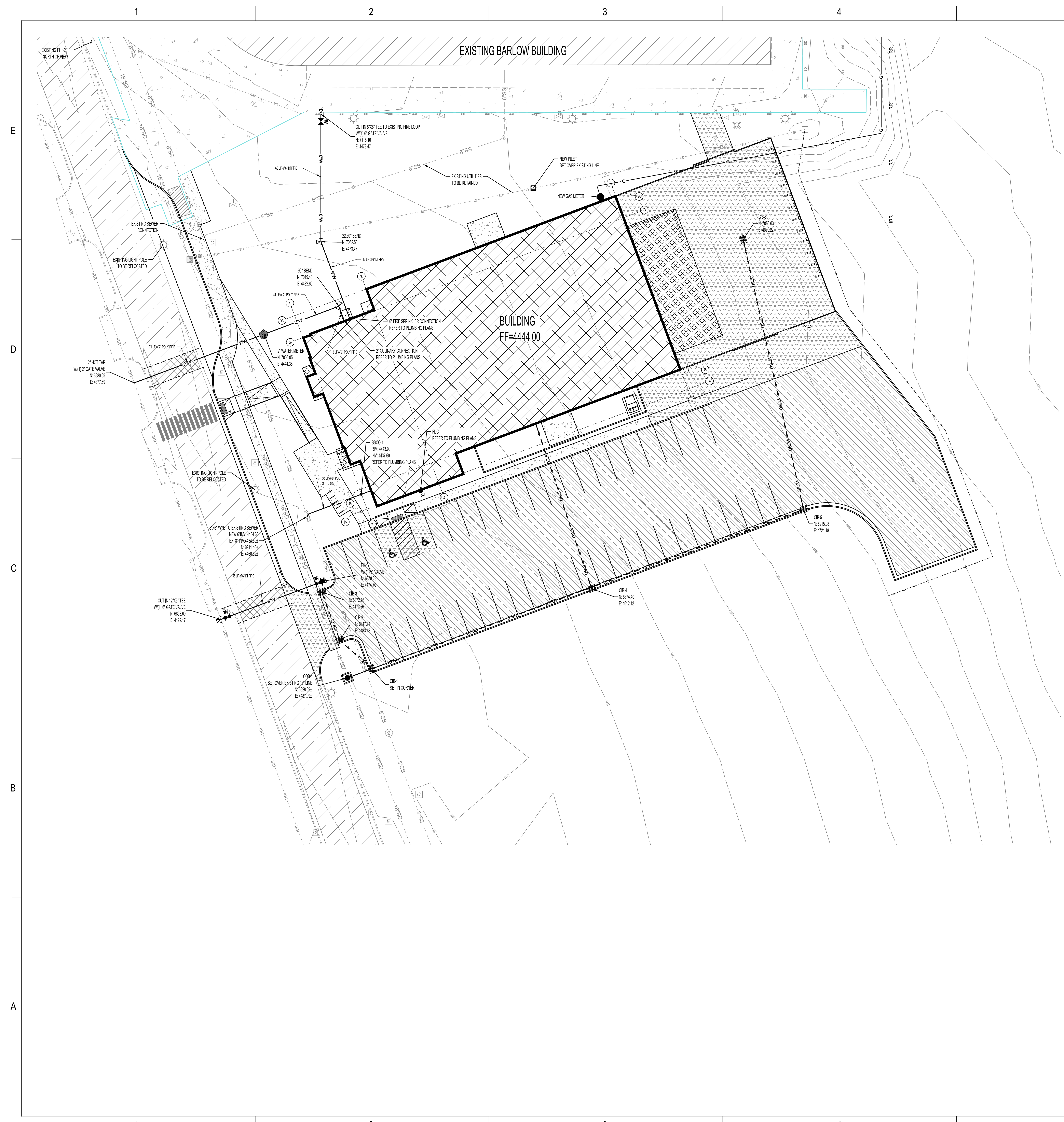
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GRADING PLAN

CG400

(801) 355-6915





EXISTING BARLOW BUILDING

BUILDING
FF=4444.00

GENERAL UTILITY NOTES:

- PLUMBING CONTRACTOR WILL TERMINATE THEIR ROOF DRAIN LINES WITH A CLEAN OUT APPROXIMATELY 5' FROM THE BUILDING. COORDINATE WITH PLUMBING CONTRACTOR ON SCHEDULE AND ALIGNMENT OF ROOF DRAIN LINES NEAR THE BUILDING.
- ALIGN ALL INTERIOR AND EXTERIOR UTILITIES. SITE UTILITY CONTRACTOR TO COORDINATE PLACEMENT HORIZONTALLY AND VERTICALLY WITH BUILDING PLUMBING CONTRACTOR. SITE INTERFACE LINE BETWEEN THE BUILDING PLUMBING CONTRACTOR AND THE SITE UTILITY CONTRACTOR WILL BE AT 5' FROM THE BUILDING AND EXCEPT FOR THE FIRE SPRINKLER LINE AND WATER LINES A CLEAN OUT WILL BE INSTALLED BY THE PLUMBING CONTRACTOR APPROXIMATELY 5' FROM THE BUILDING FOR STORM DRAIN AND SEWER LINES. CONNECTION TO BUILDING PIPING AND ALL PIPING BEYOND THIS INTERFACE SHALL BE THE SITE UTILITY CONTRACTOR'S RESPONSIBILITY. PROVIDE REDUCERS, ADAPTERS, OR OTHER FITTINGS AS REQUIRED AT THE INTERFACES TO CONNECT TO BLDG. PIPE. COLLECT ROOF DRAIN LINES AS SHOWN AND ROUTE TO NEW CATCH BASINS OR CLEAN OUTS ON SITE. PREFERRED SLOPES, APPROXIMATE DISTANCES, AND INVERTS OF GRAVITY PIPING ARE SHOWN ON THE PLAN MAY REQUIRE ADJUSTMENT TO CONNECT TO BUILDING ROOF OR SEWER DRAIN LINES. MAINTAIN 2% SLOPE FOR 4" DIAMETER OR SMALLER PIPES, 1% FOR 6" AND 0.4% FOR 8" DIAMETER PIPES.
- SITE CONTRACTOR SHALL COORDINATE WITH KAYSVILLE CITY INSPECTOR WHEN COMPLETING CONNECTIONS TO LINES ALONG DAVIS TECH DRIVE OR ON SITE WHERE REQUIRED. ALL WATER AND SEWER SYSTEM DETAILS AS WELL AS INSPECTIONS FOR THE ENTIRE SITE SHALL BE IN ACCORDANCE WITH CENTRAL DAVIS SEWER DISTRICT AND KAYSVILLE CITY STANDARD DETAILS AND SPECIFICATIONS. SEE GENERAL NOTES ON SHEET C-100. WHERE THREAT BUILDING CANNOT BE CONNECTED TO OTHER ADJACENT UTILITIES OR OTHER SITE CONSTRAINTS, RESTRAINED JOINTS WILL BE REQUIRED PER CITY STANDARD SPEC'S THURST BLOCK ALL WATERLINE FITTINGS PER CITY STANDARDS TYP.
- NO CONNECTION SHALL BE ALLOWED TO THE 8" FIRE LOOP EXCEPT FOR FIRE HYDRANTS AND THE FIRE SPRINKLER LINE. REFER TO LANDSCAPING PLANS FOR DOUBLE CHECK AND STOP & WASTE LOCATION AND DETAILS FOR RRR SYSTEM.
- COORDINATES FOR FIRE HYDRANTS, 30" CATCH BASINS, OR CLEAN OUTS ARE AT THE CENTER OF THE UTILITY SURFACE FEATURE. COORDINATES FOR WATERLINE ANGLE POINTS ARE AT THE CENTER OF THE PIPELINE. COORDINATES FOR CURB INLETS ARE AT THE FACE OF THE CURB AT THE CENTER OF THE INLET. ALL STORM DRAIN BOXES ARE 24" EXCEPT THOSE INLETS PLACED IN CURB AND GUTTERS.
- ALL VALVES, AREA CATCH BASINS (NOT IN C&G), CLEAN OUTS, OR MANHOLES SHALL HAVE CONCRETE GRADE ADJUSTMENT COLLARS PLACED AROUND THE STRUCTURE.
- STORM DRAIN CLEAN OUTS TO BE SIMILAR TO DETAIL SHOWN ON PLUMBING PLANS.
- ROOF DRAIN CONNECTIONS AT CATCH BASINS OR CLEAN OUT BOXES TO BE CORE DRILLED AND EPOXY GROUTED INTO PRECAST BOXES DUE TO FIELD ADJUSTMENTS WHICH MAY BE NECESSARY TO CONNECT TO BUILDING PIPING.
- THE FIRE SPRINKLER LINE AND DOMESTIC WATER LINES SHALL BE ROUTED INTO THE FIRE SPRINKLER ROOM INSIDE THE BUILDING AND TERMINATE 12" ABOVE FINISH FLOOR WITH A FLANGE FITTING. CAP WITH BAND FLANGES FOR LINE TESTING. REFER TO PLUMBING PLANS FOR RISER LOCATION IN THE BUILDING. THE FIRE SERVICE LINE AND FIRST FIVE FEET OF THE DOMESTIC WATER LINE SHALL BE CEMENT LINED DUCTILE IRON PIPE PER AWWA C151 350mm AND AWWA C104 WRAPPED IN POLYETHYLENE PER AWWA C105 FROM THE BUILDING CONNECTION TO THE TEE AT THE FIRE CONNECTION PER CITY STANDARD SPECIFICATIONS AND DETAILS.
- ALL PAVEMENT REPAIR IN DAVIS TECH DRIVE TO BE IN ACCORDANCE WITH AWWA STANDARDS. REPAIRS TO MATCH EXISTING PAVEMENT THICKNESS. USE 6" ASPHALT OVER 12" BASE COURSE IF EXISTING PAVEMENT IS LESS THAN THIS THICKNESS. (TYP.)
- ALL CONSTRUCTION PIPING MATERIALS AND INSTALLATION TO BE PER CITY STANDARDS FOR CULINARY WATER, SANITARY SEWER LINES AND STORM DRAIN LINES.
NEW WATER LINES - KAYSVILLE CITY STDS. TO METER CONNECTION, DIP CLASS 51, FIRE SPRINKLER & 4" CULINARY WATER LINES TO BE D.I. PER NOTE 9 ABOVE AND PER CITY STANDARDS.
SEWER LINES AND MANHOLES - CENTRAL DAVIS SEWER DISTRICT STDS., PVC PIPING (SDR 35), PRECAST MANHOLES.
STORM DRAIN - KAYSVILLE CITY STDS., RCP (CLASS III), ALONG DAVIS TECH DRIVE, 12 TO 15' PIPING HIGH PERFORMANCE HOPE FOR SITE AREA.
ROOF DRAIN PIPING - PROJECT PLUMBING SPECIFICATIONS, CAST IRON SOIL PIPE 4" TO 8" ROOF DRAIN PIPING WRAPPED IN POLYETHYLENE SLEEVES PER AWWA C105.
- BACKFLOW PROTECTION SHALL BE IN ACCORDANCE WITH ALL UTILITY DIVISION OF DRAWING REGULATIONS AND STANDARDS. BACKFLOW DEVICES AND THE STOP AND WASTE VALVE ARE SHOWN ON THE LANDSCAPE DRAWINGS.
- INSPECTION AND APPROVAL FOR ANY SEWER/WATER LINE CROSSINGS SHALL BE REVIEWED AND APPROVED BY CITY PRIOR TO CONSTRUCTION OF THE CROSSING. CITY SHALL ALSO INSPECT THE CROSSING PRIOR TO BACKFILL.
- ALL WATER LINES SHALL MAINTAIN A MINIMUM OF FOUR FEET OF COVER AT ALL TIMES. THE ANTICIPATED FROST DEPTH IS 30 INCHES.
- UNDERGROUND FIRE SERVICE MAINS TO BE FLUSHED PER CITY AND NFPA STANDARDS 13 AND 24.
- CONTRACTOR TO PROVIDE ELECTRICAL & TELEPHONE LINE TRENCHING AND BACKFILL. COORDINATE LOCATIONS WITH POWER AND TELEPHONE COMPANIES.
- WATER VALVES, SEWER MANHOLES, STORM DRAIN INLETS OR CLEANOUTS BOXES, AND OTHER SURFACE UTILITY APPARATUS SHALL BE RAISED TO ACCURATE FINISH SURFACE BY A CONCRETE GRADE COLLAR. THE CONCRETE COLLAR WILL EXTEND 12" MINIMUM AROUND THE UTILITY APPARATUS WITH A 10" MINIMUM THICKNESS. PLACE 24H HOOPS IN CONCRETE COLLAR.



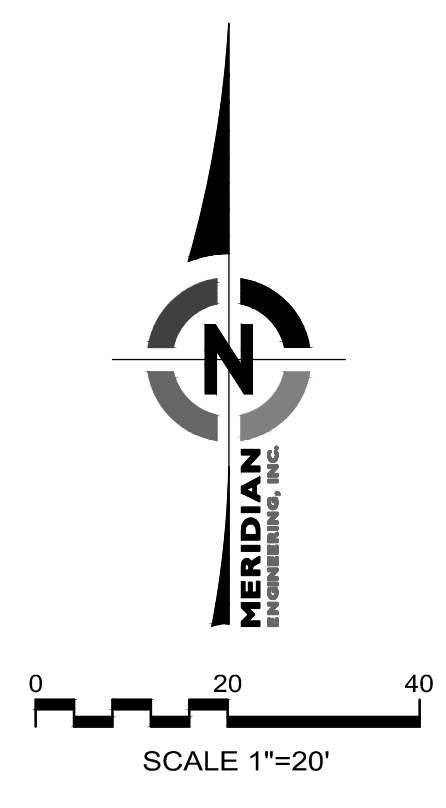
PROJECT **24-038**

BID PACKAGE #1 2024-08-26

REVISIONS

NO.	DATE	DESCRIPTION
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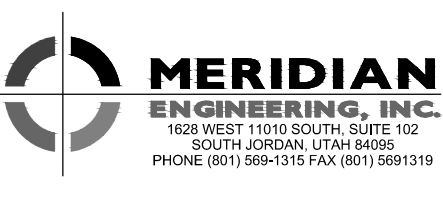
**DAVIS TECHNICAL COLLEGE
WELDING TECHNOLOGY BUILDING**
550 EAST 300 SOUTH,
KAYSVILLE, UT 84037



NOT FOR CONSTRUCTION

UTILITY PLAN

CU300



(801) 355-5915

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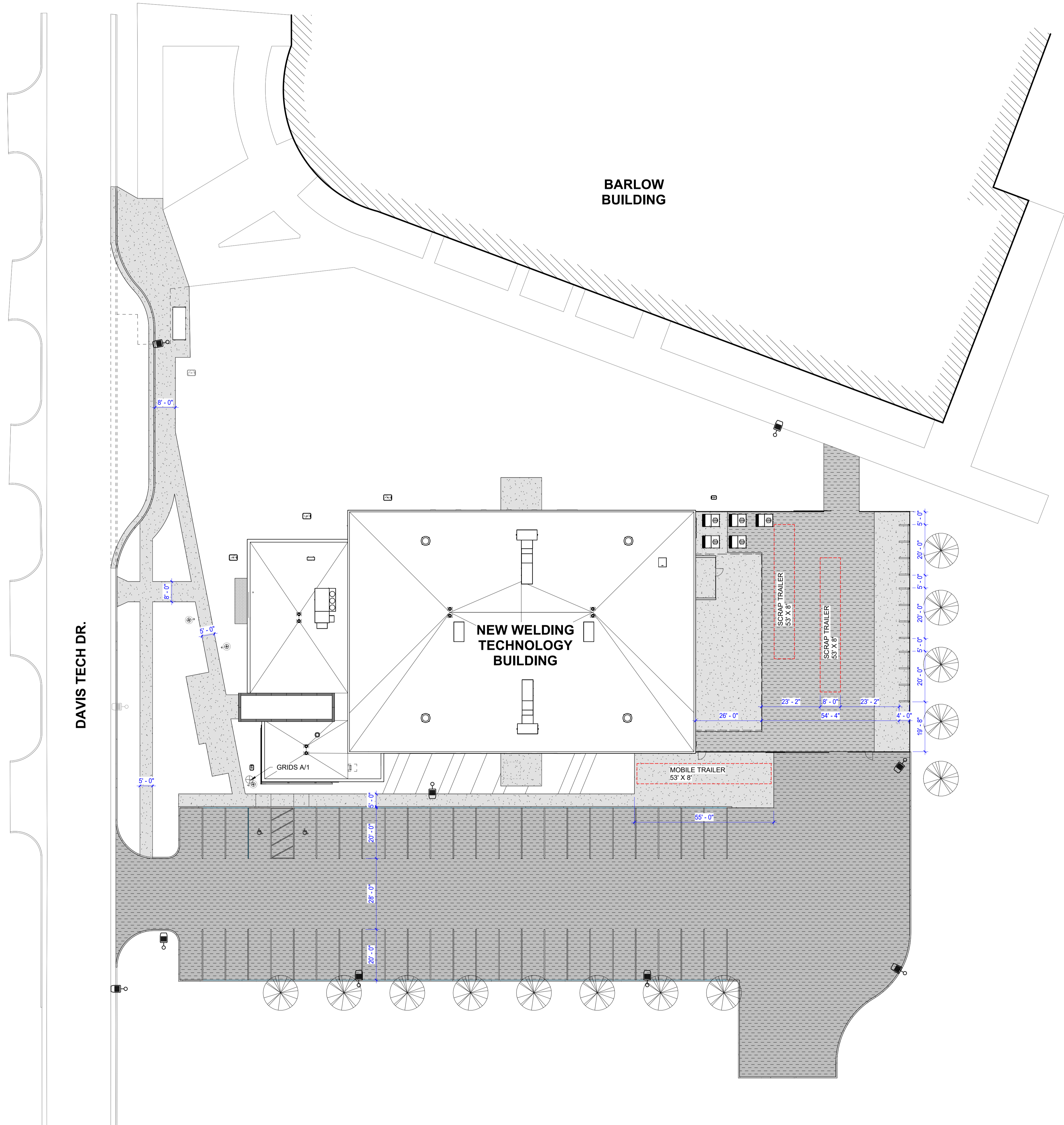
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A2 ARCHITECTURAL SITE PLAN
AS101.1 1" = 20'-0"

GENERAL NOTES

- GENERAL CONTRACTOR SHALL FIELD VERIFY ALL CONDITIONS AND SHALL REPORT TO THE ARCHITECT ANY UNKNOWN CONDITIONS, ERRORS OR CONFLICT IN THE DRAWINGS BEFORE BEGINNING WORK.
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- ITEMS HALF-TONED SHOWN FOR REFERENCE ONLY.



PROJECT **24-038**

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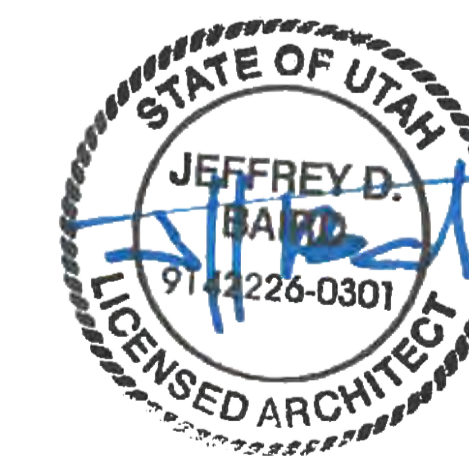
REVISIONS		
NO.	DATE	DESCRIPTION

ARCHITECTURAL SITE PLAN LEGEND

NEW ASPHALT PAVING	
NEW CONCRETE PAVING	
EXTENTS OF SITE DEMOLITION	

KEYNOTES

**DAVIS TECHNICAL COLLEGE
WELDING TECHNOLOGY BUILDING**
355 SOUTH 650 EAST
KAYSVILLE, UT 84037



ARCHITECTURAL
SITE PLAN
AS101.1

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GENERAL NOTES

- GENERAL CONTRACTOR SHALL FIELD VERIFY ALL CONDITIONS AND SHALL REPORT TO THE ARCHITECT ANY UNKNOWN CONDITIONS, ERRORS OR CONFLICT IN THE DRAWINGS BEFORE BEGINNING WORK.
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- SEE ELEVATIONS FOR OPENING HEIGHTS.



PROJECT 24-038

BID PACKAGE #1 2024-08-26

NO.	DATE	DESCRIPTION

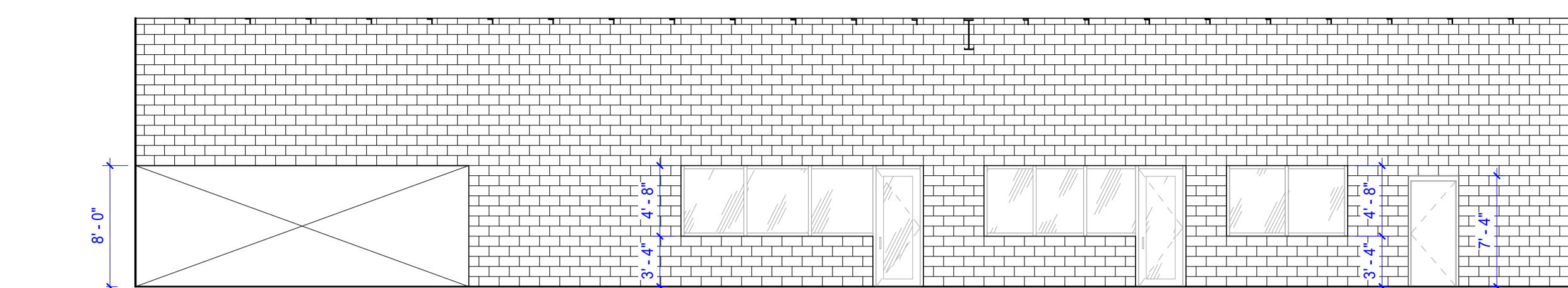
LEGEND

SLAB ON GRADE	
MASONRY WALL	

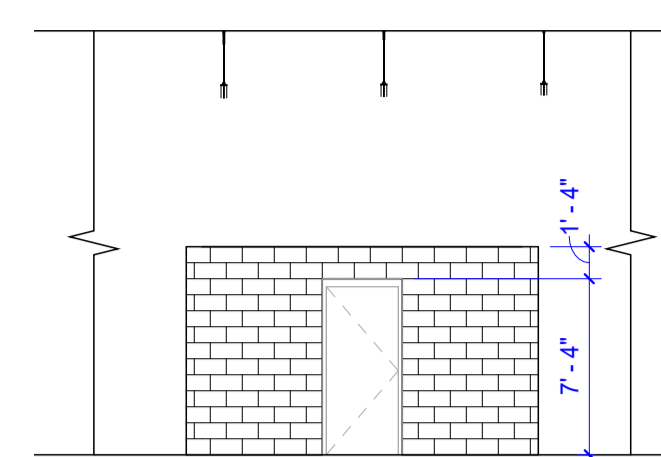
KEYNOTES

3.01	CONCRETE FOUNDATION WALL
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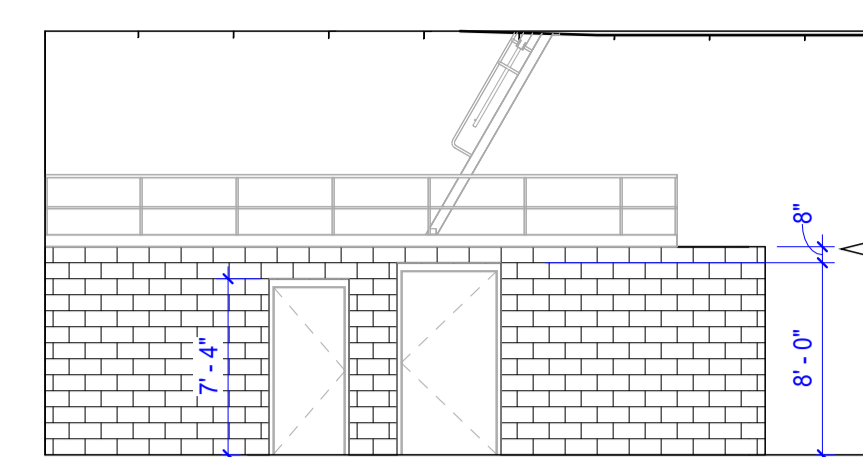
E



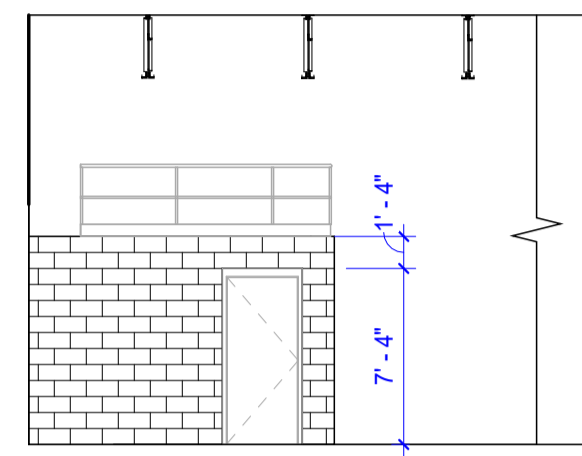
E1 MASONRY ELEVATION - WEST
1/8" = 1'-0"



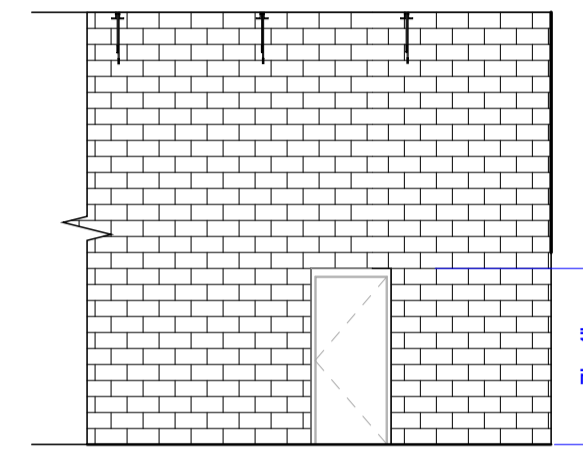
E4 MASONRY ELEVATION SOUTH
1/8" = 1'-0"



E5 MASONRY ELEVATION EAST
1/8" = 1'-0"

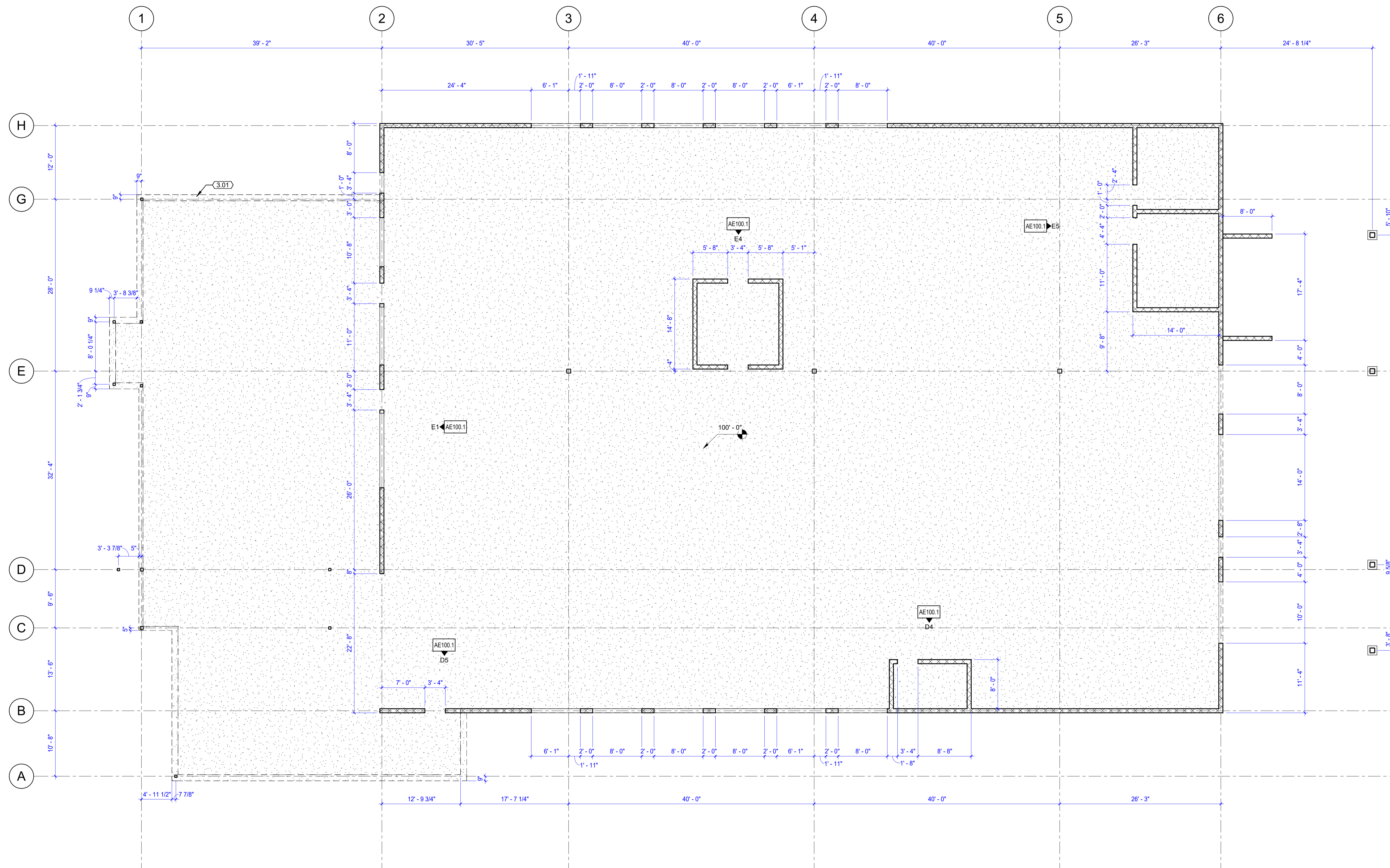


D4 MASONRY ELEVATION SOUTH 2
1/8" = 1'-0"



D5 MASONRY ELEVATION 3
1/8" = 1'-0"

D



A1 DIMENSION CONTROL PLAN
1/8" = 1'-0"

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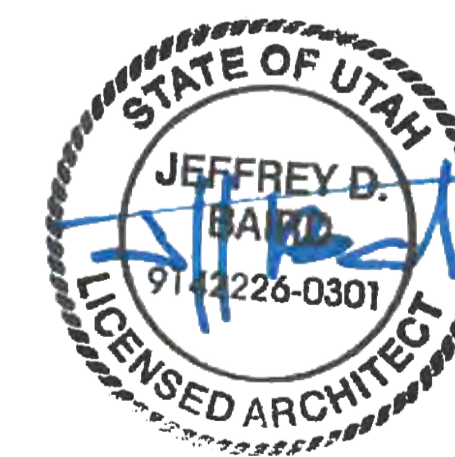
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**DAVIS TECHNICAL COLLEGE
WELDING TECHNOLOGY BUILDING**
355 SOUTH 650 EAST
KAYSVILLE, UT 84037



DIMENSION
CONTROL PLAN
AE100.1

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GENERAL NOTES

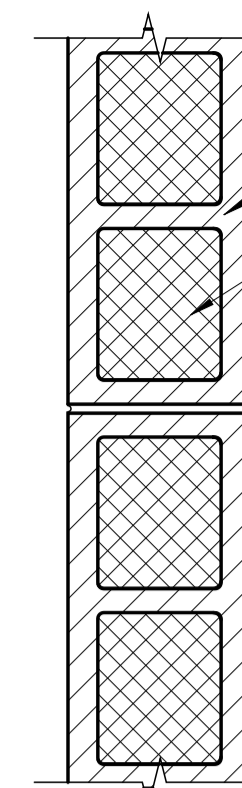
- GENERAL CONTRACTOR SHALL FIELD VERIFY ALL CONDITIONS AND SHALL REPORT TO THE ARCHITECT ANY UNKNOWN CONDITIONS, ERRORS OR CONFLICT IN THE DRAWINGS BEFORE BEGINNING WORK.
- DO NOT SCALE DRAWINGS
- ITEMS HALF-TONED SHOWN FOR REFERENCE ONLY.



PROJECT 24-038

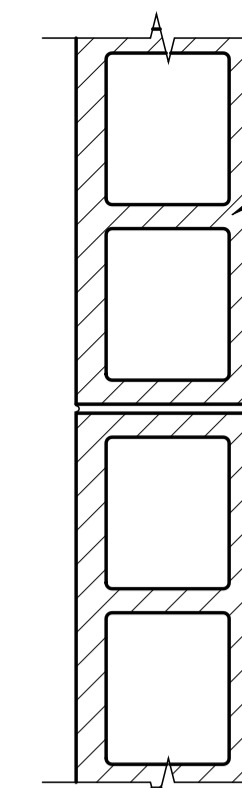
BID PACKAGE #1 2024-08-26

NO.	DATE	DESCRIPTION



8" CMU - CONCRETE MASONRY UNITS

EPS FOAM INSULATION



8" CMU - CONCRETE MASONRY UNITS

M1	M2
FIRE RATING: N/A	FIRE RATING: N/A
FIRE TEST: N/A	FIRE TEST: N/A
STC RATING: N/A	STC RATING: N/A
SOUND TEST: N/A	SOUND TEST: N/A
WALL HEIGHT: SEE ELEVATIONS	WALL HEIGHT: SEE ELEVATIONS

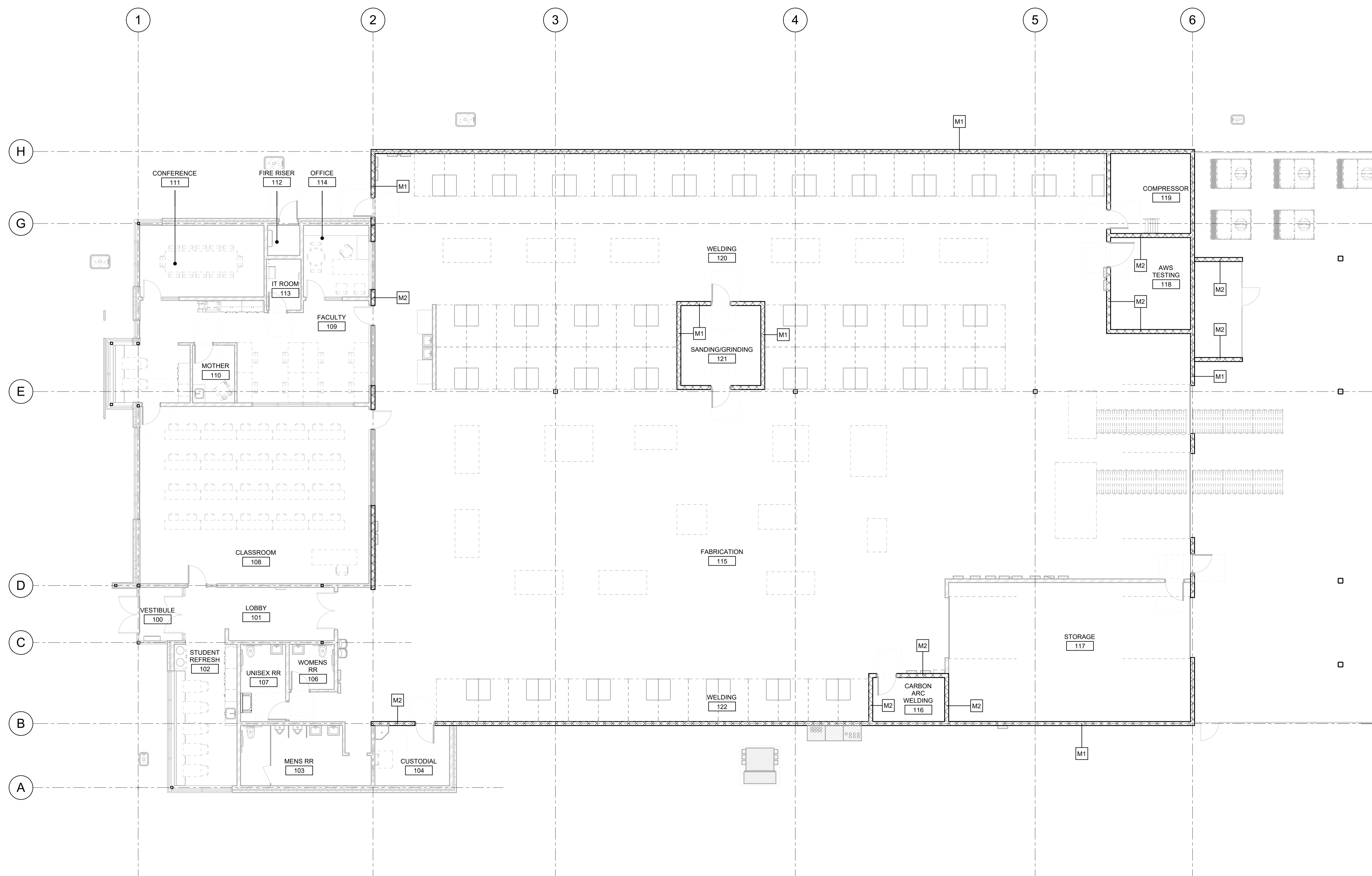
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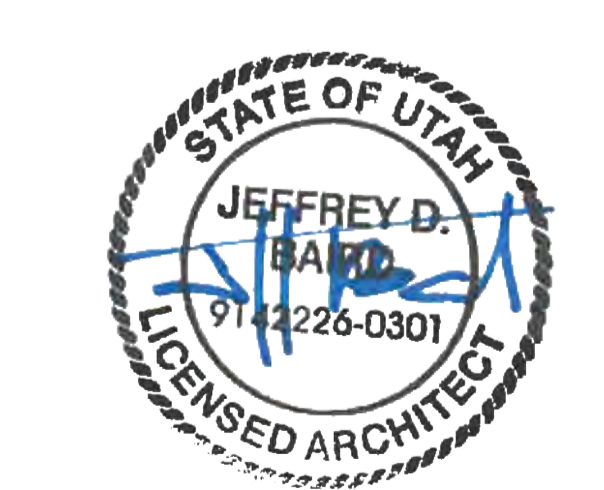
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KEYNOTES

**DAVIS TECHNICAL COLLEGE
WELDING TECHNOLOGY BUILDING**
355 SOUTH 650 EAST
KAYSVILLE, UT 84037



A1 LEVEL 1 FLOOR PLAN
1/8" = 1'-0"

LEVEL 1 FLOOR PLAN
AE101.1

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8/27/2024 12:28:00 AM

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GENERAL NOTES

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PROJECT 24-038

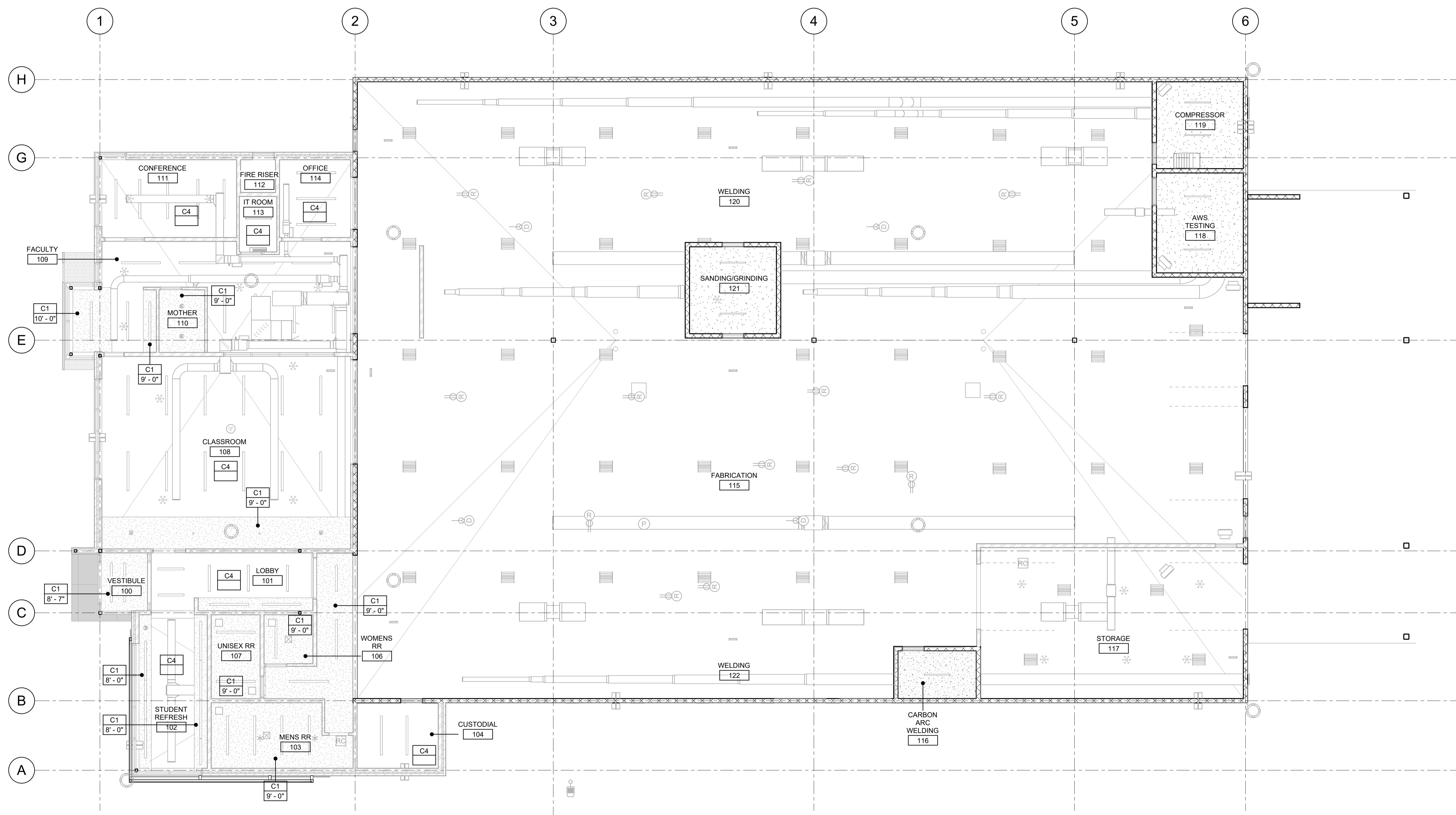
BID PACKAGE #1 2024-08-26

REVISIONS		
NO.	DATE	DESCRIPTION

CEILING LEGEND

GYPSUM BOARD CEILING PAINT.	C1 X-X'	
OPEN TO STRUCTURE ABOVE	C4 X-X'	
WINDOW SHADES		

KEYNOTES



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A1 LEVEL 1 CEILING PLAN
1/8" = 1'-0"

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DAVIS TECHNICAL COLLEGE
WELDING TECHNOLOGY BUILDING

355 SOUTH 650 EAST
KAYSVILLE, UT 84037



FOR REFERENCE ONLY

LEVEL 1
REFLECTED
CEILING PLAN
AE171.1

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PROJECT **24-038**

BID PACKAGE #1 2024-08-26

REVISIONS		
NO.	DATE	DESCRIPTION

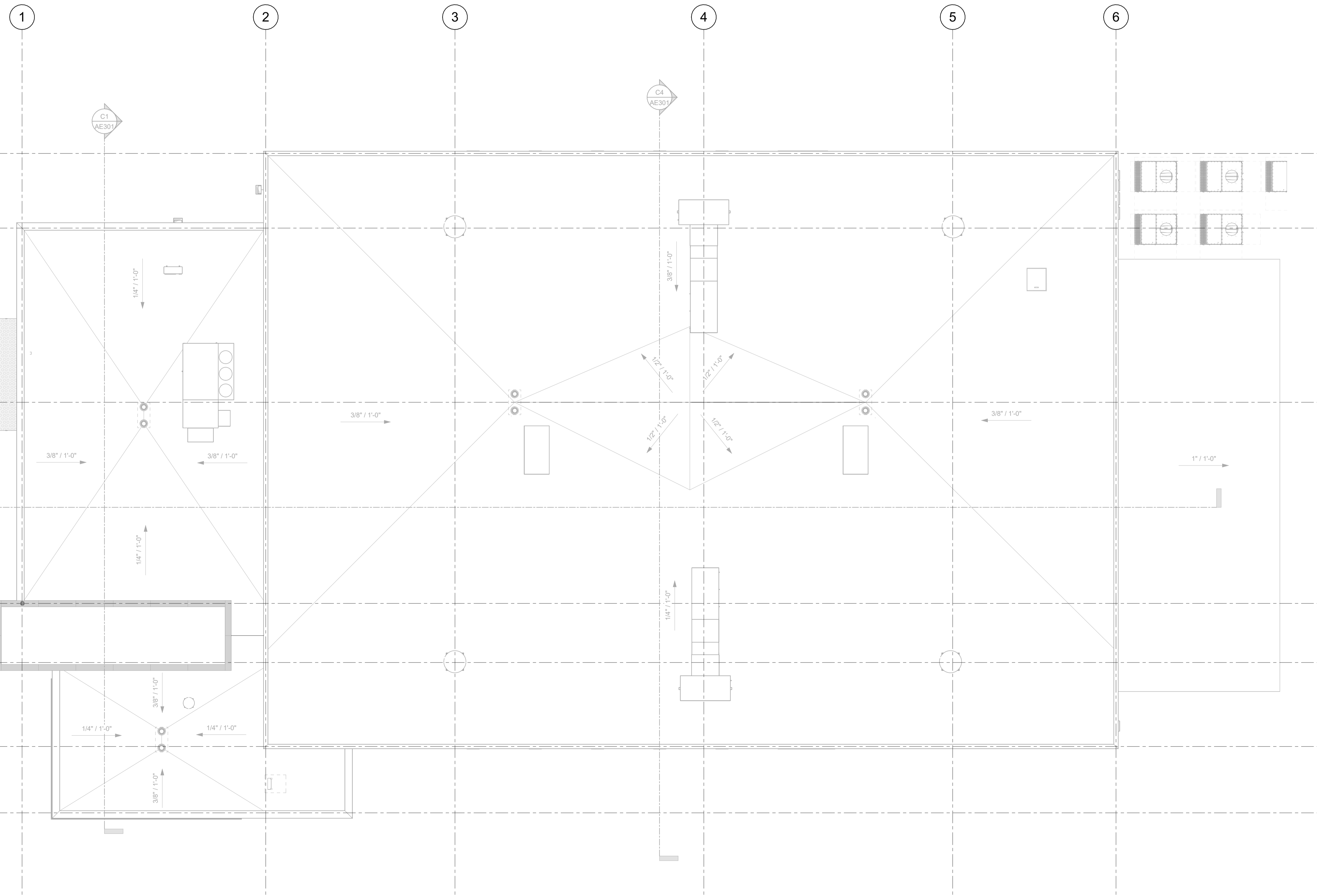
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KEYNOTES

**DAVIS TECHNICAL COLLEGE
WELDING TECHNOLOGY BUILDING**
355 SOUTH 650 EAST
KAYSVILLE, UT 84037



FOR REFERENCE ONLY

A1 ROOF PLAN
1/8" = 1'-0"

ROOF PLAN
AE191.1

8/27/2024 12:28:46 AM

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Please refer to the attached revised rendering (the first page in this bid package).

The field CMU will from from the bottom of the wall to the bottom sill of the window band and will be don with 8" Split Faced integrally colored (Sunroc - Tumbleweed).
 An accent band will go from the bottom sill of the window band to the top of the wall and will be done with 8" Smooth Faced CMU integrally colored (Sunroc - Cream).
 The brick will 4x4x16 Emperor brick (Interstate Brick - Midnight Black, or approved equal).
 Please refer to the attached revised rendering (the first page in this bid package).

Or approved Equal

GENERAL NOTES

GENERAL CONTRACTOR SHALL FIELD VERIFY ALL CONDITIONS AND SHALL REPORT TO THE ARCHITECT ANY UNKNOWN CONDITIONS, ERRORS OR CONFLICT IN THE DRAWINGS BEFORE BEGINNING WORK.
 DO NOT SCALE DRAWINGS
 ITEMS HALF-TONED SHOWN FOR REFERENCE ONLY.



PROJECT 24-038

BID PACKAGE #1 2024-08-26

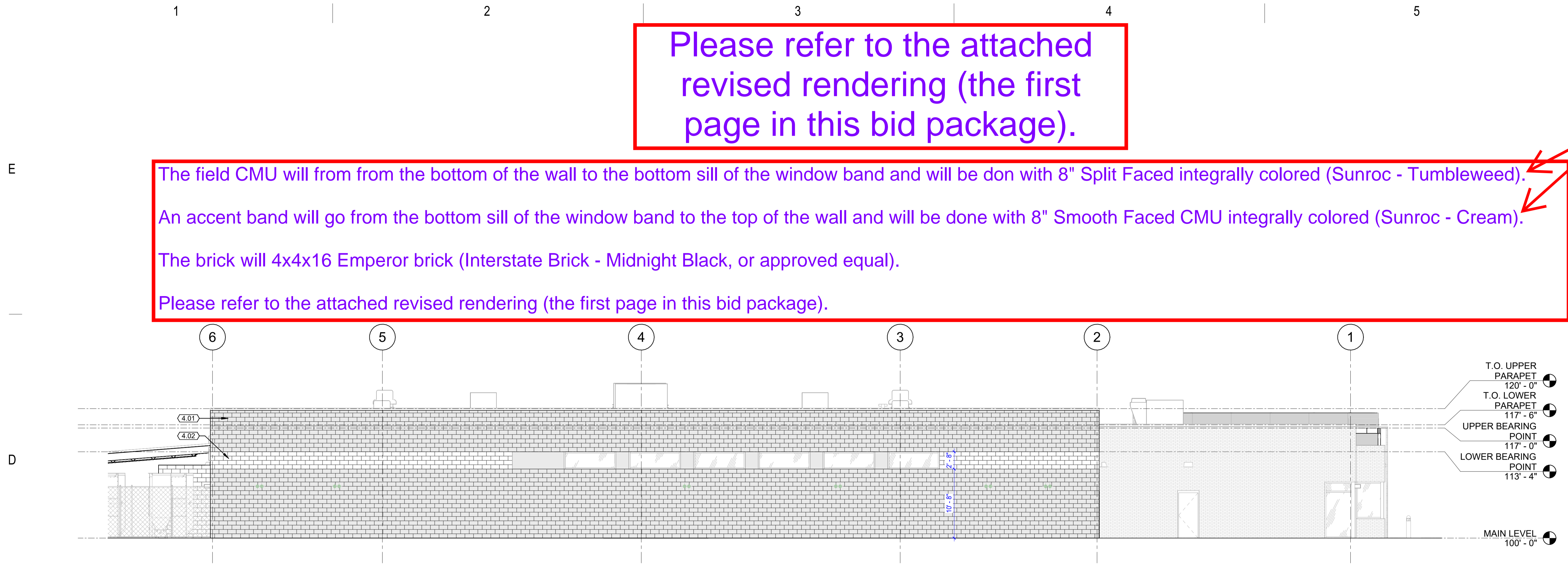
REVISIONS
 NO. DATE DESCRIPTION

LEGEND

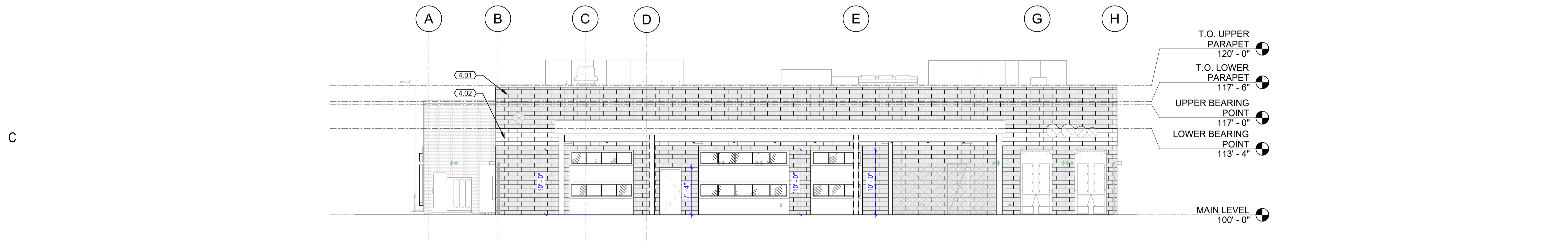
KEYNOTES

4.01	SPLIT FACE CONCRETE MASONRY UNIT Sunroc - Tumbleweed
4.02	SMOOTH FACE CONCRETE MASONRY UNIT Sunroc - Cream

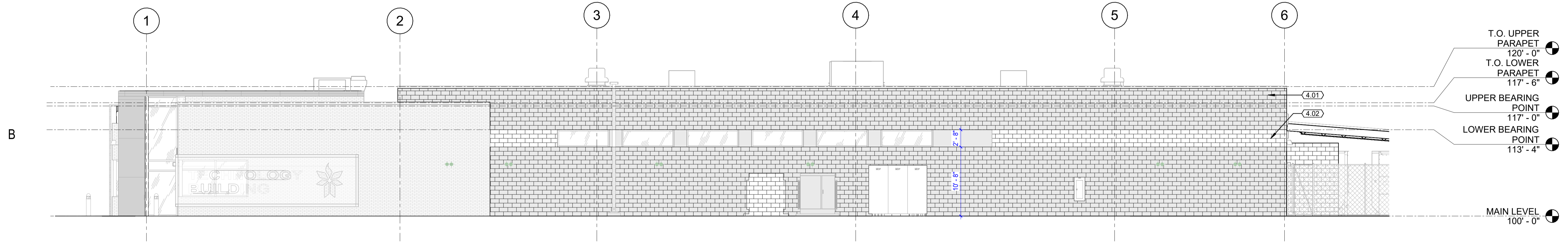
The Accent band has been adjusted to go from the bottom sill of the window band to the top of the wall. Please refer to the attached revised rendering (The first page in this bid package).



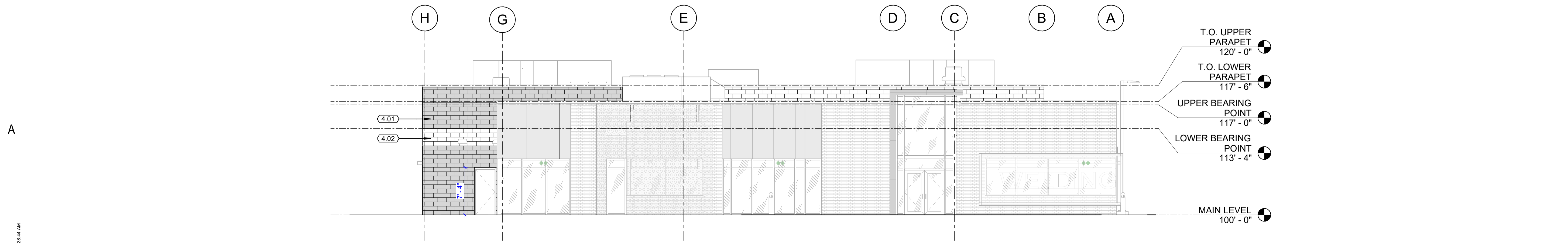
D1 BUILDING ELEVATION - NORTH
 1/8" = 1'-0"



C2 BUILDING ELEVATION - EAST
 1/8" = 1'-0"

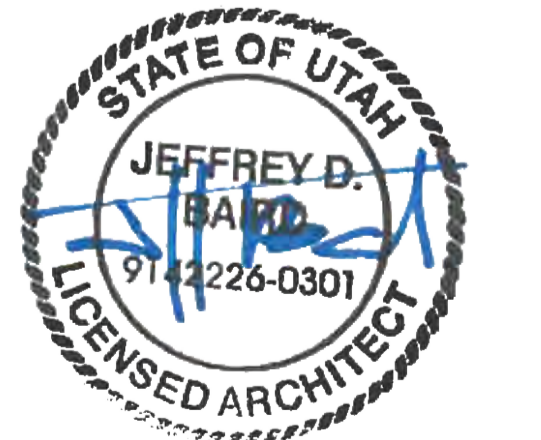


B1 BUILDING ELEVATION - SOUTH
 1/8" = 1'-0"



A2 BUILDING ELEVATION - WEST
 1/8" = 1'-0"

DAVIS TECHNICAL COLLEGE
 WELDING TECHNOLOGY BUILDING
 355 SOUTH 650 EAST
 KAYSVILLE, UT 84037



BUILDING ELEVATIONS
 AE201.1

GENERAL STRUCTURAL NOTES

GENERAL

- 1. The structural notes are intended to complement the project specifications. Specific notes and details in the drawings shall govern over the structural notes and typical details.
2. Typical details and sections shall apply where specific details are not shown.
3. The contractor shall be responsible for all conditions that differ from those shown in the contract drawings...

BASES OF DESIGN

- 1. Governing Building Code: International Building Code 2021
2. Risk Category: III
3. Floor Live Loads: Uniformly Distributed Loads
4. Roof Live Load: 20 psf or 300 lbs

CONCRETE

Table with 4 columns: Concrete Use, Comp. Strength (f'c), Exposure Classes per ACI 318 19.3.1 (a,b,c), and Nominal Max. Aggregate Size. Rows include Footings, Foundation Walls, Interior Slabs on Grade, Exterior Concrete, Reinforced, and Unreinforced.

STEEL

- 1. Steel Decking: Minimum 16 gauge, 24" deep, 2" stiffeners.
2. Steel Joists: K-Series, 16" deep, 24" spacing.
3. Steel Beams: I-Beams, 16" deep, 24" spacing.
4. Steel Columns: H-Columns, 16" deep, 24" spacing.

WOOD

- 1. Wood Joists: 2x12, 24" spacing, 16' max span.
2. Wood Beams: 6x8, 24" spacing, 16' max span.
3. Wood Columns: 4x4, 16' max height.

EXTERIOR FACADE FRAMING AND CLADDING

- 1. All systems (including facade, cladding, components, elements and/or attachments to the structure) intended for use to enclose the building shall comply with the following criteria for integrity...
2. Systems shall be designed and detailed for seismic (probable) intensity, drift and/or other systems shall remain intact, undamaged and/or...
3. All systems shall be designed and detailed for seismic (probable) intensity, drift and/or other systems shall remain intact, undamaged and/or...

FOUNDATION

- 1. Soil Report by: Geotechnical Engineering
2. Report Number and Date: GEC-2024-0923-24
3. Soil Bearing Pressure: 2000 psf on structural fill or natural soils.
4. Foundation Type: 12" x 12" cast-in-place concrete.

MECHANICAL

- 1. Mechanical systems shall be installed in accordance with the manufacturer's instructions.
2. All mechanical systems shall be installed in accordance with the manufacturer's instructions.
3. All mechanical systems shall be installed in accordance with the manufacturer's instructions.

MASONRY

- 1. Materials shall be as shown, unless noted otherwise.
2. Design strength of all masonry shall be f'm=2000 psi at 28 days, unless noted otherwise.
3. Concrete Masonry Units (CMU) shall be ASTM C-90, Grade N, Type 1, medium weight (115 pcf) with a minimum unit strength of 2000 psi at 28 days for not in place.
4. Solid Concrete Masonry Units shall be ASTM C92, Grade SW, with a minimum compression strength of 3000 psi at 28 days.

REINFORCING BARS

- 1. Reinforcing bars shall be provided in accordance with the manufacturer's instructions.
2. All reinforcing bars shall be provided in accordance with the manufacturer's instructions.
3. All reinforcing bars shall be provided in accordance with the manufacturer's instructions.

CONSTRUCTION

- 1. Construction Joints and Control Joints: Provide a control joint in all horizontal and vertical structural members including between top of footing and foundation walls.
2. Control Joints: Control joints shall be installed in slabs on grade so the length of the slab is no more than 12m (40'). Control joints shall be completed within 12 hours of concrete placement.
3. Tied Joints: Tied joints shall be installed in slabs on grade so the length of the slab is no more than 12m (40').

POST-INSTALLED ANCHORS

- 1. Post-installed anchors shall be installed in accordance with the manufacturer's instructions.
2. All post-installed anchors shall be installed in accordance with the manufacturer's instructions.
3. All post-installed anchors shall be installed in accordance with the manufacturer's instructions.

MASONRY VENEER

- 1. Masonry veneer shall be attached to steel stud and wood stud walls with 'Holman & Barnard HB 213 seismic veneer anchors' (or equal) spaced at 16" on center.
2. Masonry veneer shall be attached to reinforced masonry walls with 'Hollow stud type reinforcement' spaced at a maximum of 16" on center vertically.
3. Masonry veneer shall be attached to reinforced masonry walls with 'Hollow stud type reinforcement' spaced at a maximum of 16" on center vertically.

STEEL DECKING

- 1. Steel deck shall comply with the latest requirements of the Steel Deck Institute.
2. Steel deck material shall comply with the manufacturer's ICC Report and have a minimum yield strength of 50ksi.
3. All steel deck shall be 3-span continuous minimum. In areas where 3-span continuous is not possible, the contractor shall provide a bridge gird deck as required to provide the equivalent loading of the specified deck under a 3-span condition.

OPEN STEEL JOISTS AND GIRDERS

- 1. All open web steel joist and girders shall be fabricated and erected in accordance with the latest edition of Steel Joist Institute (SJI) 'Standard Specifications and Code of Standard Practice'.
2. All open web steel joist and girders shall be fabricated and erected in accordance with the latest edition of Steel Joist Institute (SJI) 'Standard Specifications and Code of Standard Practice'.
3. All open web steel joist and girders shall be fabricated and erected in accordance with the latest edition of Steel Joist Institute (SJI) 'Standard Specifications and Code of Standard Practice'.

METAL DECKING

- 1. Steel deck shall comply with the latest requirements of the Steel Deck Institute.
2. Steel deck material shall comply with the manufacturer's ICC Report and have a minimum yield strength of 50ksi.
3. All steel deck shall be 3-span continuous minimum. In areas where 3-span continuous is not possible, the contractor shall provide a bridge gird deck as required to provide the equivalent loading of the specified deck under a 3-span condition.

COLD FORMED STEEL

- 1. Cold Formed Steel Framing: A. The design of cold formed steel and low alloy steel structural members shall be in accordance with AISI/S100, North American Specification for the Design of Cold-Formed Steel Structural Members.
2. Cold Formed Steel Decking: A. The design of cold formed steel and low alloy steel structural members shall be in accordance with AISI/S100, North American Specification for the Design of Cold-Formed Steel Structural Members.
3. Cold Formed Steel Decking: A. The design of cold formed steel and low alloy steel structural members shall be in accordance with AISI/S100, North American Specification for the Design of Cold-Formed Steel Structural Members.

WOOD STRUCTURE

- 1. Wood Structure: A. The design of wood structure shall be in accordance with the National Design Specification (NDS) for Wood Construction.
2. Wood Structure: A. The design of wood structure shall be in accordance with the National Design Specification (NDS) for Wood Construction.
3. Wood Structure: A. The design of wood structure shall be in accordance with the National Design Specification (NDS) for Wood Construction.

GENERAL NOTES

- 1. The contractor shall be responsible for all conditions that differ from those shown in the contract drawings.
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GENERAL NOTES

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GENERAL NOTES

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PROJECT 240104

BID PACKAGE #1 2024-08-26

REVISIONS

Table with 3 columns: NO., DATE, DESCRIPTION. Revision 1: 2024-08-26, Description: Additions to structural notes and drawings.

DTC WELDING TECH & FABRICATION
BUILDING 355 SOUTH 650 EAST KAYSVILLE, UT 84037



DAVIS TECHNICAL COLLEGE



2024-08-26 BID PACKAGE #1

GENERAL STRUCTURAL NOTES SE001.1

(801) 355-5915

GENERAL STRUCTURAL NOTES



PROJECT 240104

BID PACKAGE #1 2024-08-26

REVISIONS NO. DATE DESCRIPTION

DTC WELDING TECH & FABRICATION BUILDING 355 SOUTH 650 EAST KAYSVILLE, UT 84037



2024-08-26 BID PACKAGE #1

NOTE: THESE STRUCTURAL DRAWINGS ARE BASED ON ARCHITECTURAL DRAWINGS DATED July 23, 2024. DIMENSIONS AND ELEVATIONS AS THEY RELATE TO THE BUILDING IN GENERAL, IN GRID TO GRID DIMENSIONS OR DECK BEARING ELEVATIONS, ARE SUPPLIED BY THE ARCHITECT. THEY ARE PROVIDED ON THE STRUCTURAL PLANS AND DETAILS FOR THE CONVENIENCE OF THE CONTRACTOR. VERIFY DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS.

GENERAL STRUCTURAL NOTES SE002.1 (801) 355-5915

ABBREVIATIONS

Table with 3 columns: Abbreviation, Full Name, and Description. Includes items like AB Anchor Bolt, ALT Alternate, ARCH Architect, ADDL Additional, etc.

SPECIAL INSPECTION, TESTING AND STRUCTURAL OBSERVATION REQUIREMENTS

- 1. Special Inspections and Testing
A. Special inspections and testing as required per the approved construction documents and per IBC Chapter 17 shall be provided for this project unless waived by the Building Official.
B. An independent agency, or agencies, employed by the Owner, shall perform the special inspection and testing services required.
C. The special inspection and testing requirements of this section of the General Structural Notes and the special inspection tables serve as the Engineer of Record's statement of special inspections and structural observations required by IBC Chapter 17.

- 5. Special Inspections for Wind Resistance (1705.12): Special inspections for wind resistance are not required for this project per IBC Section 1705.12.
6. Special Inspections for Seismic Resistance (1705.13): Special inspections for seismic resistance are required for this project per IBC section 1705.13.
A. Structural Steel (1705.13.1): Special inspections for seismic resistance shall be in accordance with the following as applicable:
1. Seismic Force-Resisting Systems: Special inspections of structural steel in the seismic force-resisting systems of buildings and structures assigned to Seismic Design Category B, C, D, E, or F shall be performed in accordance with the Quality Assurance requirements of ASCE 341 and the construction documents.

Typically our sheets are organized as follows:

Table with 2 columns: SHEET SERIES and DESCRIPTION. Includes S-00X General Notes, Specifications and Loading Plans, S-10X Plan Sheets, S-20X Elevations, etc.

DEFERRED SUBMITTALS

- 1. Deferred submittals are items that are not part of our scope which require architectural and/or engineering review. Deferred submittals include plans, details, calculations and/or relevant design information prepared in the state in which construction will occur.
2. Deferred submittals shall first be submitted to the project architect and/or engineer for review and coordination. Upon completion of the architect/engineer review, the architect/engineer will submit the deferred submittals to the Building Official for review and approval.

DEFERRED SUBMITTALS

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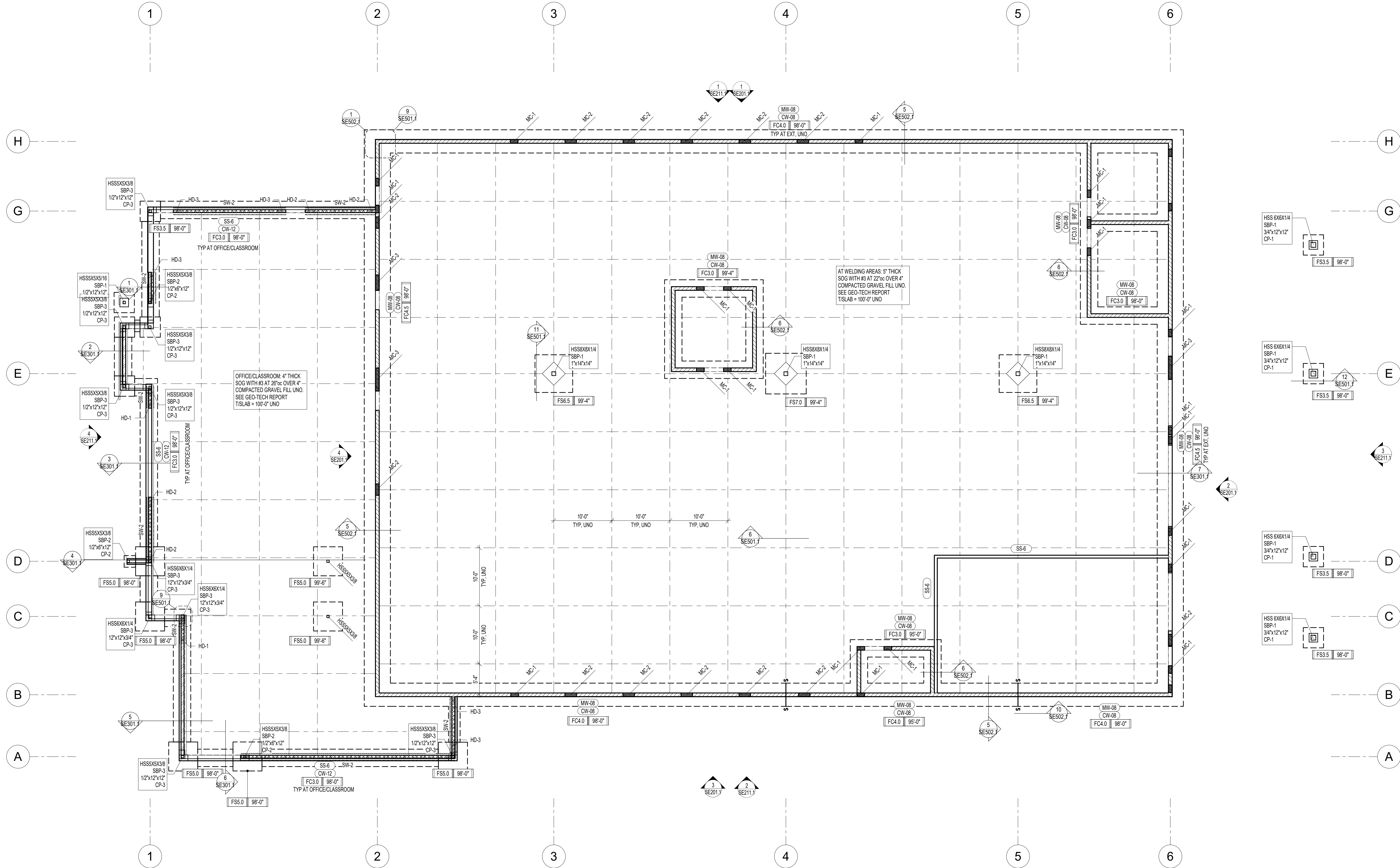
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- ### FOOTING AND FOUNDATION PLAN NOTES:
- COORDINATE LOCATION OF DEPRESSED SLABS, SLOPED SLABS AND FLOOR DRAINS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
 - SEE ARCHITECTURAL DRAWINGS AND CIVIL DRAWINGS FOR EXTERIOR CONCRETE WORK AT DOORS, SIDEWALKS, ETC. PER BC 1806.5.1. PROVIDE CLEAN GRAVEL (NO FINES) DOWN TO FROST DEPTH UNDER EXTERIOR LANDINGS OF ALL REQUIRED EXITS WITH OUTWARD-SWINGING DOORS UNDER THE FOOTPRINT OF THE DOOR SWING.
 - SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO COLUMNS, WALLS, SLAB EDGES, SLOPES, ELEVATIONS, CURBS, DEPRESSIONS AND DRAINS.
 - SEE ELEVATIONS FOR MASONRY LINTELS, PLASTERS, AND WALL REINFORCEMENT.
 - CENTER ALL SPOT FOOTINGS UNDER COLUMNS AS SHOWN ON PLAN, TYPICAL UNLESS NOTED OTHERWISE.
 - SEE SCHEDULES ON (SE500) SHEETS FOR:
 - FOOTINGS
 - CONCRETE WALLS
 - CONCRETE PIERS
 - REINFORCING SPLICE LENGTHS
 - STEEL COLUMNS
 - SEE TYPICAL FOOTING AND FOUNDATION DETAILS ON (SE500) SHEETS FOR:
 - SLAB CONSTRUCTION AND CONTROL JOINTS
 - FOOTING STEPS
 - CORNER BARS
 - PIPES PERPENDICULAR AND PARALLEL TO FOOTINGS
 - REINFORCING AT MISCELLANEOUS OPENINGS
 - REINFORCING AT SLAB DISCONTINUITIES
 - FROST COVER AND STRUCTURAL FILL
 - SEE TYPICAL FOOTING AND FOUNDATION DETAILS ON (SE500) SHEETS FOR:
 - REINFORCING AT MISCELLANEOUS MASONRY OPENINGS AND RECESSES
 - MASONRY CONTROL JOINTS
 - TERMINATION OF HORIZONTAL WALL REINFORCING AT ENDS OF MASONRY WALLS
 - CORNER WALL REINFORCING FOR SINGLE REINFORCED MASONRY WALLS
 - SEE TYPICAL STEEL STUD DETAILS ON SE704.1 INCLUDING:
 - TYPICAL BOTTOM TRACK ANCHORAGE FOR NON-SHEAR WALLS
 - STEEL STUD BRIDGING
 - DEFLECTION TRACK ASSEMBLY
 - FRAMING AT CORNERS
 - JAMBS AND HEADERS
 - NOMENCLATURE
 - PIPE HANGERS
 - NON-BEARING WALL TO METAL DECK CONNECTIONS

MARKS AND SYMBOLS LEGEND

	SECTION MARK SHEET NUMBER
	FRAME ELEVATION SHEET NUMBER SEE DETAIL (13/SE501.1)
	FOOTING DESIGNATION TOP ELEVATION
	DEPRESSED AND POUR SLAB OVER
	FLOOR OFFSET. SEE DETAILS
	CONCRETE WALL (SLP)
	MASONRY WALL
	STEEL COLUMN
	MASONRY COLUMN
	CONTROL JOINT
	CONCRETE PIER. SEE SCHEDULE
	CONTINUOUS FOOTING. SEE SCHEDULE
	SPOT FOOTING. SEE SCHEDULE
	THICKENED SLAB FOOTING. SEE SCHEDULE
	STEEL BASE PLATE. SEE SCHEDULE
	CONCRETE WALL. SEE SCHEDULE
	CONC WALL BLW AND MAS WALL ABV. SEE SCHEDULE AND ELEVATIONS
	STEEL STUD WALL. SEE SCHEDULE
	STL STUD SHEAR WALL AND TYPE ABOVE. DASHED LINE INDICATES SIDE OR SHEATHING. SEE SCHEDULE.
	CONTINUOUS ROD HOLD-DOWN LOCATION. SEE SCHEDULE
	MASONRY COLUMN. SEE SCHEDULE

NORTH
 1 FOOTING AND FOUNDATION PLAN
 SE101.1 SCALE: 1/8" = 1'-0"



PROJECT **240104**

BID PACKAGE #1 2024-08-26

REVISIONS
 NO. DATE DESCRIPTION

DTC WELDING TECH & FABRICATION
BUILDING
 355 SOUTH 650 EAST
 KAYSVILLE, UT 84037



2024-08-26
 BID PACKAGE #1

NOTE:
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FOOTING AND FOUNDATION PLAN
SE101.1
 (801) 355-5915

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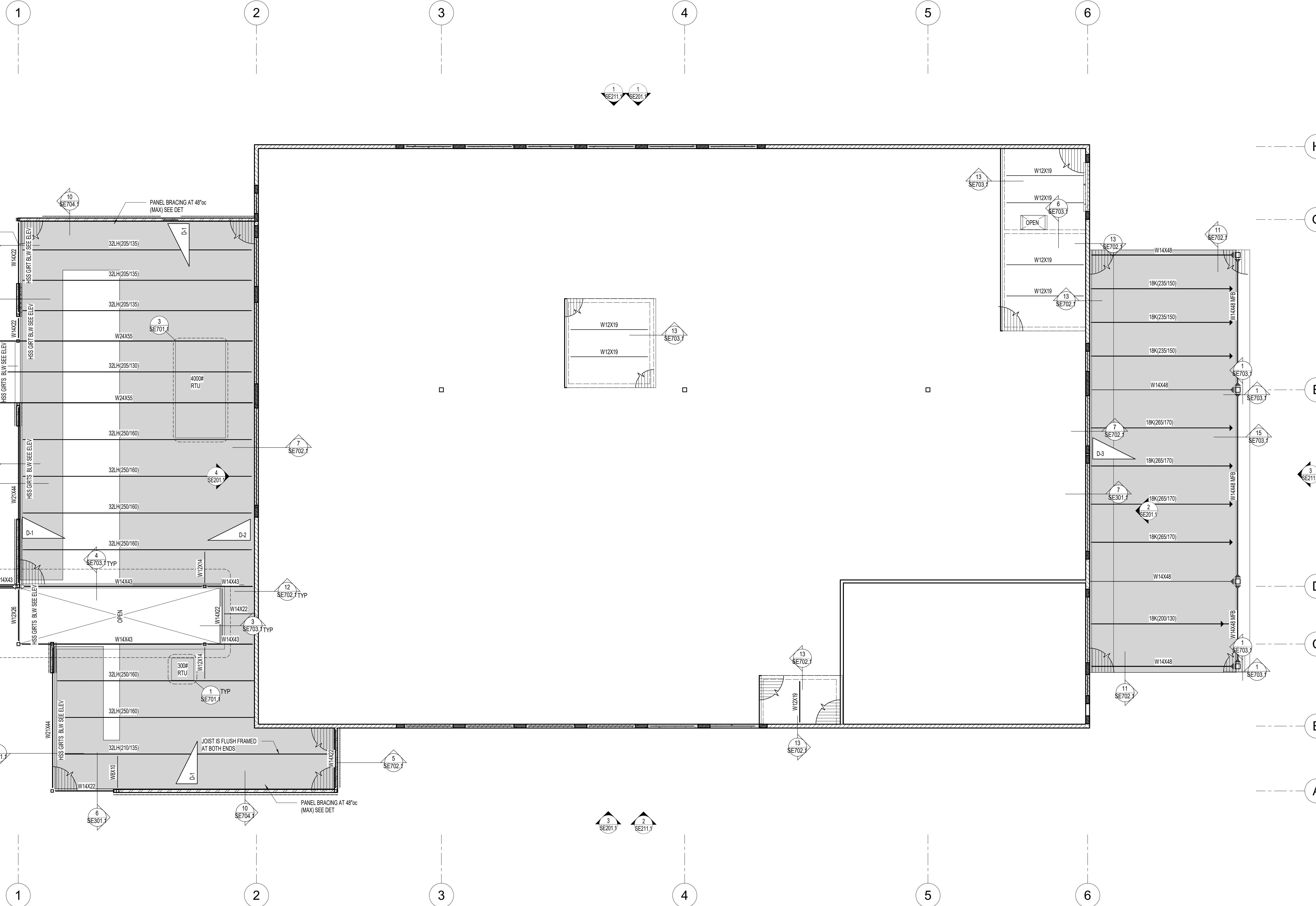
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- ROOF FRAMING PLAN NOTES:**
1. VERIFY ROOF SLOPES, DRAINS, AND DECK BEARING ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
 2. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO COLUMNS, WALLS, SLAB EDGES, SLOPES, ELEVATIONS, CURBS, DEPRESSIONS AND DRAINS.
 3. ALL JOISTS SHALL HAVE 1" DEEP BEARING ENDS UNLESS NOTED OTHERWISE.
 4. ALL ORDERS SHALL HAVE 1" DEEP BEARING ENDS UNLESS NOTED OTHERWISE.
 5. OPEN WEB STEEL JOISTS AND JOIST ORDERS SHALL BE DESIGNATED BY THE MANUFACTURER TO SUPPORT THE MECHANICAL AND LATERAL LOADS SHOWN ON THE ROOF FRAMING PLANS IN ADDITION TO THE UNIFORM AND POINT LOADS SHOWN.
 6. CONTRACTOR SHALL NOT SUPPORT ANY LOADS FROM JOIST BRIDGING.
 7. CONTRACTOR SHALL NOT ATTACH BRACING OR MEP OR FIRE PROTECTION TO BOTTOM CHORDS OF JOISTS OR BRIDGINS.
 8. WHERE SKYLIGHTS OR MECHANICAL UNITS INTERRUPT HORIZONTAL BRIDGINS, PROVIDE CROSS BRIDGINS AT JOIST SPACES ON EA. SIDE, TYPICAL.
 9. WHERE DIAGONAL BRIDGING CONFLICTS WITH MECHANICAL DUCTS, REMOVE DIAGONAL BRIDGINS AND REPLACE WITH HORIZONTAL BRIDGINS (AFTER ROOF DECK IS IN PLACE).
 10. * - INDICATES THAT THESE JOISTS SHALL BE DESIGNED FOR AN ADDL. LOAD OF 100LB/AF AT ANY TOP CHORD PANEL POINT. THIS LOAD IS TO BE ADDED TO THE ORDERS TOP CHORD LOAD MULTIPLE " " ARE ADDITIVE.
 11. JOIST DESIGNER SHALL DESIGN JOISTS AND SUPPLY ADDITIONAL BRIDGING AS REQUIRED FOR NET UPLIFT DUE TO WIND (ALLOWABLE), SEE (SE804).
 12. VERIFY SIZE, WEIGHT, AND LOCATION OF ALL ROOF TOP MECHANICAL UNITS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS. COORDINATE OPENINGS WITH MECHANICAL, ELECTRICAL, AND GENERAL CONTRACTORS.
 13. SPRINKLER DESIGNER SHALL COORDINATE THE SPRINKLER DESIGN WITH THE JOIST GIRDER SHOP DRAWINGS.
 14. ALL JOIST TOP CHORDS BEARING AT THE MASONRY WALLS SHALL BE DESIGNED FOR A MINIMUM AXIAL LOAD OF 10 KIPS IN TENSION AND COMPRESSION THAT REDUCES TO 0.0 KIPS AT THE OPPOSITE END OF THE JOIST. LOADS ARE DUE TO SEISMIC AND ARE PROVIDED AT ULTIMATE STRENGTH LEVEL INCLUDING OVERSTRENGTH FACTOR OF 1.4. JOIST SUPPLIER TO DESIGN JOISTS TO TRANSFER THIS ANCHORAGE LOAD THROUGH THE JOIST BEARING SEAT.
 15. SPRINKLER DESIGNER SHALL COORDINATE:
 - STEEL COLUMN
 - CONCRETE AND MASONRY LINTELS
 17. SEE TYPICAL FRAMING DETAILS ON (SE700) SHEETS FOR:
 - FRAMING AT ROOF OPENINGS, MECHANICAL UNITS, SKYLIGHTS AND ROOF DRAINS
 - PIPE SLEEVES THROUGH ROOF DECK
 - SUSPENDED LOADS ON ROOF DECK
 - JOIST REINFORCING AT POINT LOADS
 - JOIST TOP CHORD TIE

MARKS AND SYMBOLS LEGEND

- SECTION MARK SHEET NUMBER
- FRAME ELEVATION SHEET NUMBER
- MTL ROOF DECK, SEE GSN
- CONCRETE OVER METAL DECK, SEE GENERAL STRUCTURAL NOTES
- MASONRY WALL
- BRG WALL BLW
- STEEL COLUMN
- JOIST TOP CHORD TIE
- CONCRETE LINTEL, SEE SCHEDULE
- MASONRY LINTEL, SEE SCHEDULE
- ROOF JOIST TOP CHORD SHALL BE DESIGNED FOR AN ADDITIONAL LOAD OF 1/2 KIPS IN TENSION AND COMPRESSION. LOADS ARE PROVIDED AT ULTIMATE STRENGTH LEVEL.
- PIPE DRAIN SEE IS / SE701.1 AND SEE ARCHITECTURAL AND MECHANICAL FOR EXACT LOCATION
- LATERAL FRAME MOMENT CONNECTIONS
- FRAME OR COLLECTOR BRACE, SEE DET (15 / SE703.1)

SNOW DRIFT LOADING DIAGRAM

D-1	MAX = 33 PSF L = 7' - 0"	D-2	MAX = 63 PSF L = 22' - 2"
D-3	MAX = 57 PSF L = 24' - 2"		

MAX PSF
D-x
UNIFORM ROOF SNOW LOAD, SEE GSN

WHERE 'L' EXCEEDS LENGTH OF LOWER ROOF, DRIFT TAPERS TO 0 PSF AT THE FAR END OF LOWER ROOF. SHADED AREA ON PLAN DENOTES EXTENT OF SNOW DRIFT



PROJECT **240104**

BID PACKAGE #1 2024-08-26

REVISIONS

NO.	DATE	DESCRIPTION

DTC WELDING TECH & FABRICATION
BUILDING
 355 SOUTH 650 EAST
 KAYSVILLE, UT 84037



2024-08-26
 BID PACKAGE #1

NOTE:
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 DIMENSIONS AND ELEVATIONS AS THEY RELATE TO THE BUILDING IN GENERAL, IN GRID TO GRID DIMENSIONS OR DECK BEARING ELEVATIONS, ARE SUPPLIED BY THE ARCHITECT. THEY ARE PROVIDED FOR THE CONVENIENCE OF THE CONTRACTOR. VERIFY DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS.

LOW ROOF
 FRAMING PLAN
SE102.1

(801) 355-5915

NORTH
1 LOW ROOF FRAMING PLAN
 SE102.1 SCALE: 1/8" = 1'-0"

1

2

3

4

5

6

- ROOF FRAMING PLAN NOTES:**
1. VERIFY ROOF SLOPES, DRAINS, AND DECK BEARING ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
 2. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO COLUMNS, WALLS, SLAB EDGES, SLOPES, ELEVATIONS, CURBS, DEPRESSIONS AND DRAINS.
 3. ALL JOISTS SHALL HAVE 1" DEEP BEARING ENDS UNLESS NOTED OTHERWISE.
 4. ALL GIRDERS SHALL HAVE 1 1/2" DEEP BEARING ENDS UNLESS NOTED OTHERWISE.
 5. OPEN WEB STEEL JOISTS AND JOIST GIRDERS SHALL BE DESIGNATED BY THE MANUFACTURER TO SUPPORT THE MECHANICAL AND LATERAL LOADS SHOWN ON THE ROOF FRAMING PLANS IN ADDITION TO THE UNIFORM AND POINT LOADS SHOWN.
 6. CONTRACTOR SHALL NOT SUPPORT ANY LOADS FROM JOIST BRIDGING.
 7. CONTRACTOR SHALL NOT ATTACH BRACING OR MEP OR FIRE PROTECTION TO BOTTOM CHORDS OF JOISTS OR BRIDGINS.
 8. WHERE SKYLIGHTS OR MECHANICAL UNITS INTERRUPT HORIZONTAL BRIDGINS, PROVIDE CROSS BRIDGINS AT JOIST SPACES ON A SIDE, TYPICAL.
 9. WHERE DIAGONAL BRIDGING CONFLICTS WITH MECHANICAL DUCTS, REMOVE DIAGONAL BRIDGING AND REPLACE WITH HORIZONTAL BRIDGING (AFTER ROOF DECK IS IN PLACE).
 10. * - INDICATES THAT THESE JOISTS SHALL BE DESIGNED FOR AN ADDITIONAL LOAD OF 100#/SF AT ANY TOP CHORD PANEL POINT. THIS LOAD IS TO BE ADDED TO THE GROSS TOP CHORD LOAD MULTIPLE "X" ARE ADDITIVE.
 11. JOIST DESIGNER SHALL DESIGN JOISTS AND SUPPLY ADDITIONAL BRACING AS REQUIRED FOR NET UPLIFT DUE TO WIND (ALLOWABLE), SEE (SE804).
 12. VERIFY SIZE, WEIGHT, AND LOCATION OF ALL ROOF TOP MECHANICAL UNITS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS. COORDINATE OPENINGS WITH MECHANICAL, ELECTRICAL, AND GENERAL CONTRACTORS.
 13. SPRINKLER DESIGNER SHALL COORDINATE THE SPRINKLER DESIGN WITH THE JOIST GIRDER SHOP DRAWINGS.
 14. ALL JOIST TOP CHORDS BEARING AT THE MASONRY WALLS SHALL BE DESIGNED FOR A MINIMUM AXIAL LOAD OF 10 KIPS IN TENSION AND COMPRESSION THAT REDUCES TO 0.0 KIPS AT THE OPPOSITE END OF THE JOIST. LOADS ARE DUE TO SEISMIC AND ARE PROVIDED AT ULTIMATE STRENGTH LEVEL INCLUDING OVERSTRENGTH FACTOR OF 1.4. JOIST SUPPLIER TO DESIGN JOISTS TO TRANSFER THIS ANCHORAGE LOAD THROUGH THE JOIST BEARING SEAT.
 15. SPRINKLER DESIGNER SHALL COORDINATE:
 - STEEL COLUMN
 - CONCRETE AND MASONRY LINTELS
 17. SEE TYPICAL FRAMING DETAILS ON (SE701) SHEETS FOR:
 - FRAMING AT ROOF OPENINGS, MECHANICAL UNITS, SKYLIGHTS AND ROOF DRAINS.
 - PIPE SLEEVES THROUGH ROOF DECK
 - SUSPENDED LOADS ON ROOF DECK
 - JOIST REINFORCING AT POINT LOADS
 - JOIST TOP CHORD TIE

MARKS AND SYMBOLS LEGEND

- SECTION MARK SHEET NUMBER
- FRAME ELEVATION SHEET NUMBER
- MTL ROOF DECK, SEE GSN
- CONCRETE OVER METAL DECK, SEE GENERAL STRUCTURAL NOTES
- MASONRY WALL
- BRG WALL BLW
- STEEL COLUMN
- JOIST TOP CHORD TIE
- CL-x CONCRETE LINTEL, SEE SCHEDULE
- ML-x MASONRY LINTEL, SEE SCHEDULE
- T-C=XX ROOF JOIST TOP CHORD SHALL BE DESIGNED FOR AN ADDITIONAL LOAD OF X KIPS IN TENSION AND COMPRESSION. LOADS ARE PROVIDED AT ULTIMATE STRENGTH LEVEL.
- RD ROOF DRAIN, SEE IS / SE701.1 AND SEE ARCHITECTURAL AND MECHANICAL FOR EXACT LOCATION
- LATERAL FRAME MOMENT CONNECTIONS
- FRAME OR COLLECTOR BRACE, SEE DET (15) / SE703.1

SNOW DRIFT LOADING DIAGRAM

D-1	MAX = 33 PSF L = 7'-0"	D-2	MAX = 63 PSF L = 22'-2"
D-3	MAX = 57 PSF L = 24'-2"		

MAX PSF
D-x
UNIFORM ROOF SNOW LOAD, SEE GSN

WHERE 'L' EXCEEDS LENGTH OF LOWER ROOF, DRIFT TAPERS TO 0 PSF AT THE FAR END OF LOWER ROOF. SHADED AREA ON PLAN DENOTES EXTENT OF SNOW DRIFT

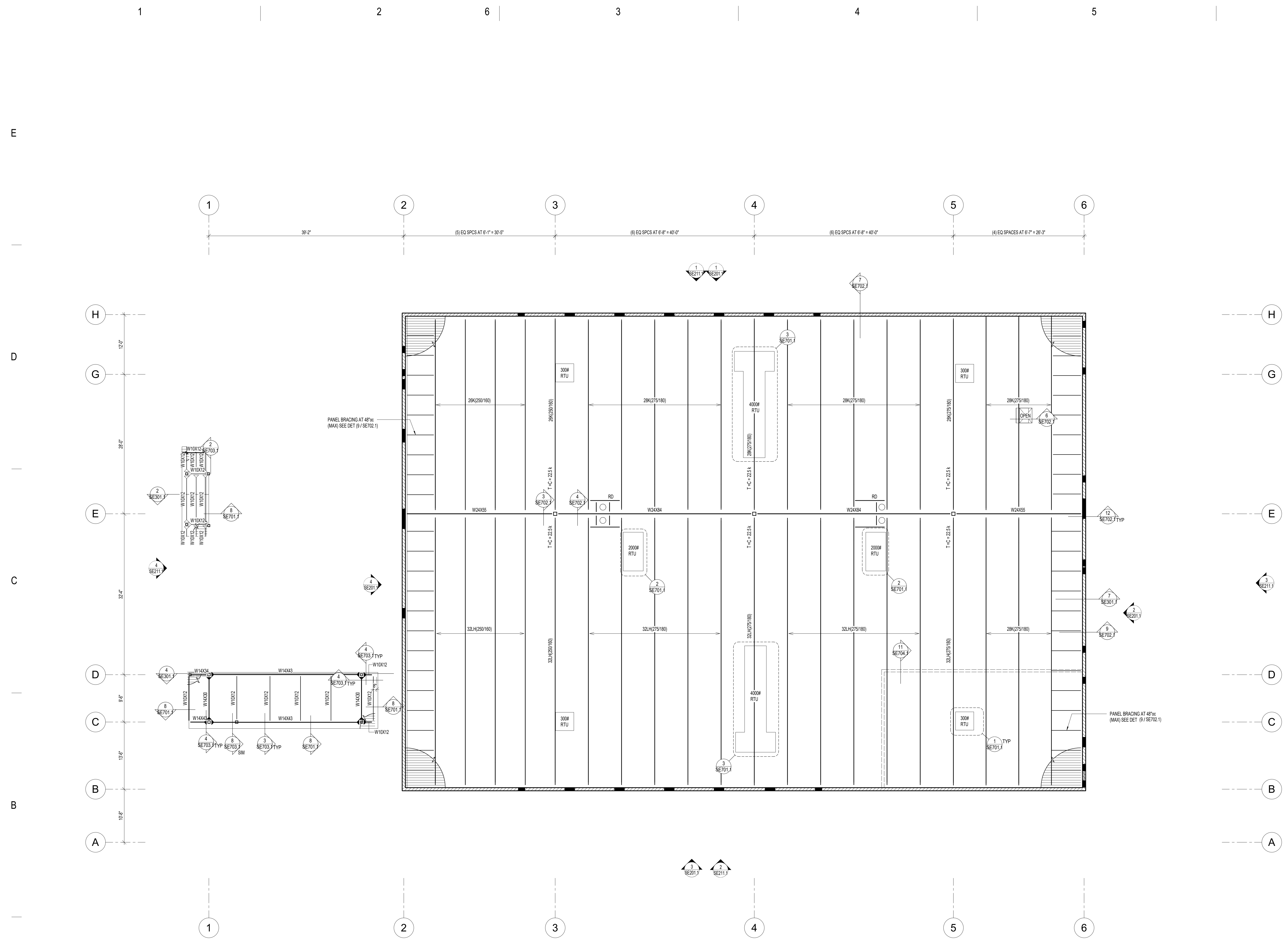
DTC WELDING TECH & FABRICATION
BUILDING
355 SOUTH 650 EAST
KAYSVILLE, UT 84037



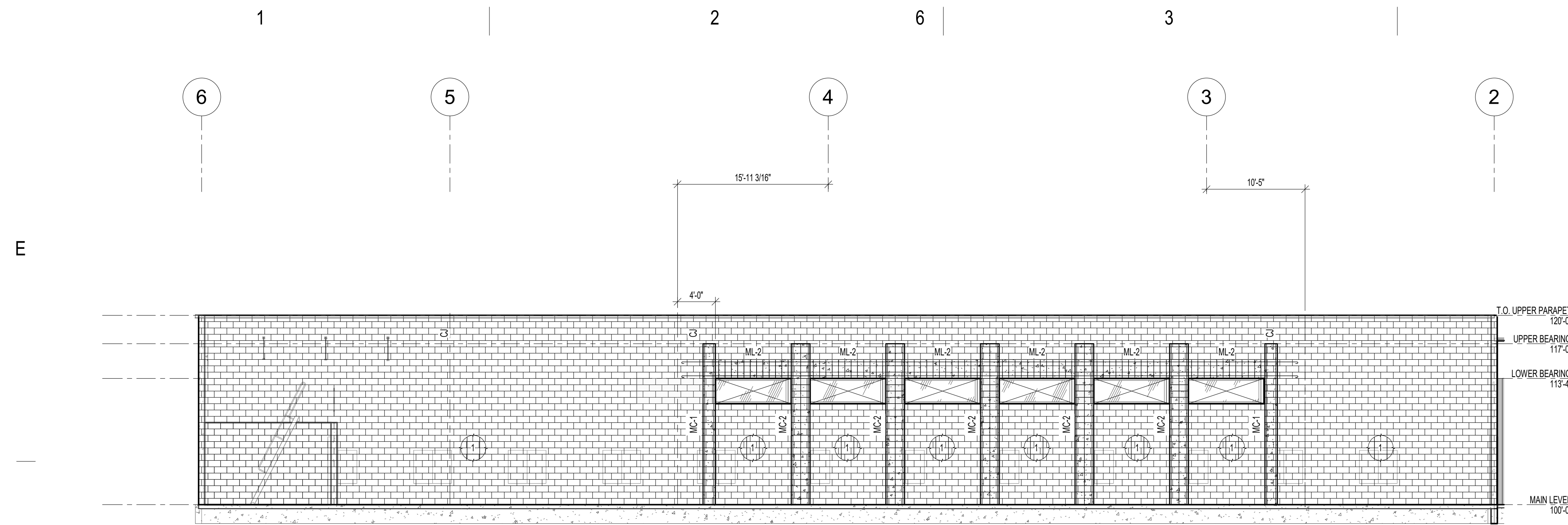
2024-08-26
BID PACKAGE #1

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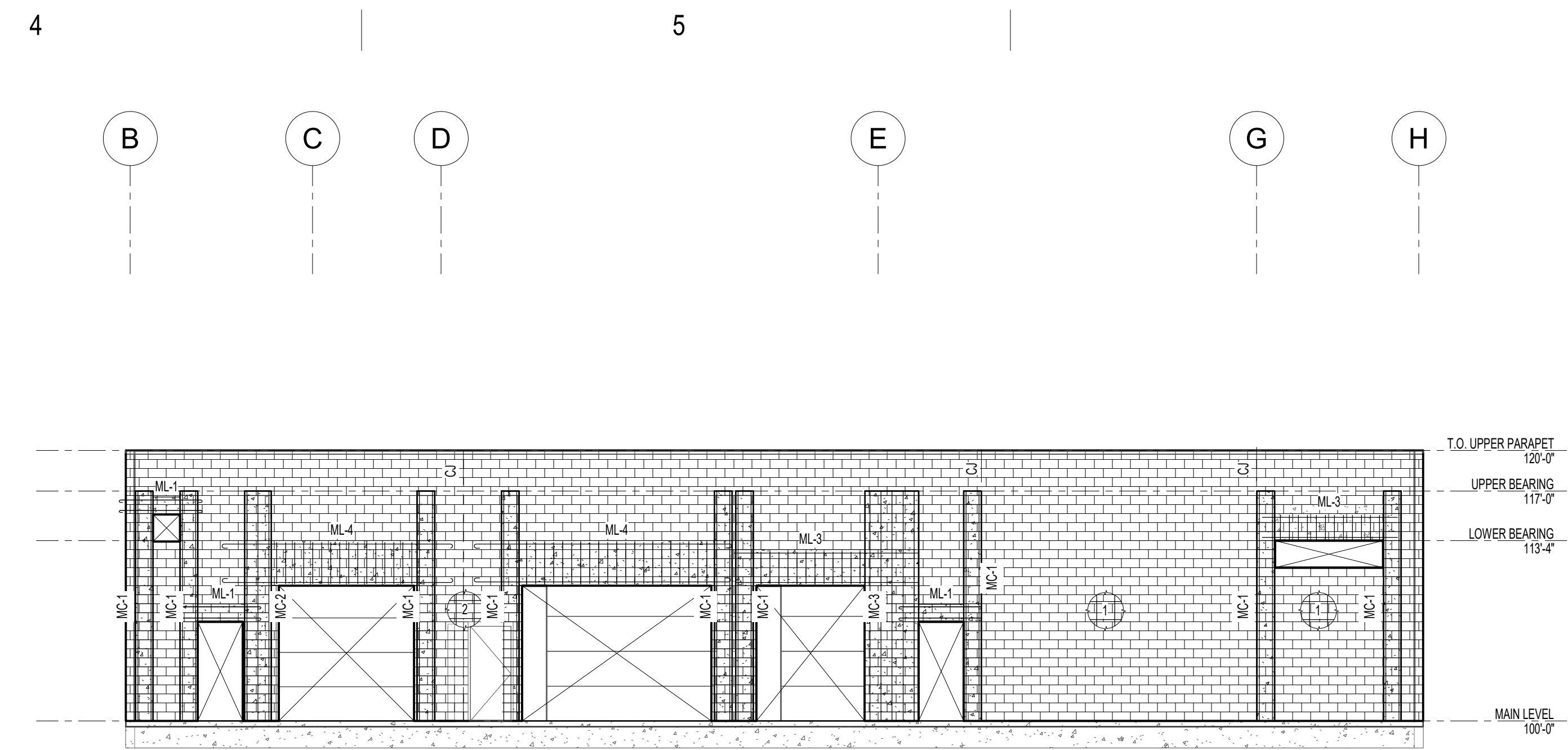
HIGH ROOF FRAMING PLAN
SE103.1



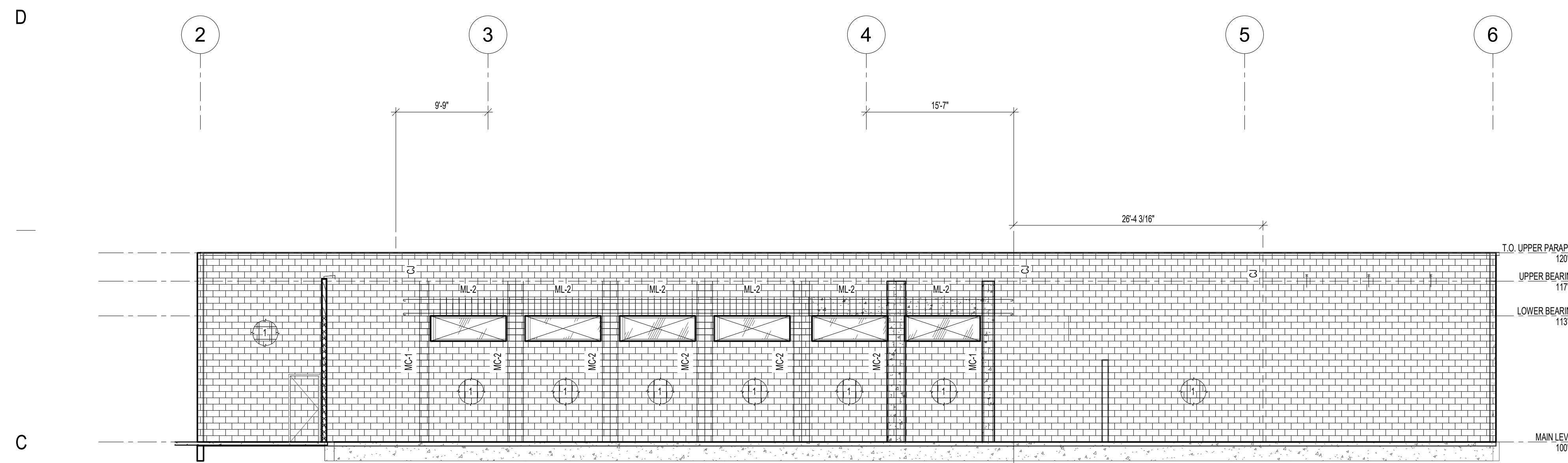
1 HIGH ROOF FRAMING PLAN
SE103.1 SCALE: 1/8" = 1'-0"



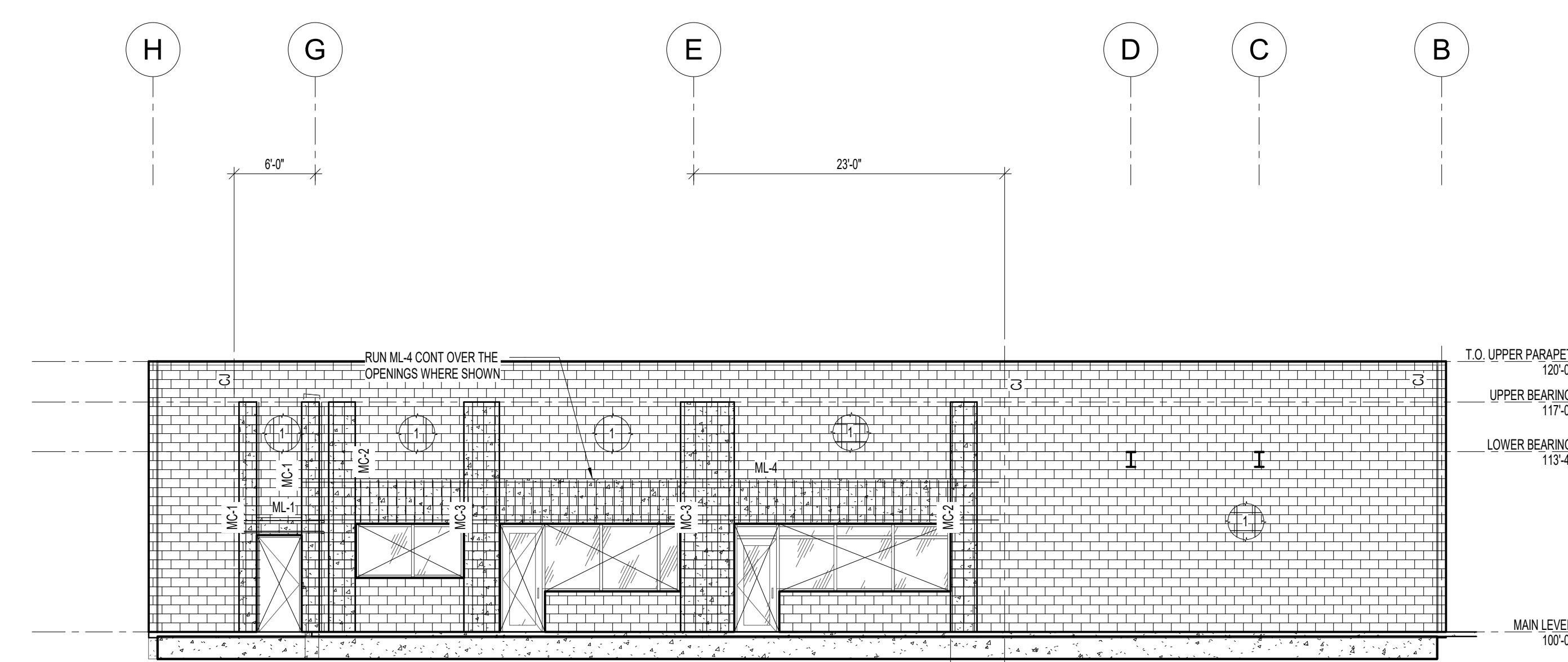
1 MASONRY WALL REINFORCING ELEVATION
SE201.1 SCALE: 1/8" = 1'-0"



2 MASONRY WALL REINFORCING ELEVATION
SE201.1 SCALE: 1/8" = 1'-0"



3 MASONRY WALL REINFORCING ELEVATION
SE201.1 SCALE: 1/8" = 1'-0"



4 MASONRY WALL REINFORCING ELEVATION
SE201.1 SCALE: 1/8" = 1'-0"

The field CMU will from from the bottom of the wall to the bottom sill of the window band and will be don with 8" Split Faced integrally colored (Sunroc - Tumbleweed).
An accent band will go from the bottom sill of the window band to the top of the wall and will be done with 8" Smooth Faced CMU integrally colored (Sunroc - Cream).
The brick will 4x4x16 Emperor brick (Interstate Brick - Midnight Black, or approved equal).
Please refer to the attached revised rendering (The first page in this bid package).

Or approved Equal

Please refer to the attached revised rendering (The first page in this bid package).

MASONRY WALL REINFORCING		
WALL TYPE	VERTICAL REINFORCING	HORIZONTAL REINFORCING
1	(1) #5 AT 32"oc	(1) #5 AT 32"oc
2	(1) #6 AT 16"oc	(1) #5 AT 16"oc

	MASONRY WALL REINFORCING TYPE
	SECTION MARK SHEET NUMBER
ML-x	MASONRY LINTEL, SEE SCHEDULE
MC-x	MASONRY COLUMN, SEE SCHEDULE
CJ	CONTROL JOINT

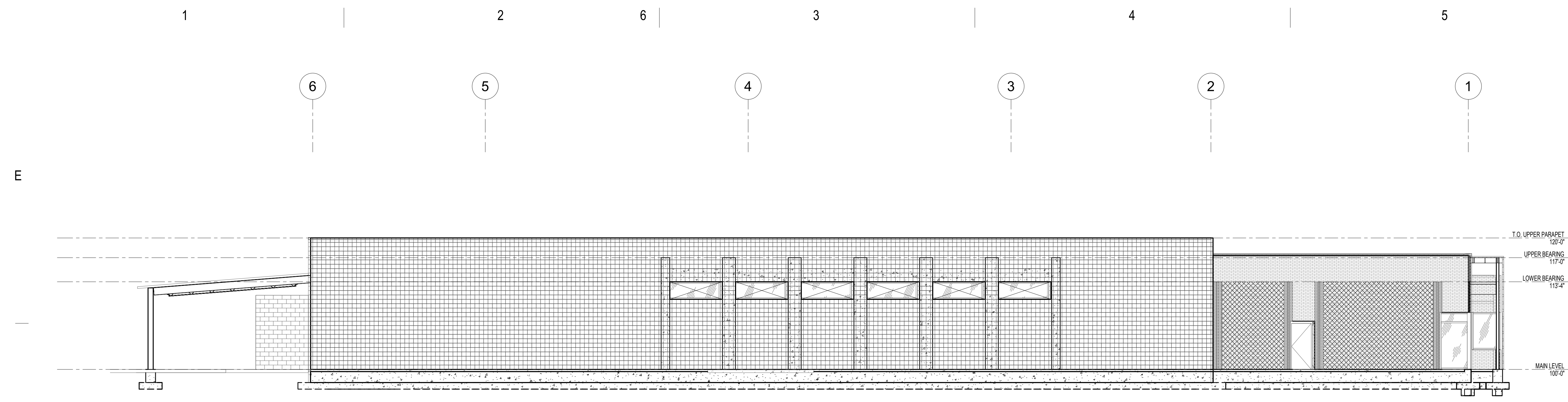
DTC WELDING TECH & FABRICATION
BUILDING
 355 SOUTH 650 EAST
 KAYSVILLE, UT 84037



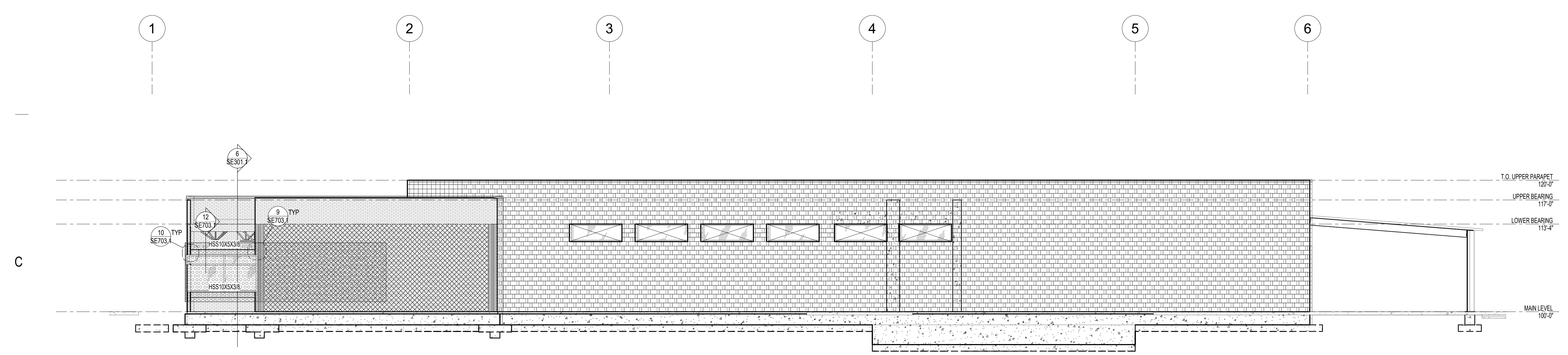
2024-08-26
BID PACKAGE #1

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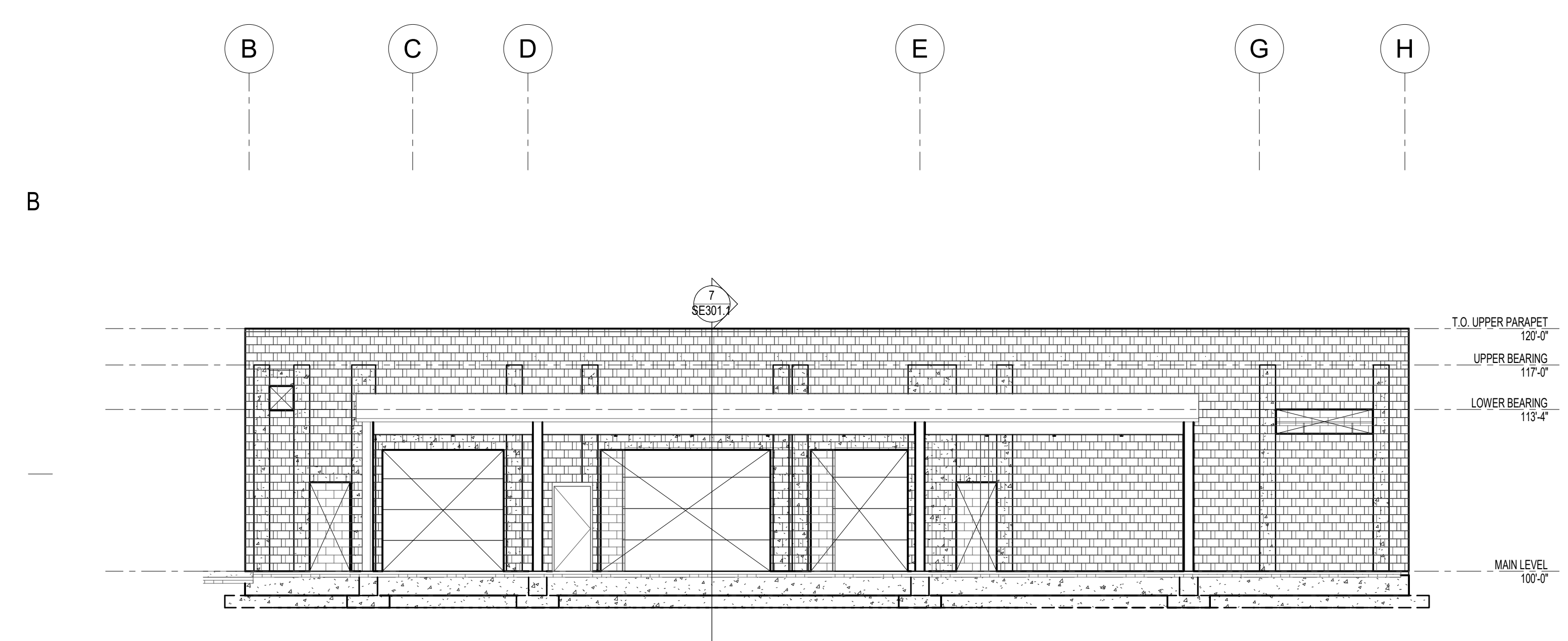
Please refer to the attached revised rendering (The first page in this bid package)



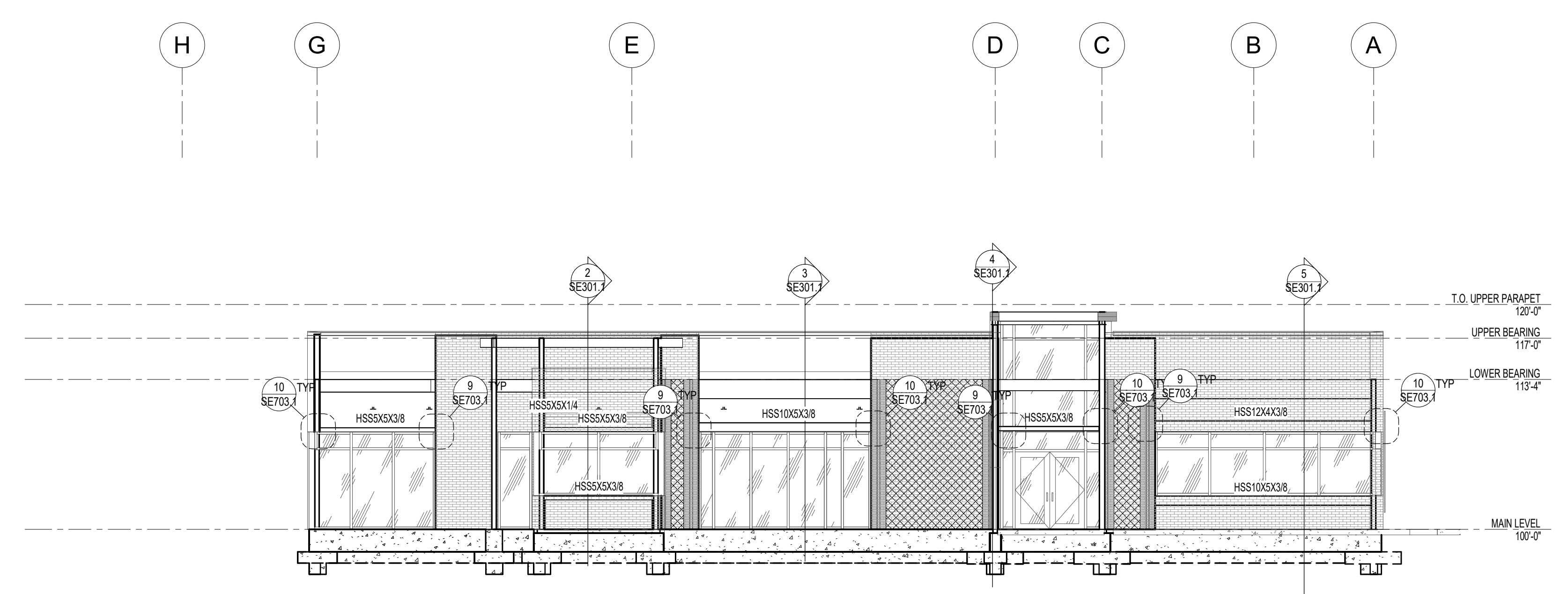
1 BUILDING ELEVATION
SE211.1 SCALE: 1/8" = 1'-0"



2 BUILDING ELEVATION
SE211.1 SCALE: 1/8" = 1'-0"



3 BUILDING ELEVATION
SE211.1 SCALE: 1/8" = 1'-0"



4 BUILDING ELEVATION
SE211.1 SCALE: 1/8" = 1'-0"

The field CMU will from from the bottom of the wall to the bottom sill of the window band and will be don with 8" Split Faced integrally colored (Sunroc - Tumbleweed).
An accent band will go from the bottom sill of the window band to the top of the wall and will be done with 8" Smooth Faced CMU integrally colored (Sunroc - Cream).
The brick will 4x4x16 Emperor brick (Interstate Brick - Midnight Black)
Please refer to the attached revised rendering (The first page in this bid package).

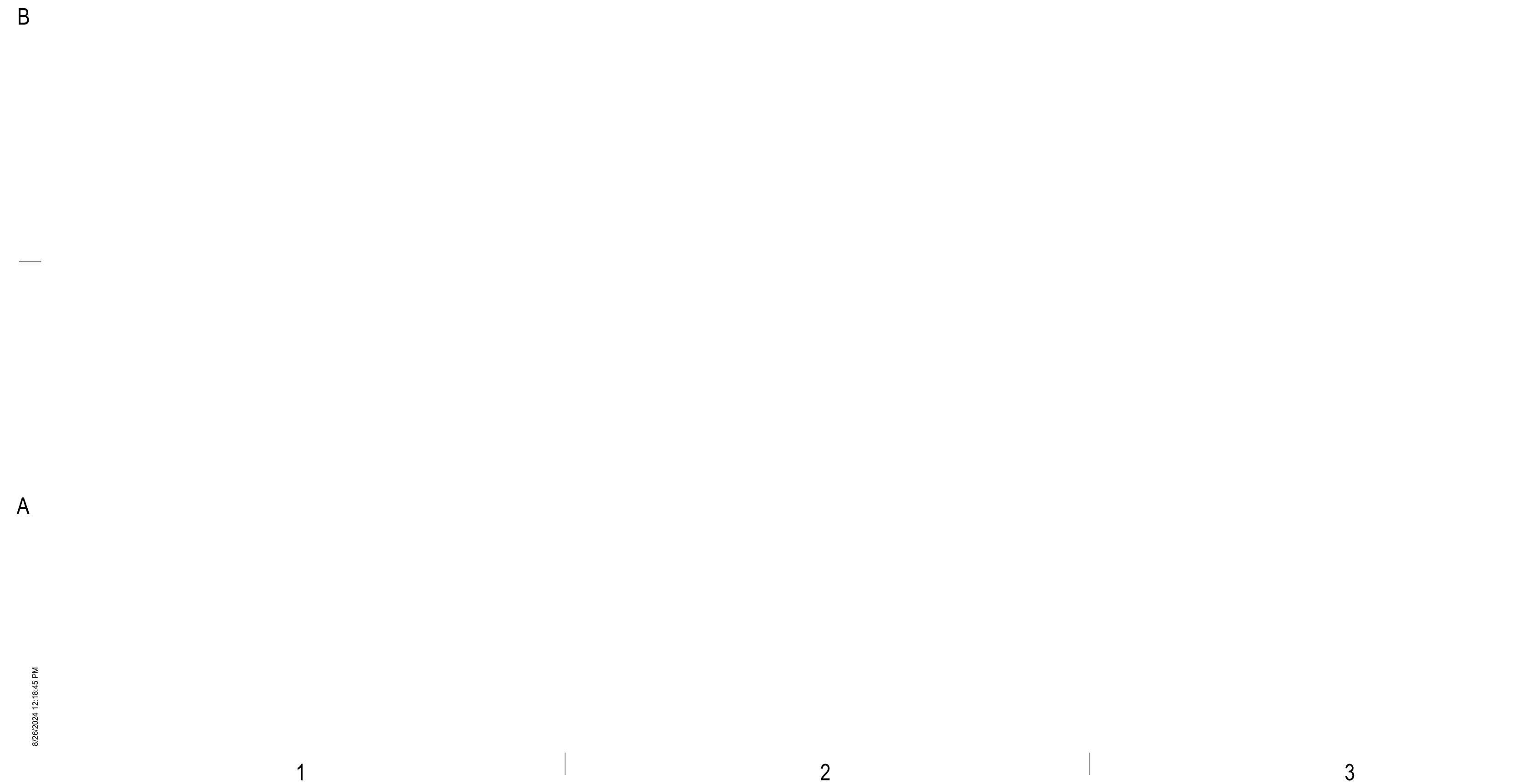
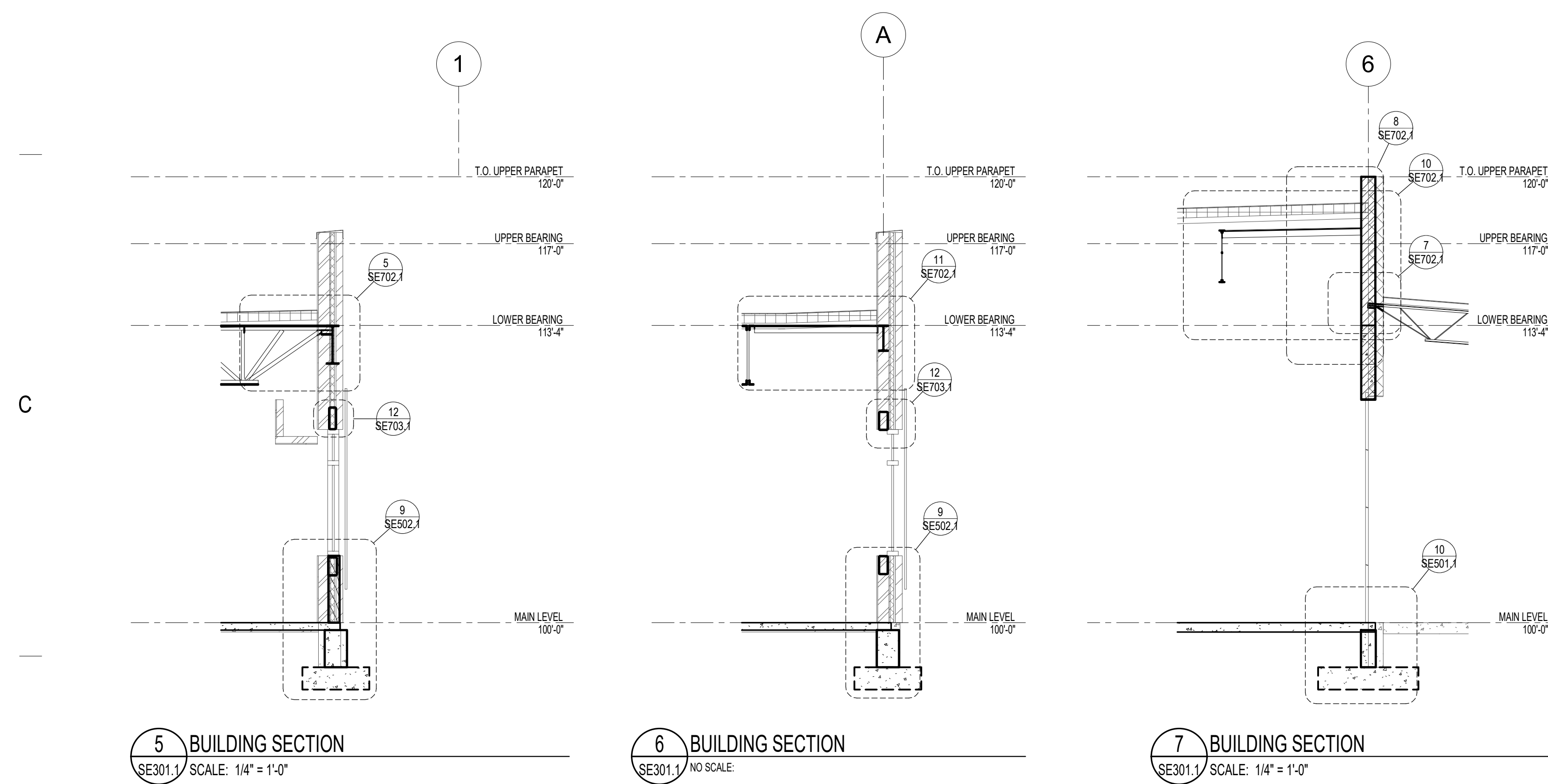
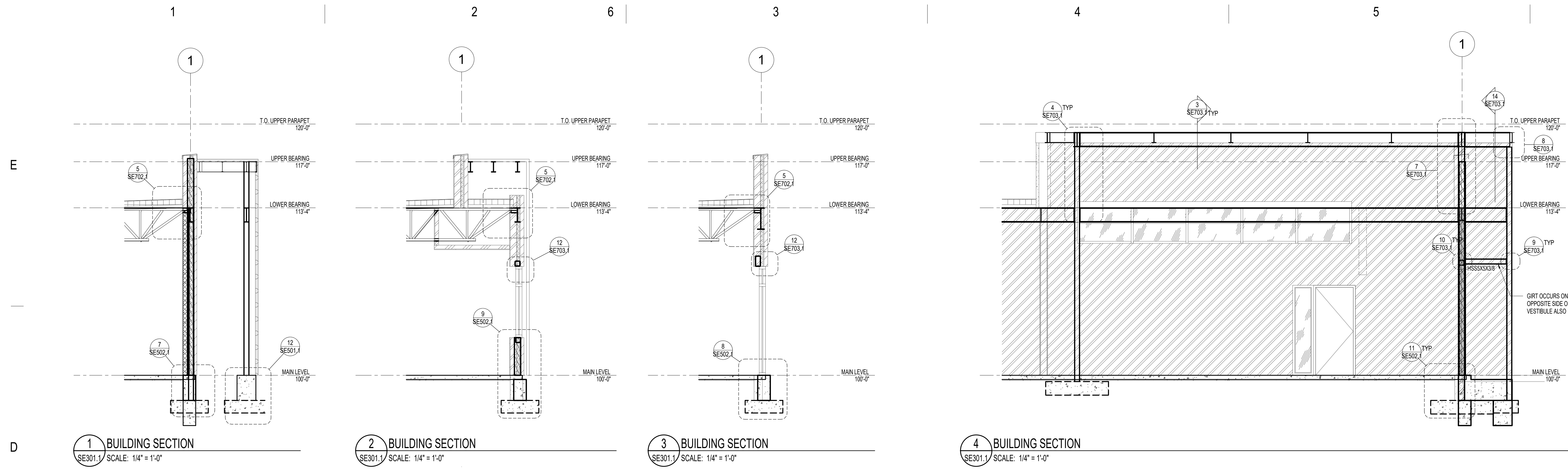
DTC WELDING TECH & FABRICATION
BUILDING
355 SOUTH 650 EAST
KAYSVILLE, UT 84037



2024-08-26
BID PACKAGE #1

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BUILDING ELEVATIONS
SE211.1



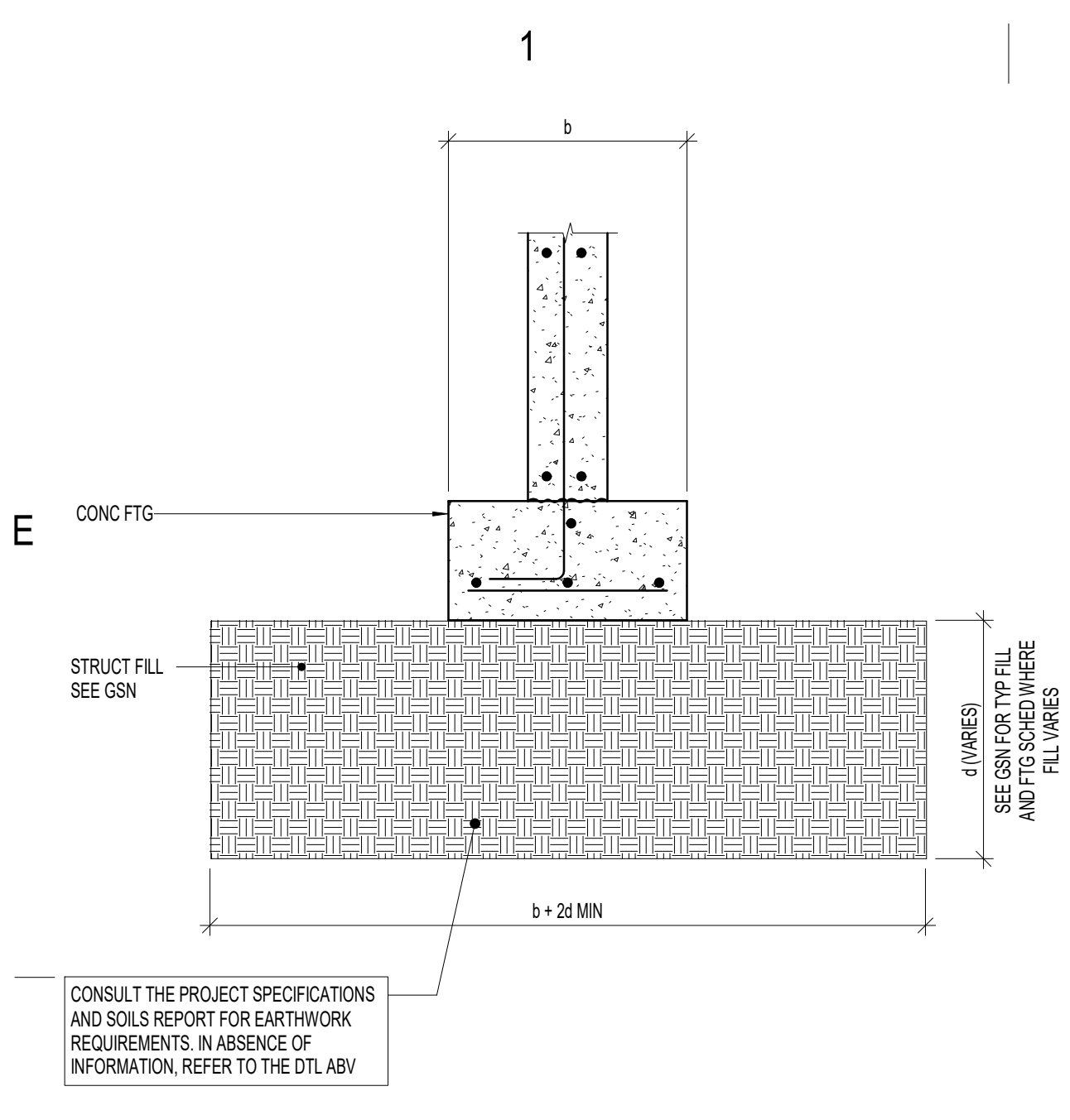
DTC WELDING TECH & FABRICATION
BUILDING
355 SOUTH 650 EAST
KAYSVILLE, UT 84037



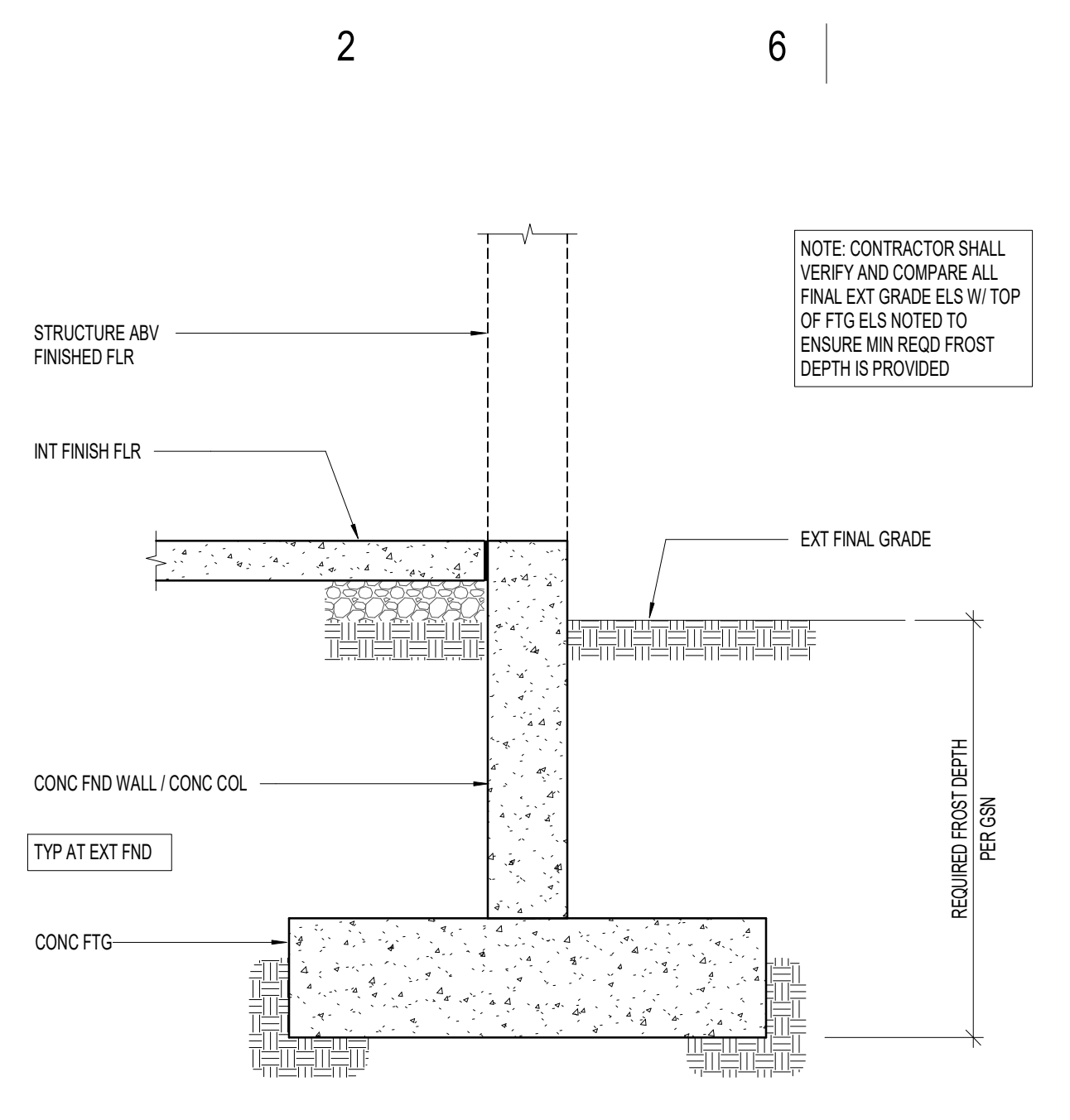
2024-08-26
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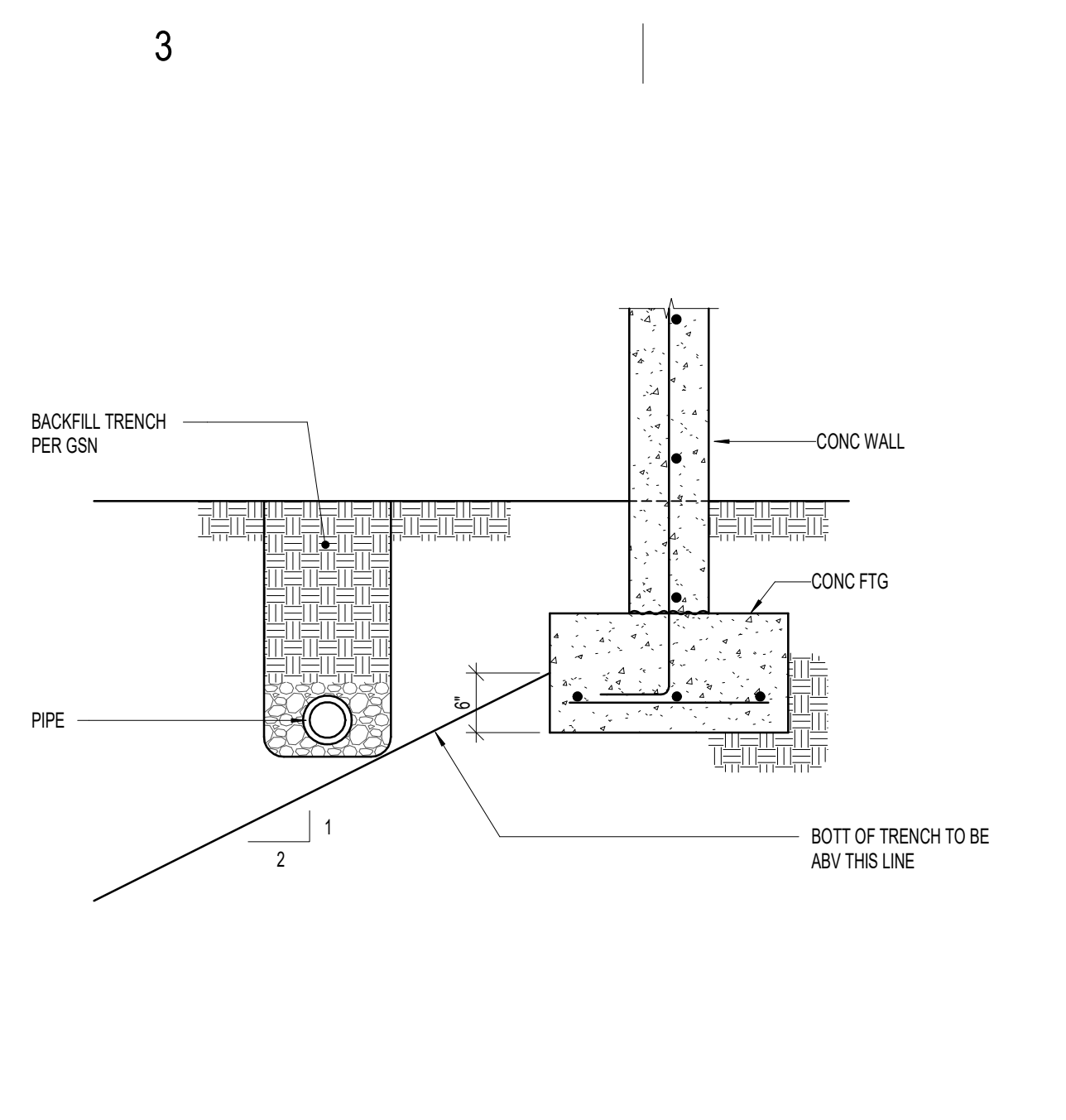
BUILDING SECTIONS
SE301.1



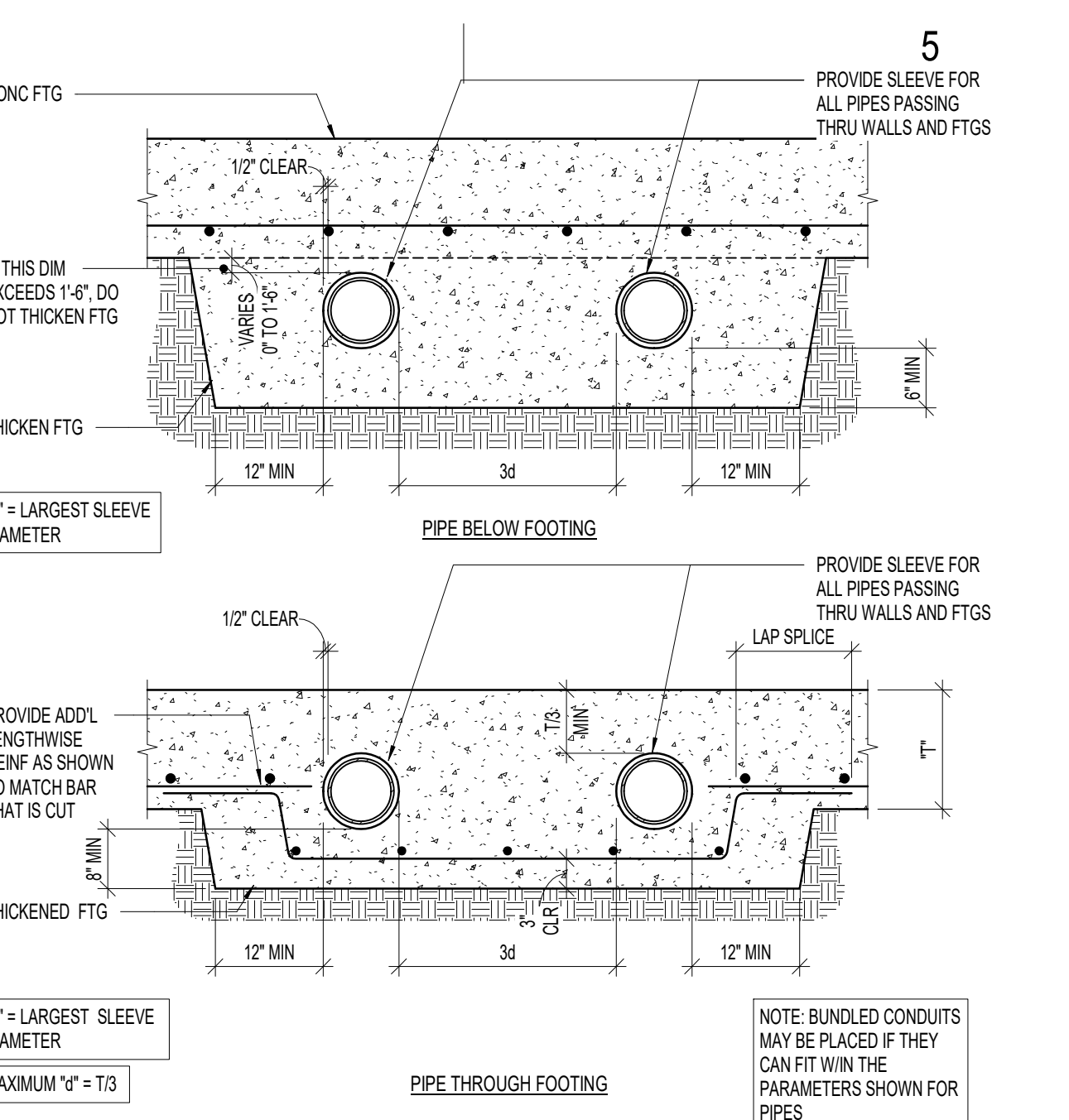
1 TYPICAL COMPACTED STRUCTURAL FILL
 SE501.1 NO SCALE
 10001



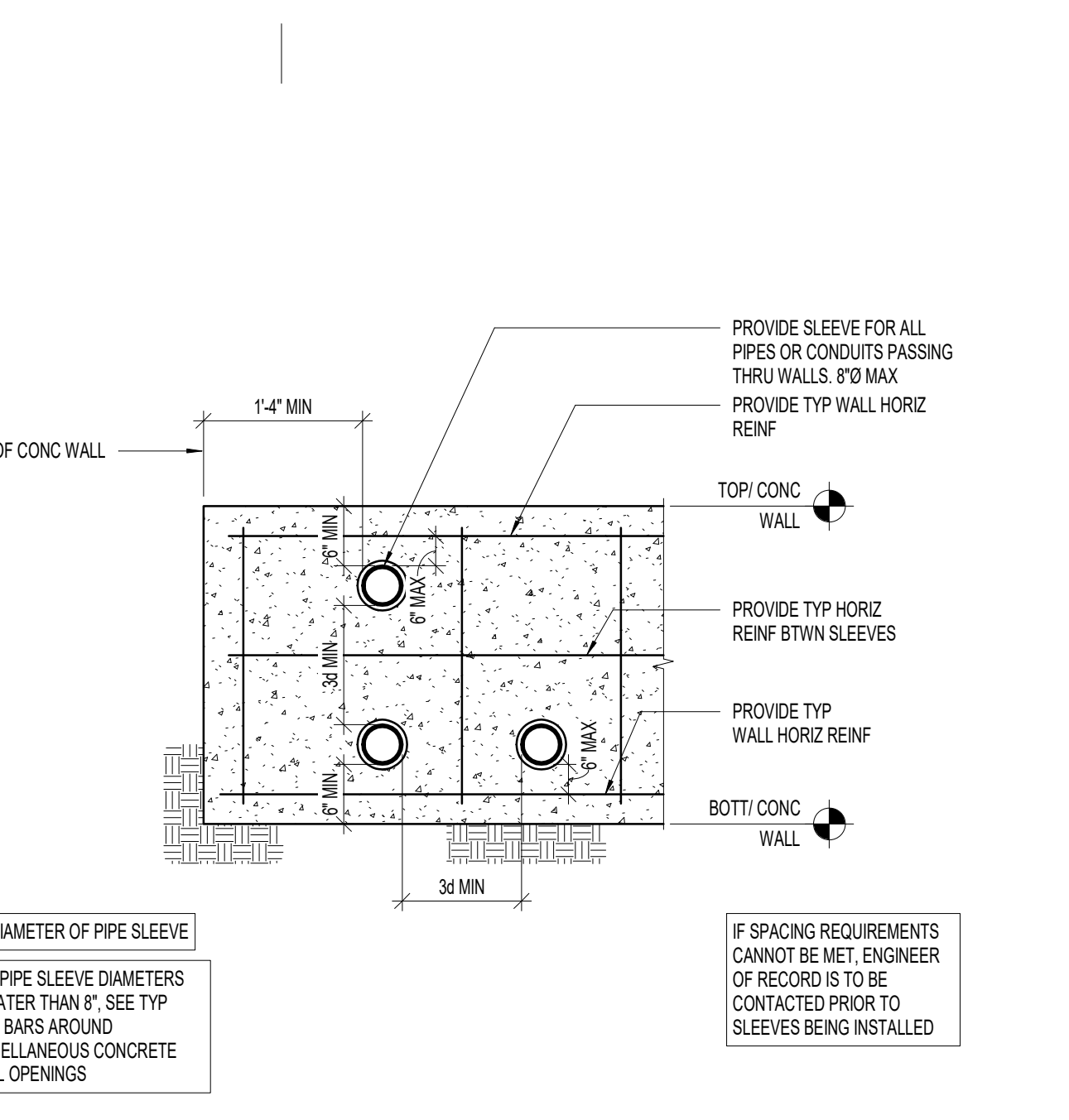
2 TYPICAL FOOTING DEPTH DETAIL FOR FROST PROTECTION
 SE501.1 NO SCALE
 10002



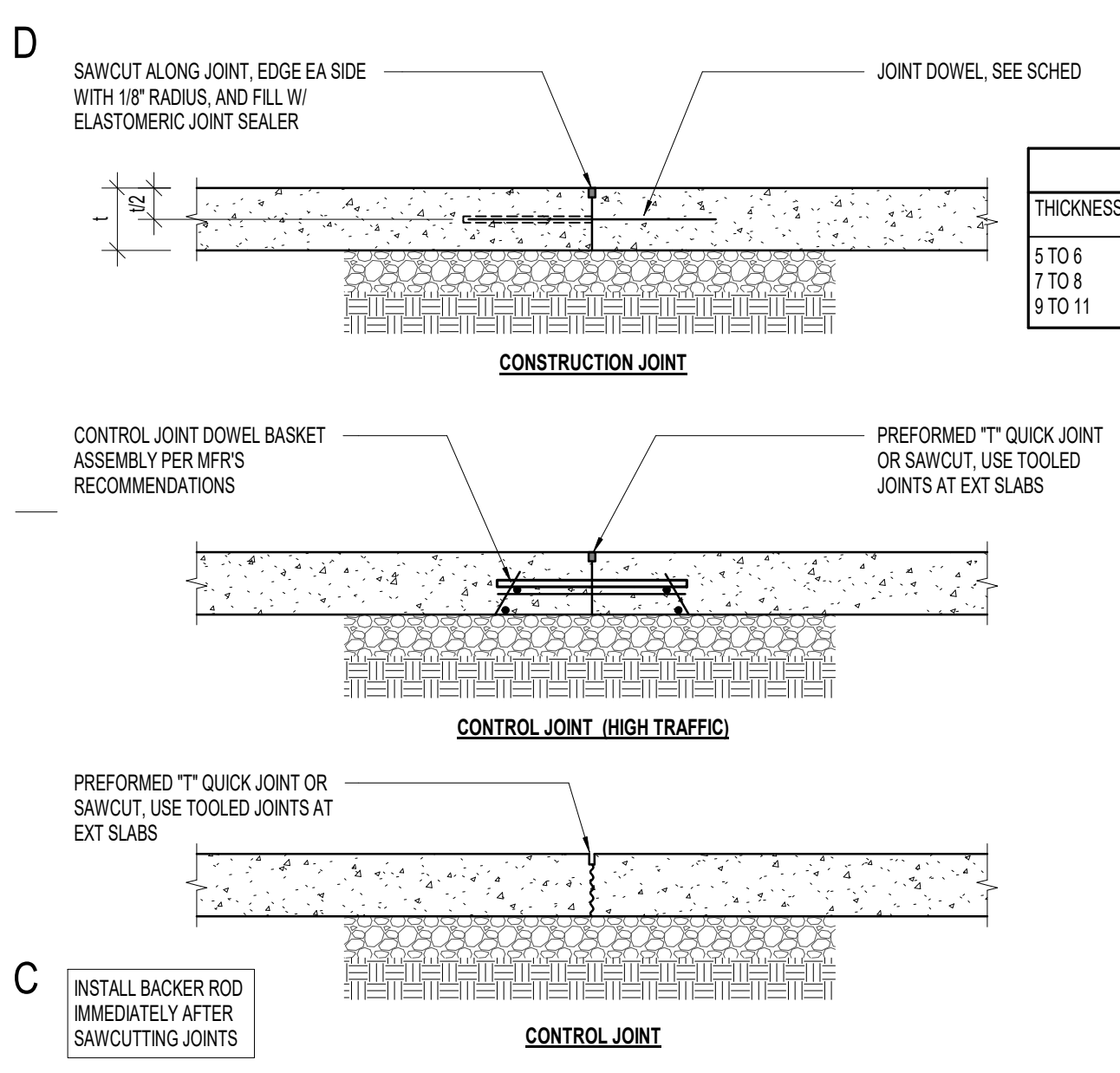
3 TYPICAL PIPE PARALLEL TO FOOTING
 SE501.1 NO SCALE
 10003



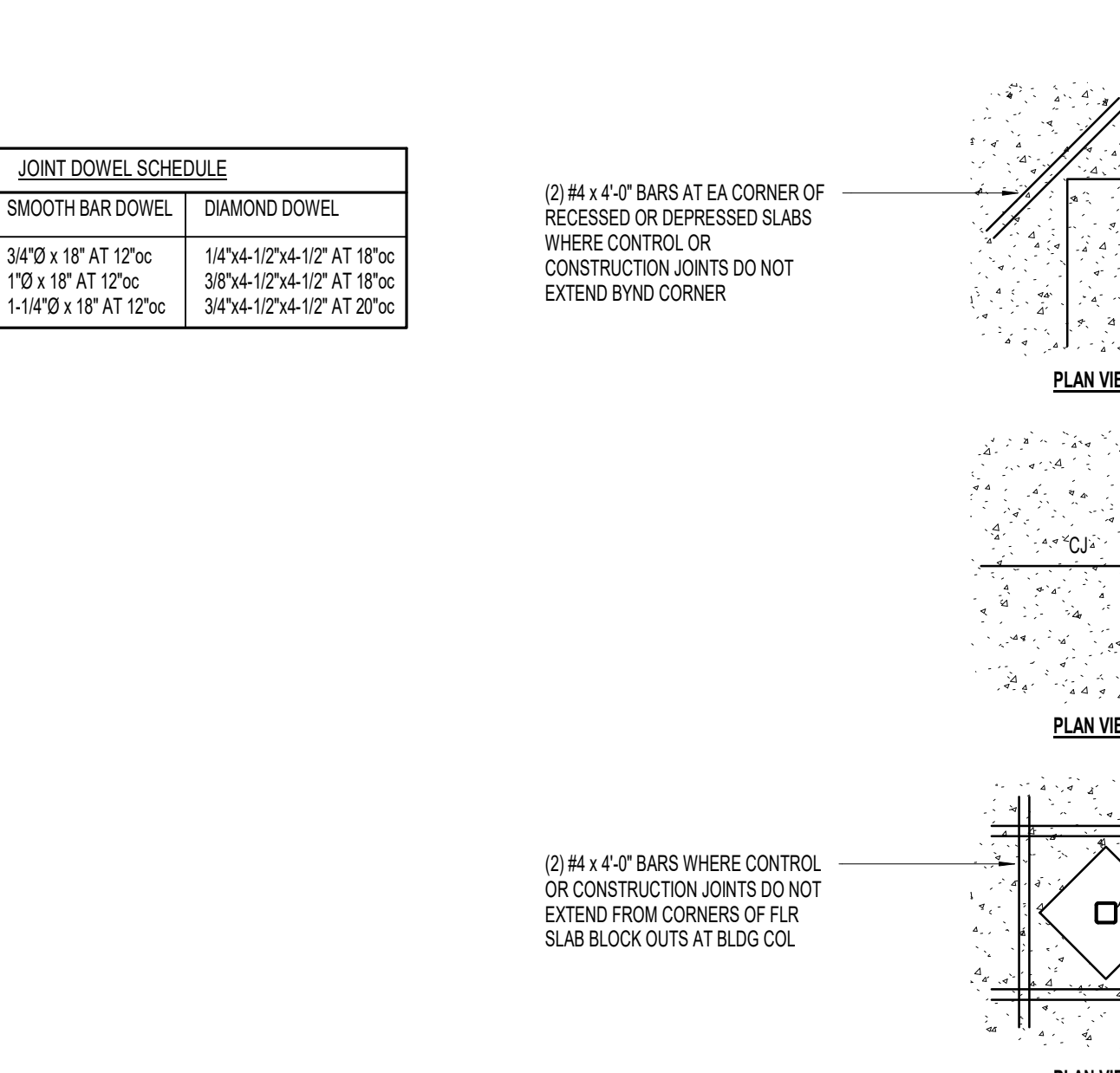
4 TYPICAL PIPE PERPENDICULAR TO FOOTING
 SE501.1 NO SCALE
 10004



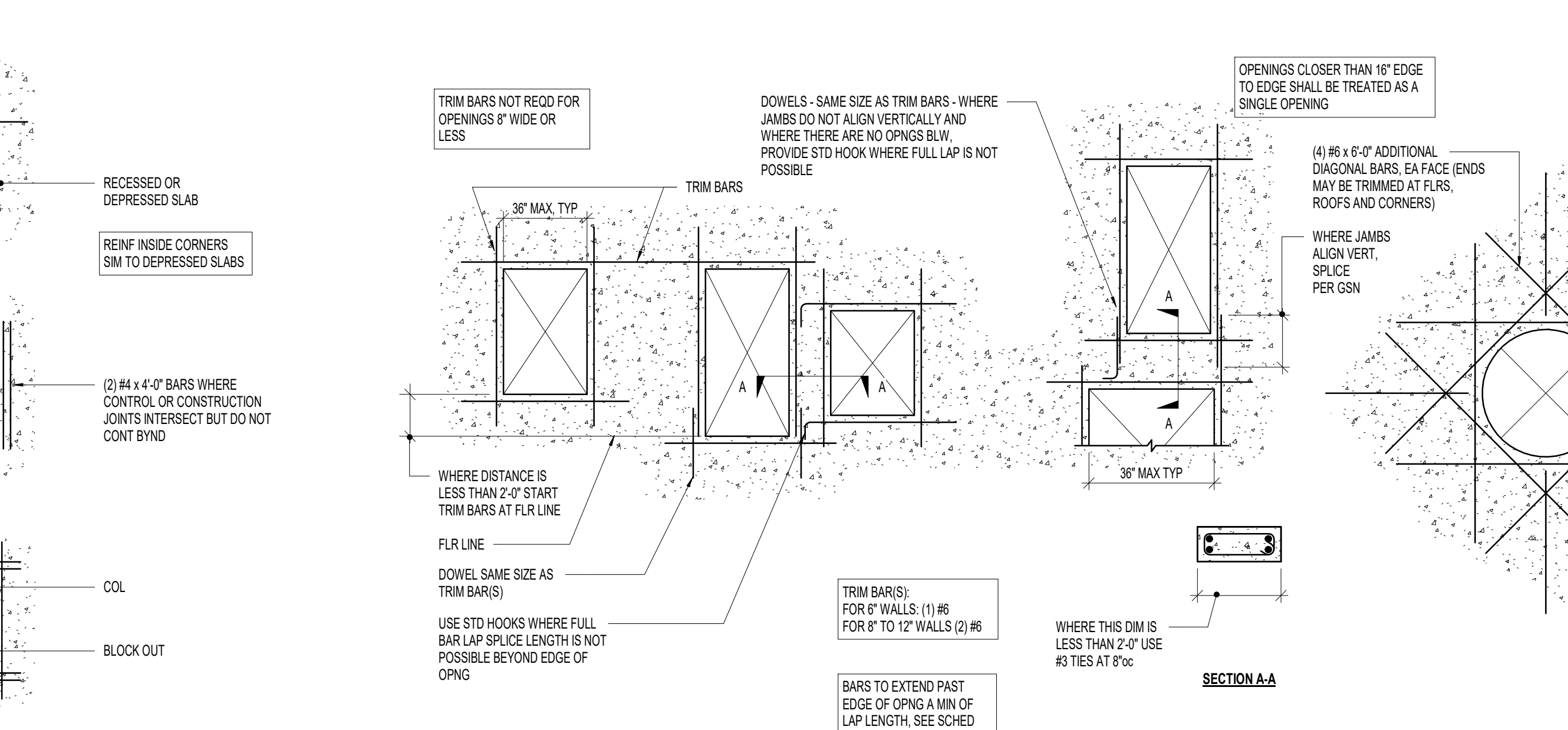
5 TYPICAL SMALL PIPE OR CONDUIT THROUGH CONCRETE WALL
 SE501.1 NO SCALE
 10005



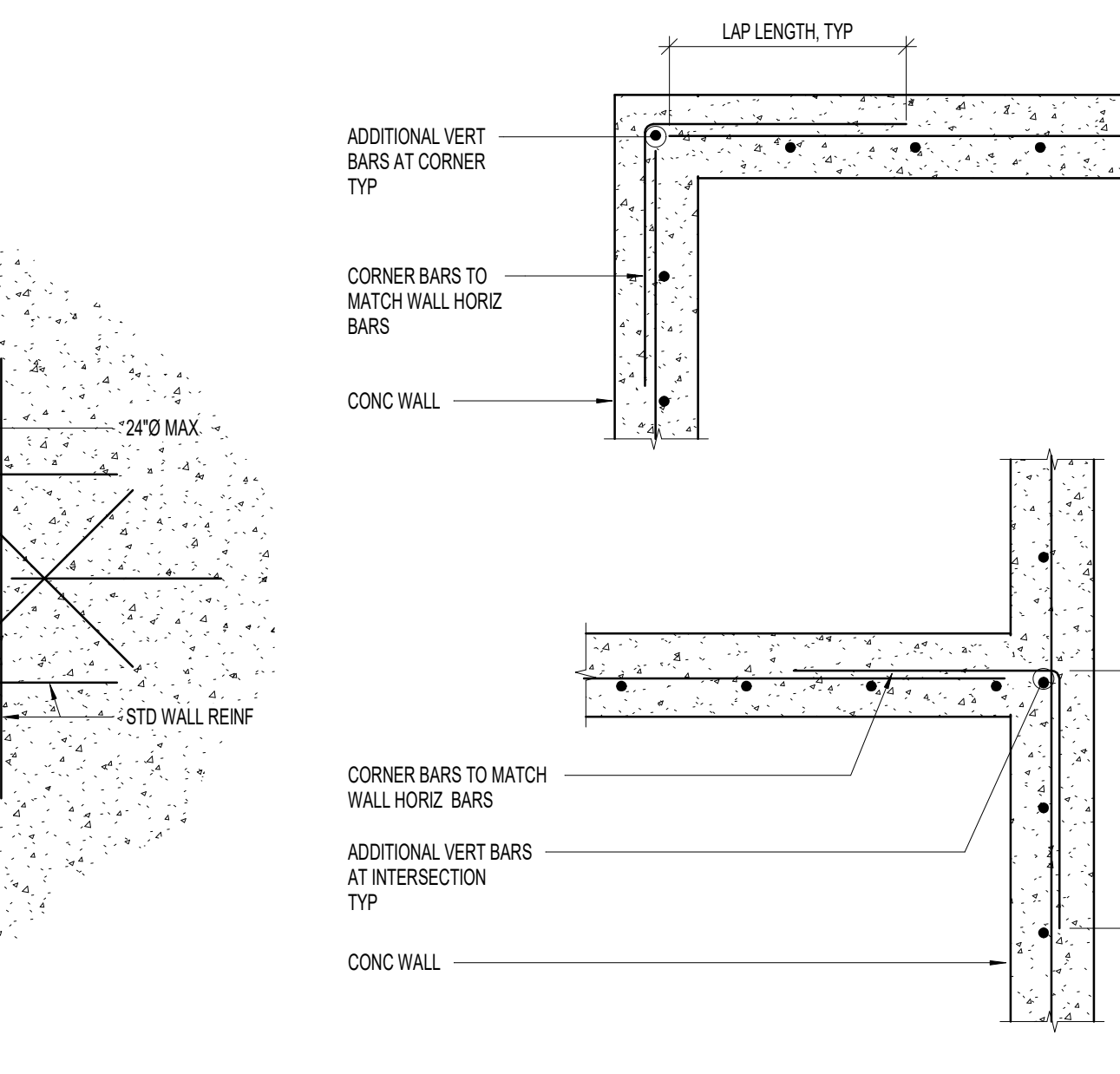
6 TYPICAL UNREINFORCED STRUCTURAL SLAB ON GRADE JOINTS
 SE501.1 NO SCALE
 10006



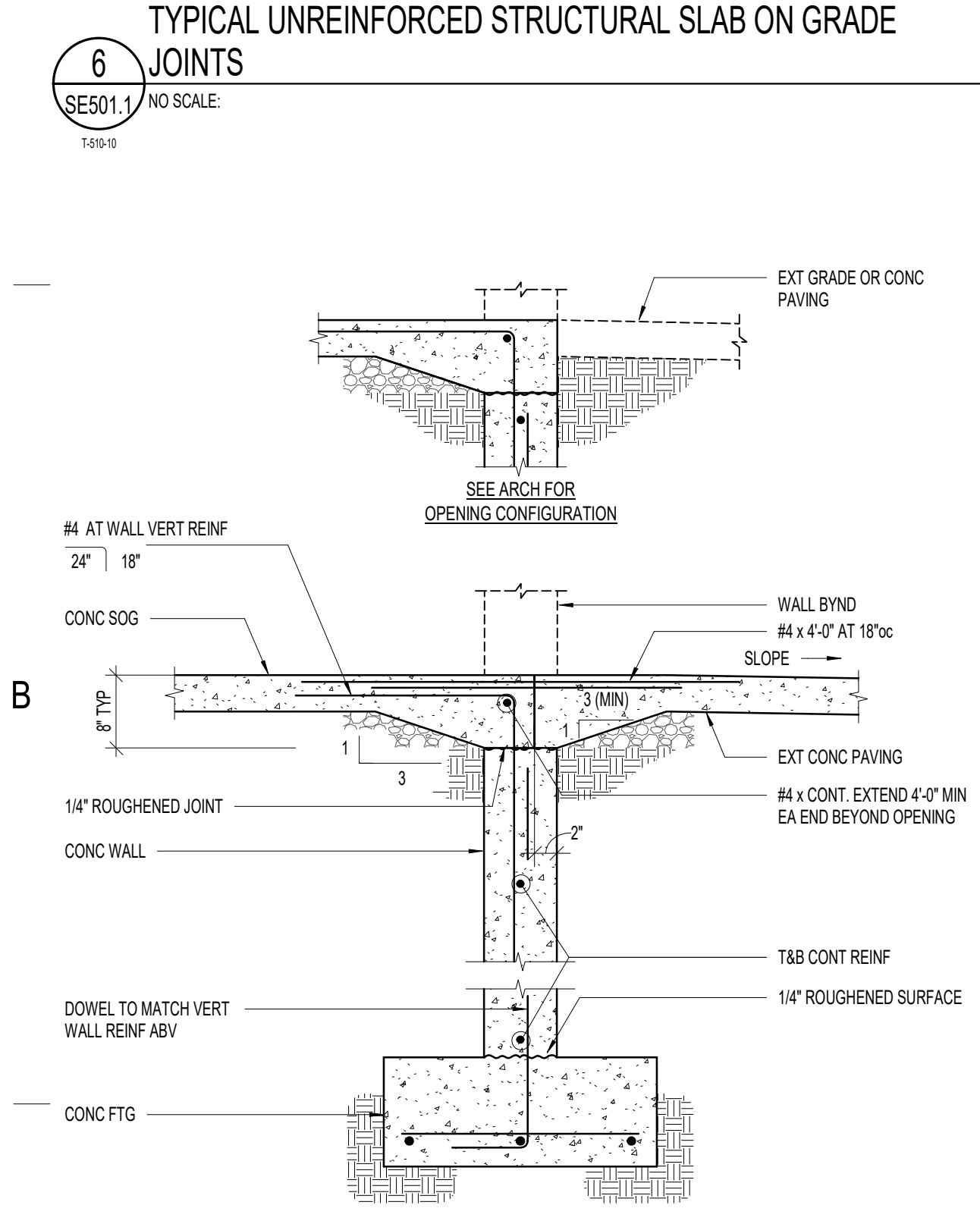
7 TYPICAL SLAB ON GRADE DISCONTINUITIES REQUIRING ADDITIONAL REINFORCING
 SE501.1 NO SCALE
 10007



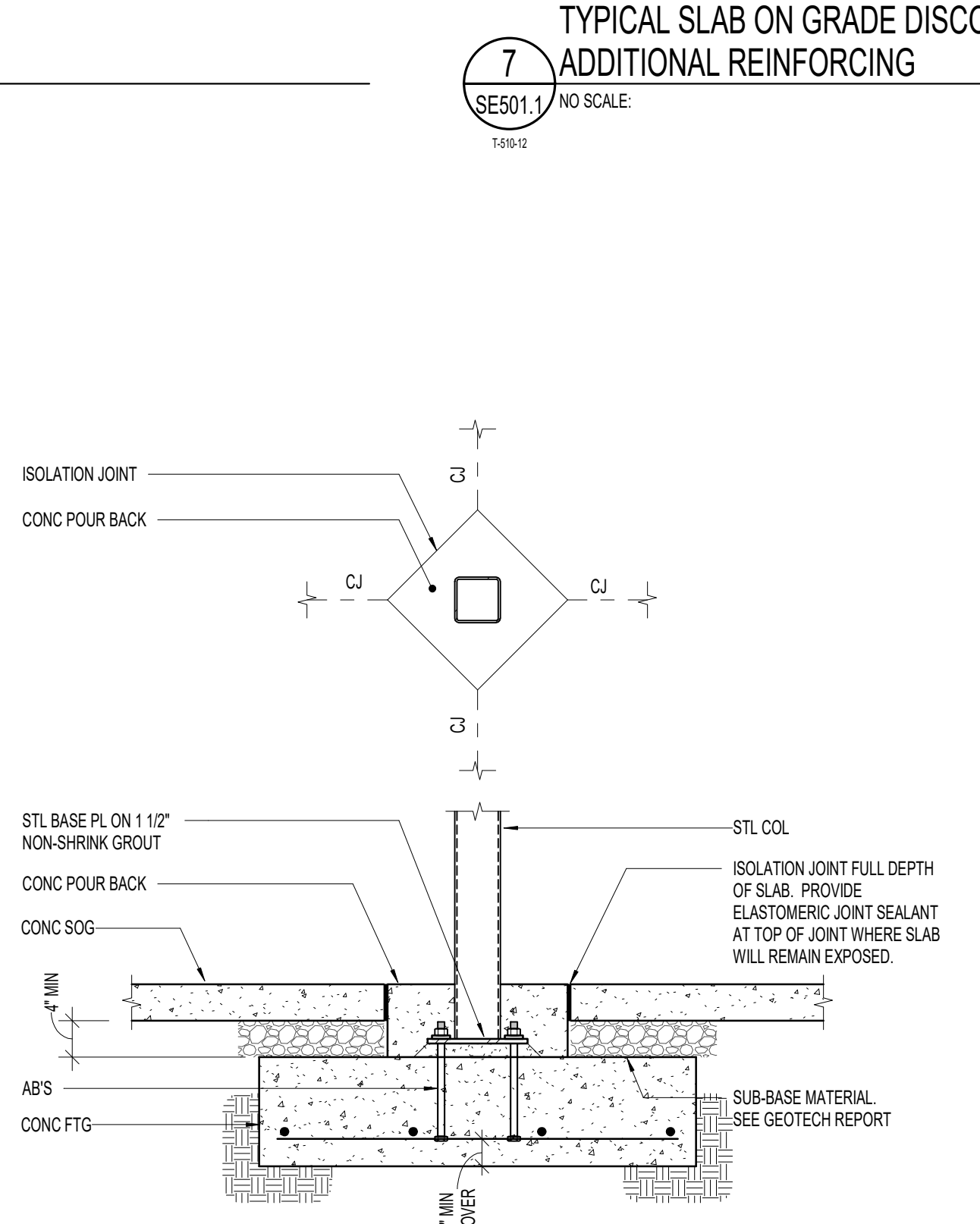
8 TYPICAL TRIM BARS AROUND MISCELLANEOUS CONCRETE WALL OPENINGS UNLESS NOTED OTHERWISE
 SE501.1 NO SCALE
 10008



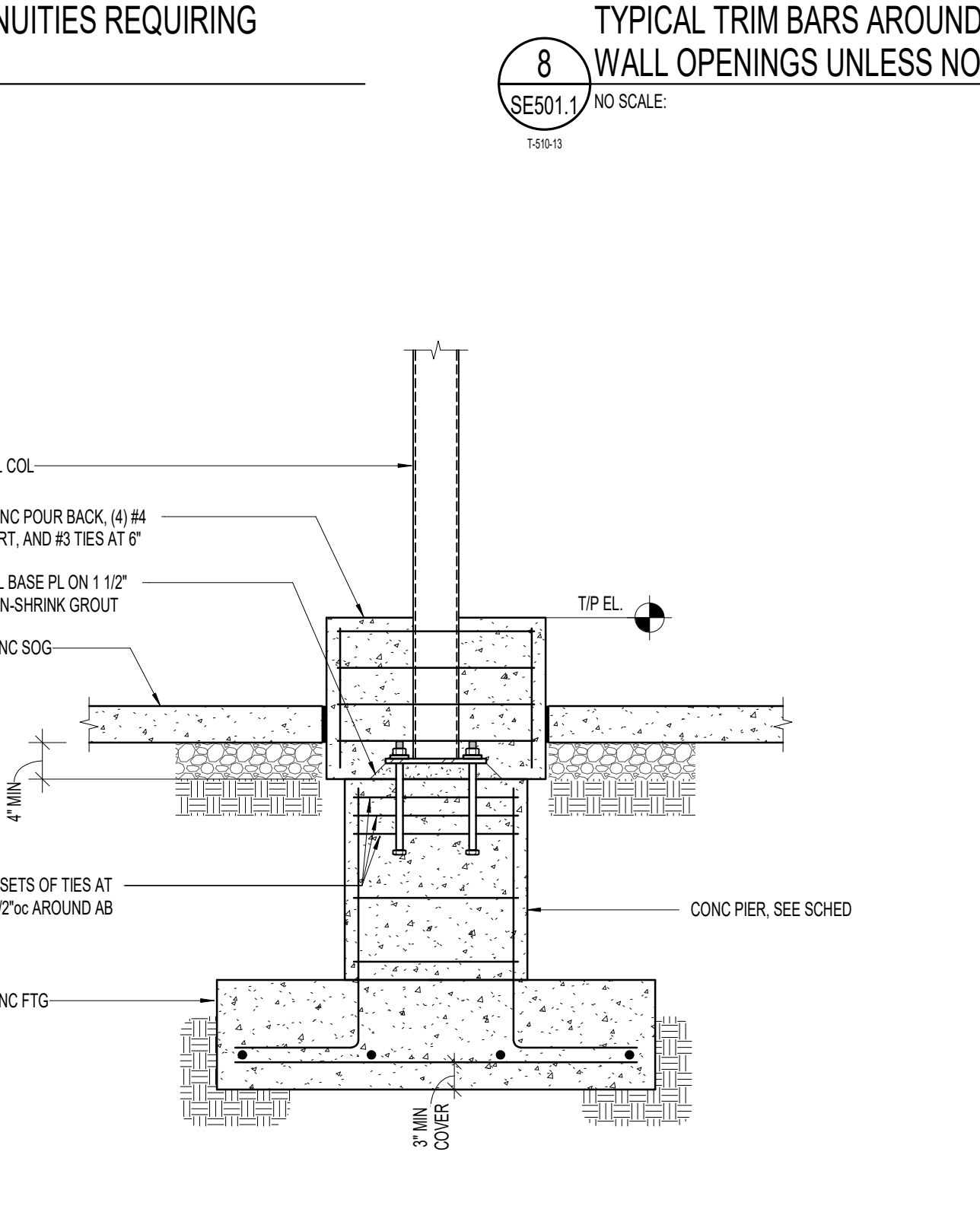
9 TYPICAL CORNER BARS FOR SINGLE REINFORCED CONCRETE WALLS (PLAN VIEW)
 SE501.1 NO SCALE
 10009



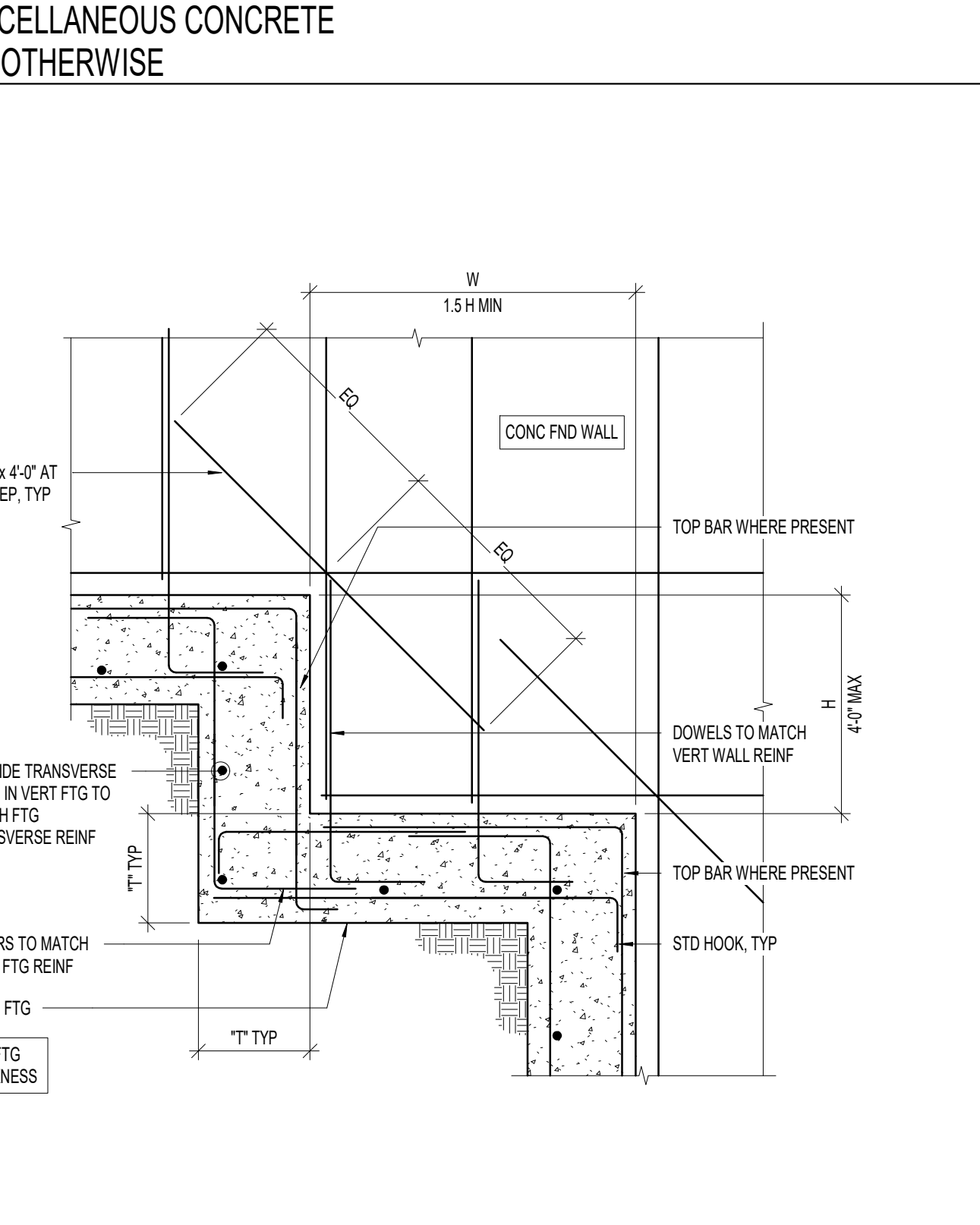
10 FOUNDATION WALL AT OPENING DETAIL
 SE501.1 NO SCALE
 10010



11 TYPICAL TUBE STEEL COLUMN TO CONCRETE FOOTING
 SE501.1 NO SCALE
 10011

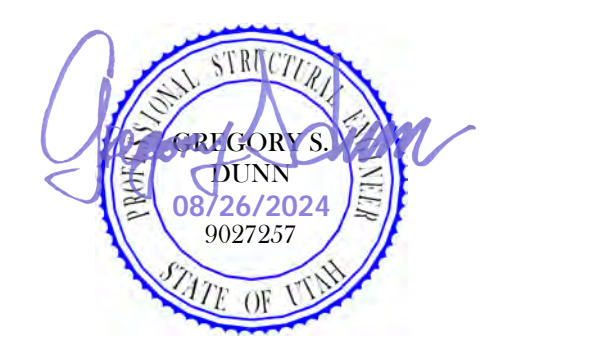


12 TYPICAL TUBE STEEL COLUMN TO CONCRETE PIER
 SE501.1 NO SCALE
 10012



13 TYPICAL FOOTING STEP AT CONCRETE FOUNDATION WALL
 SE501.1 NO SCALE
 10013

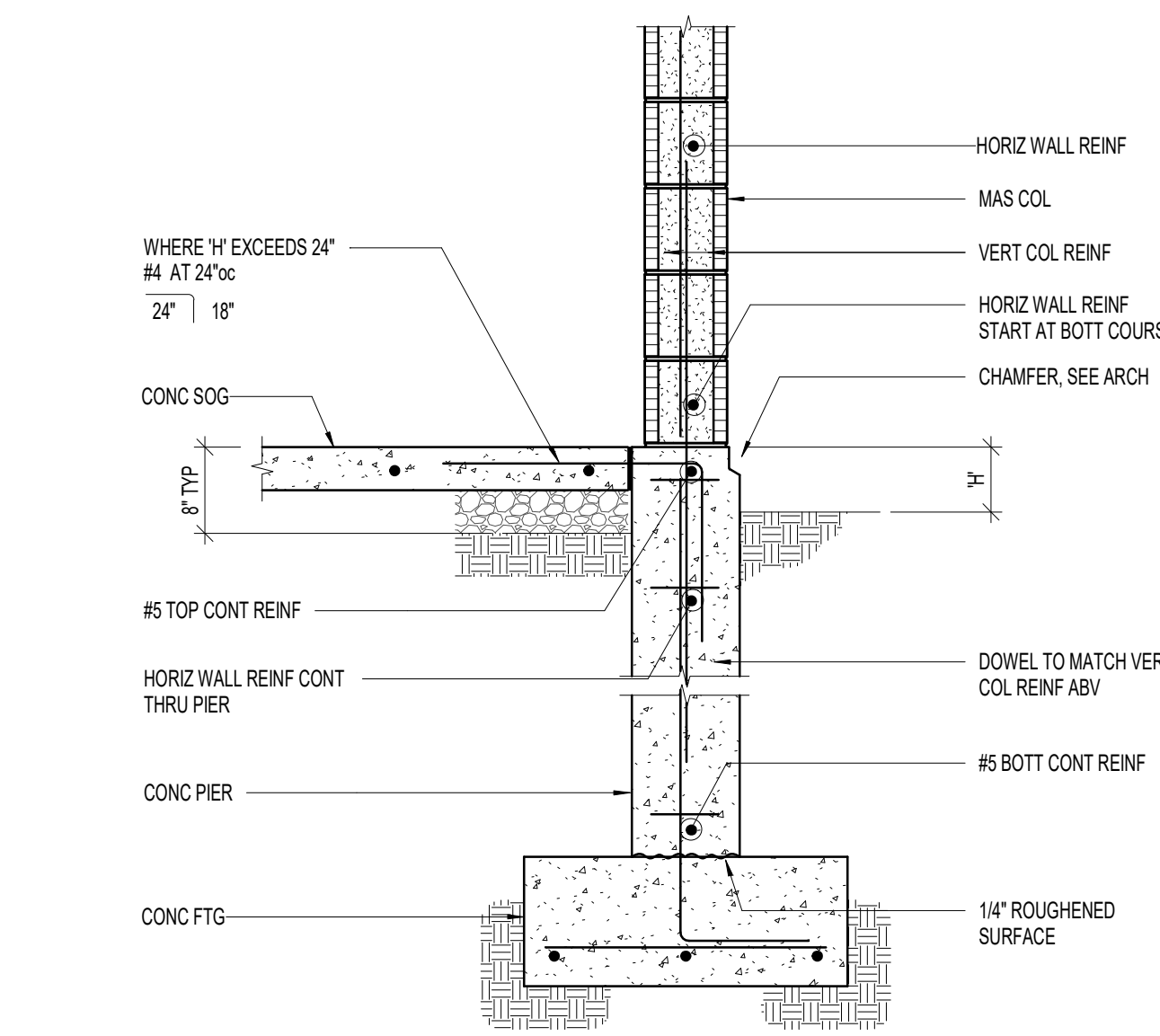
DTC WELDING TECH & FABRICATION
BUILDING
 355 SOUTH 650 EAST
 KAYSVILLE, UT 84037



2024-08-26
 BID PACKAGE #1

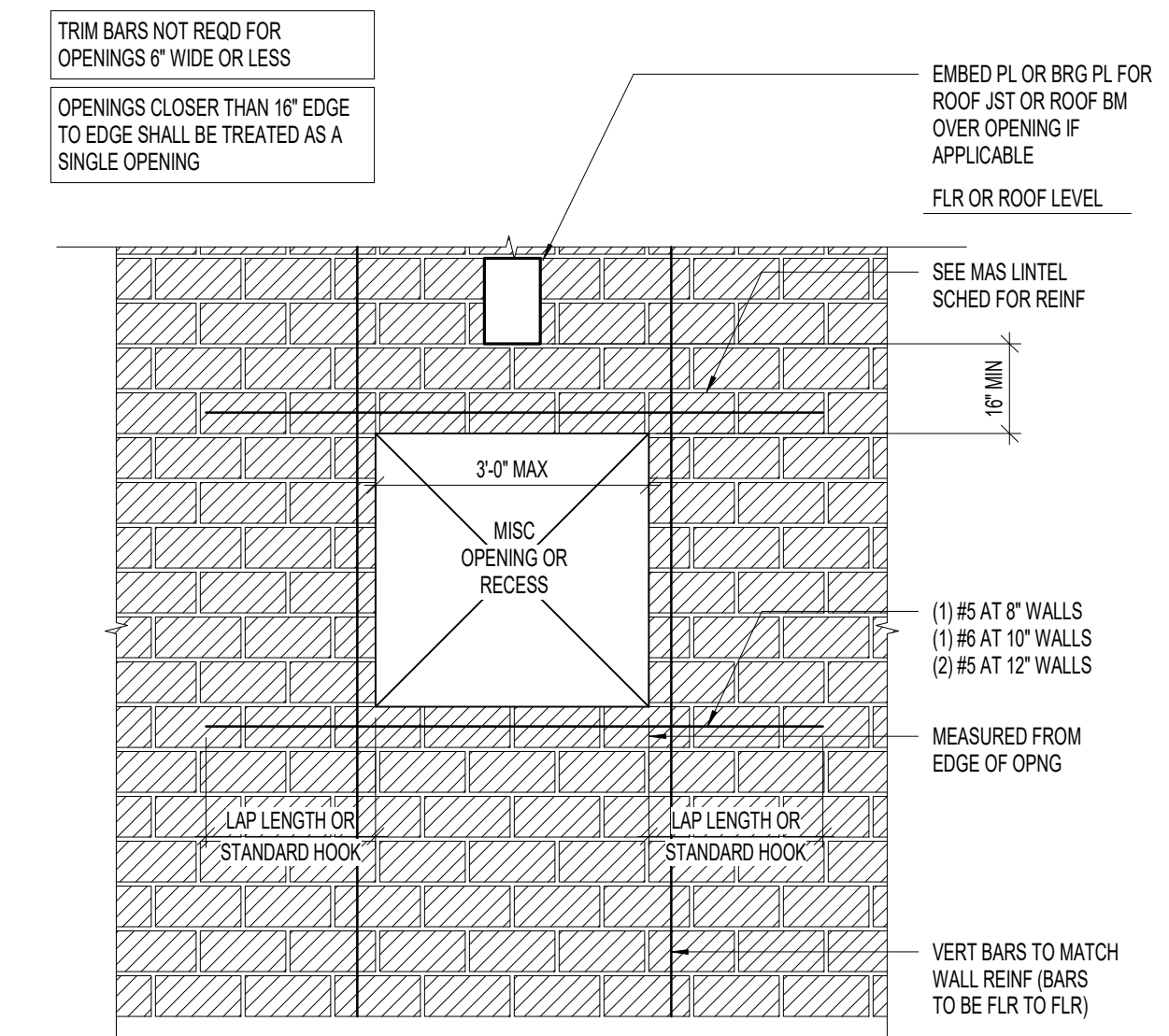
NOTE: THESE STRUCTURAL DRAWINGS ARE BASED ON ARCHITECTURAL DRAWINGS DATED July 23, 2024. DIMENSIONS AND ELEVATIONS AS THEY RELATE TO THE BUILDING IN GENERAL, IN GRID TO GRID DIMENSIONS OR DECK BEARING ELEVATIONS, ARE SUPPLIED BY THE ARCHITECT. THEY ARE PROVIDED ON THE STRUCTURAL PLANS AND DETAILS FOR THE CONVENIENCE OF THE CONTRACTOR. VERIFY DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS.

FOOTING AND FOUNDATION DETAILS
SE501.1
 (801) 355-5915



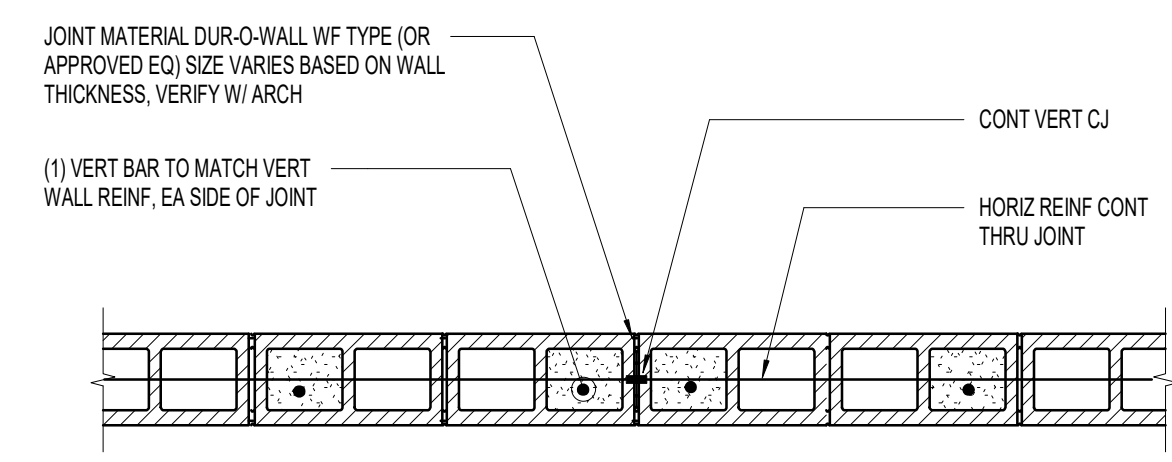
5 TYPICAL MASONRY WALL ON FOUNDATION WALL

SE502.1 NO SCALE: 1/8\"/>



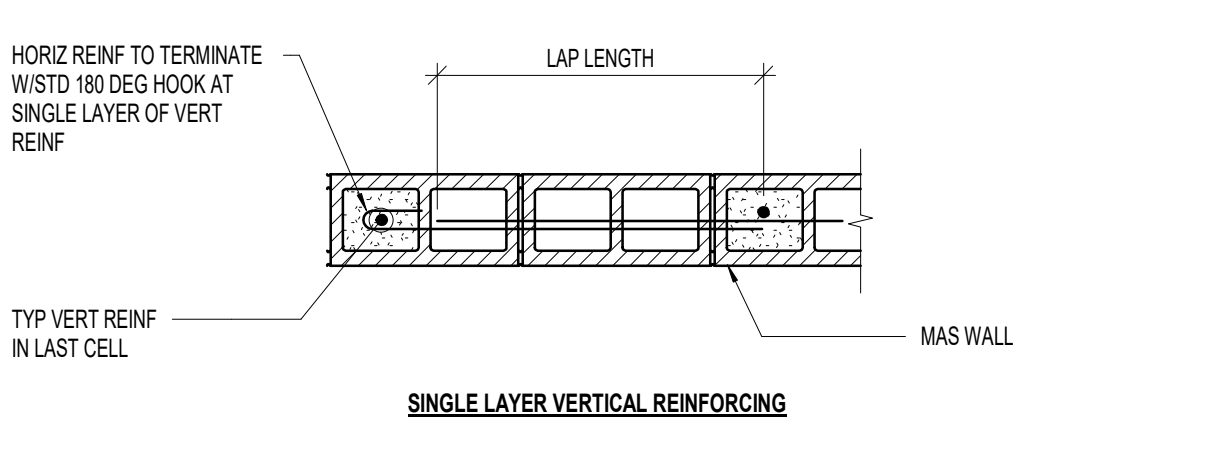
4 TYPICAL REINFORCING DETAIL FOR MISCELLANEOUS MASONRY WALL OPENINGS AND RECESSES

SE502.1 NO SCALE: 1/8\"/>



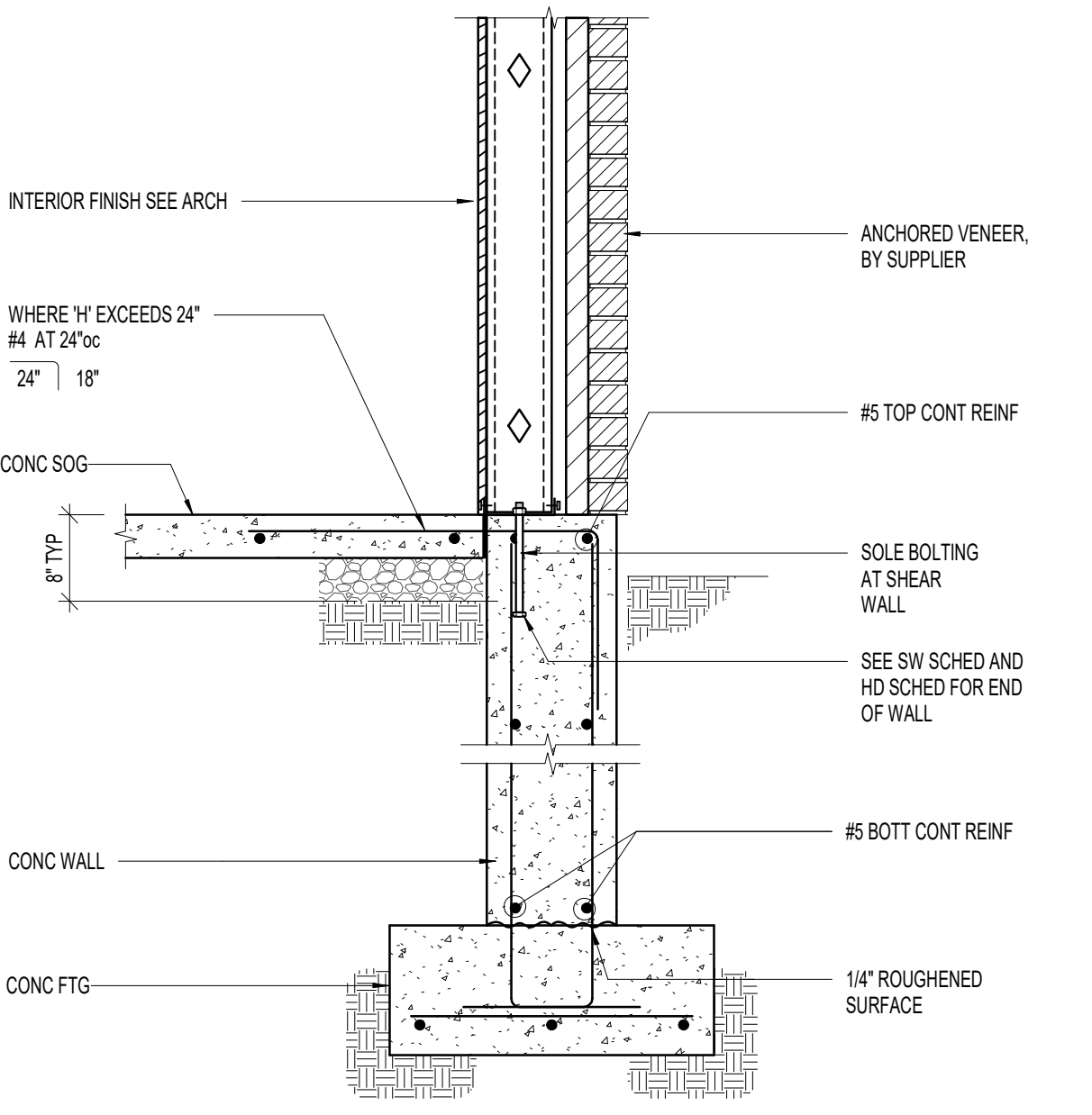
3 TYPICAL CONTROL JOINT FOR SINGLE REINFORCED MASONRY WALLS (PLAN VIEW)

SE502.1 NO SCALE: 1/8\"/>



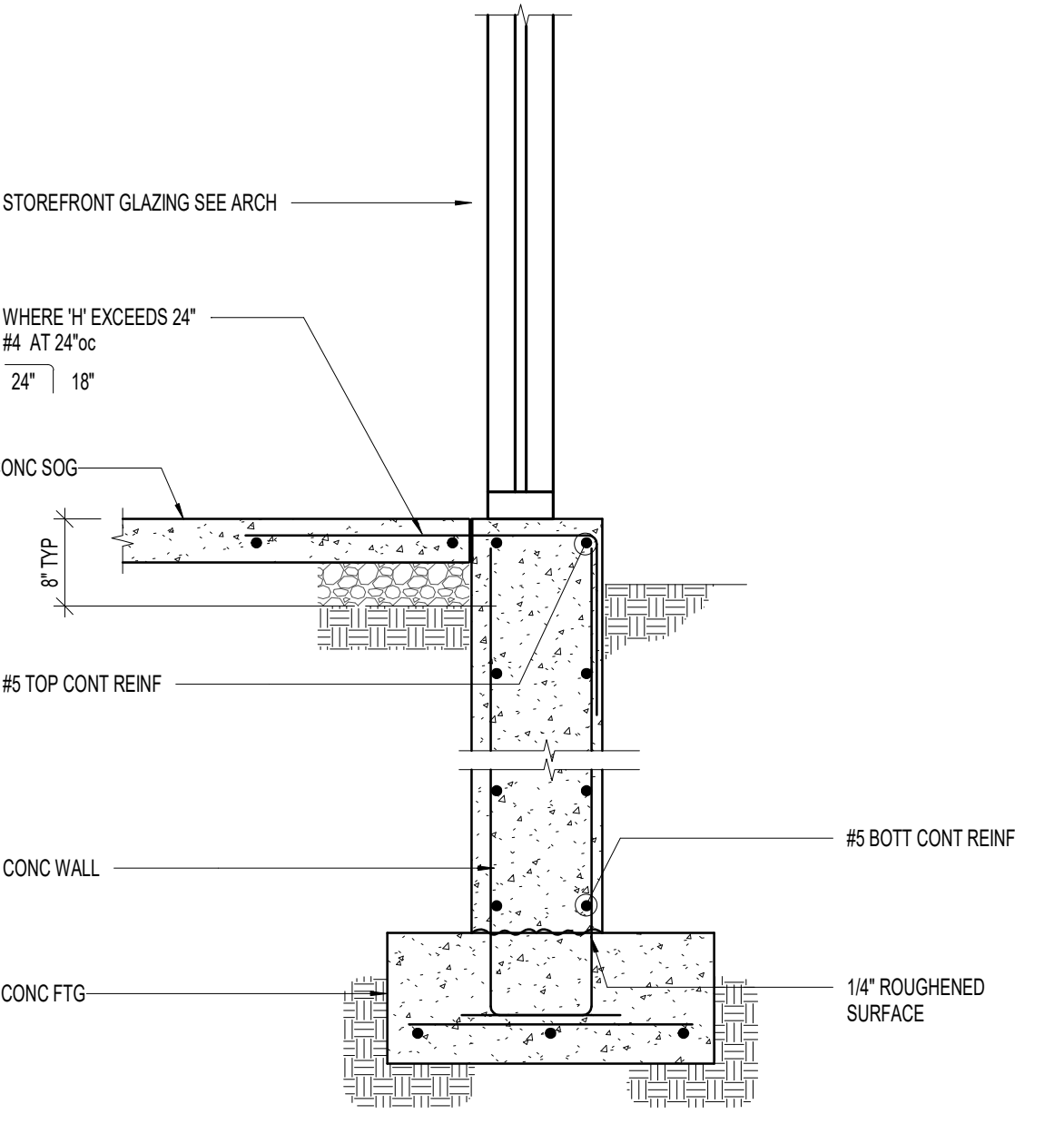
2 TYPICAL TERMINATION OF HORIZONTAL REINFORCING FOR REINFORCED MASONRY WALLS (PLAN VIEW)

SE502.1 NO SCALE: 1/8\"/>



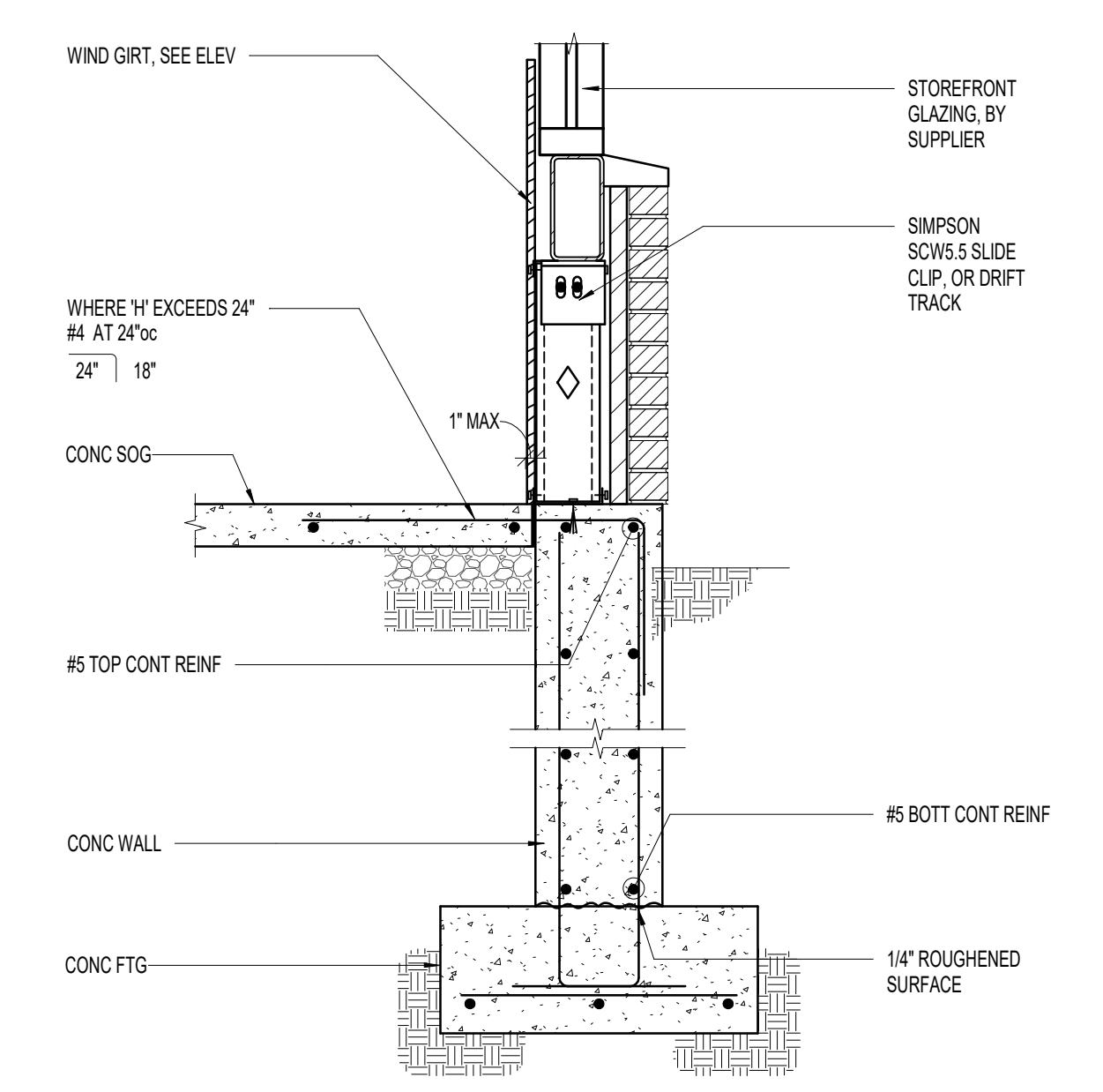
7 TYPICAL STEEL STUD ON CONCRETE FOUNDATION WALL

SE502.1 NO SCALE: 1/8\"/>



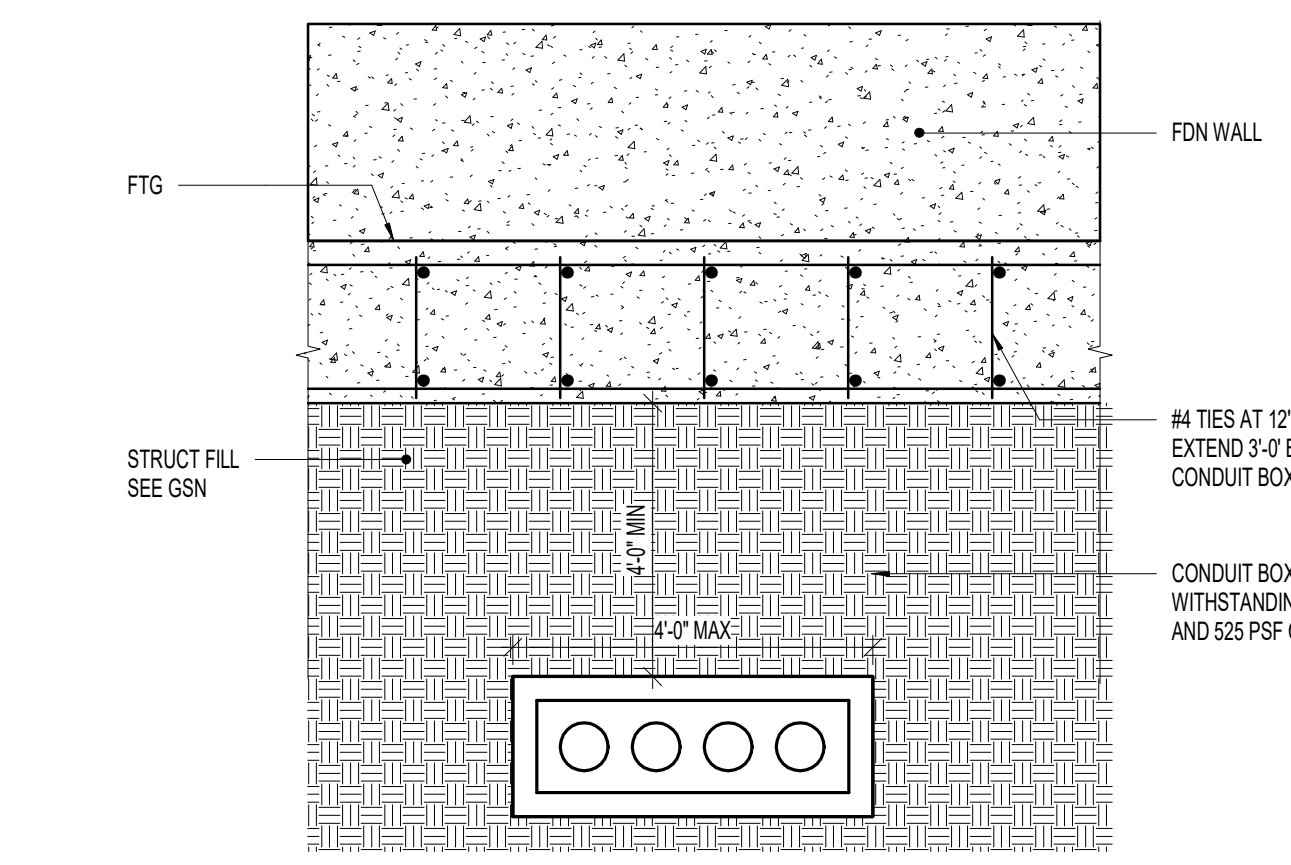
8 TYPICAL STEEL STUD ON CONCRETE FOUNDATION WALL

SE502.1 NO SCALE: 1/8\"/>



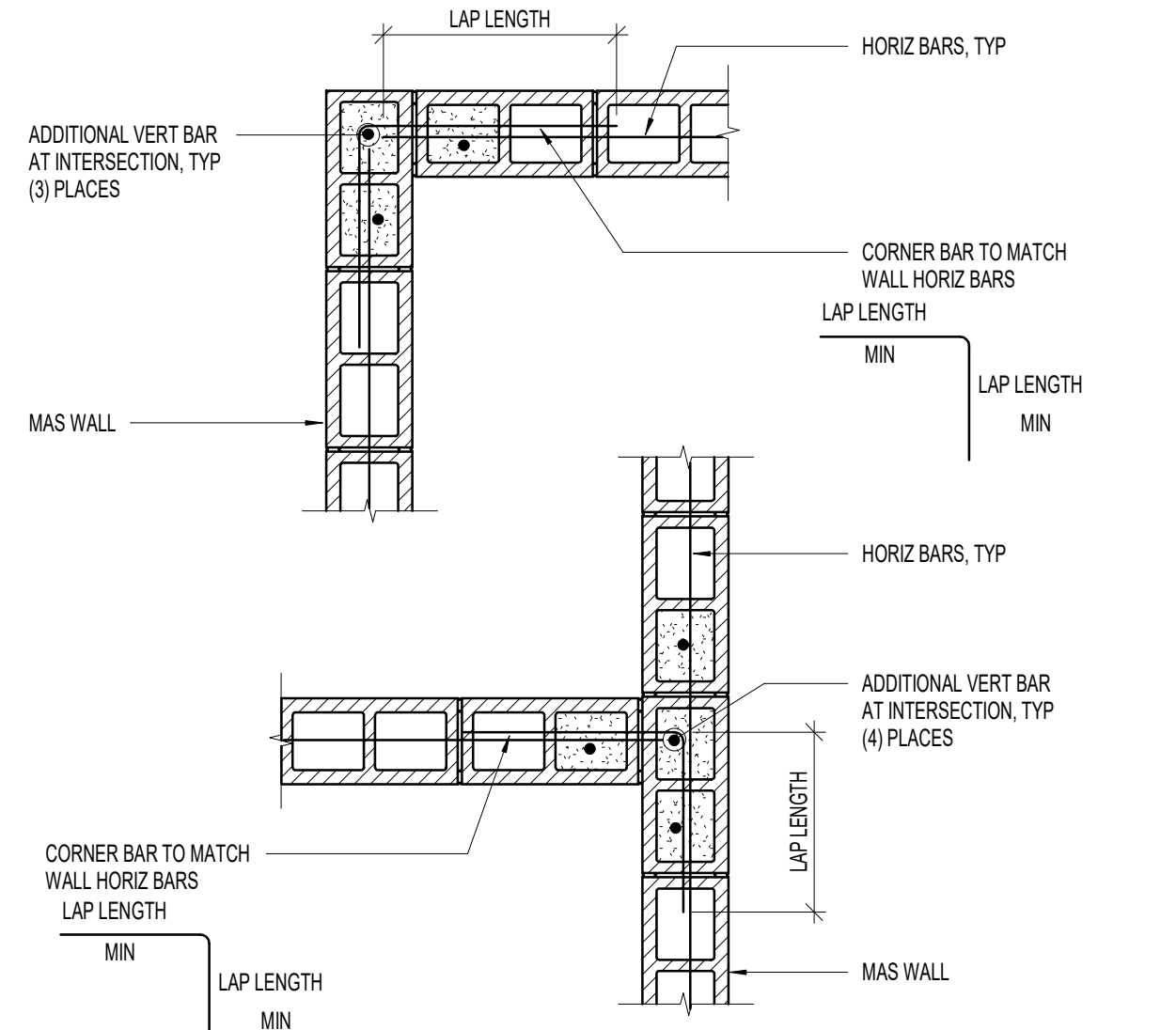
9 STEEL STUD ON CONCRETE FOUNDATION WALL AT WINDOW

SE502.1 NO SCALE: 1/8\"/>



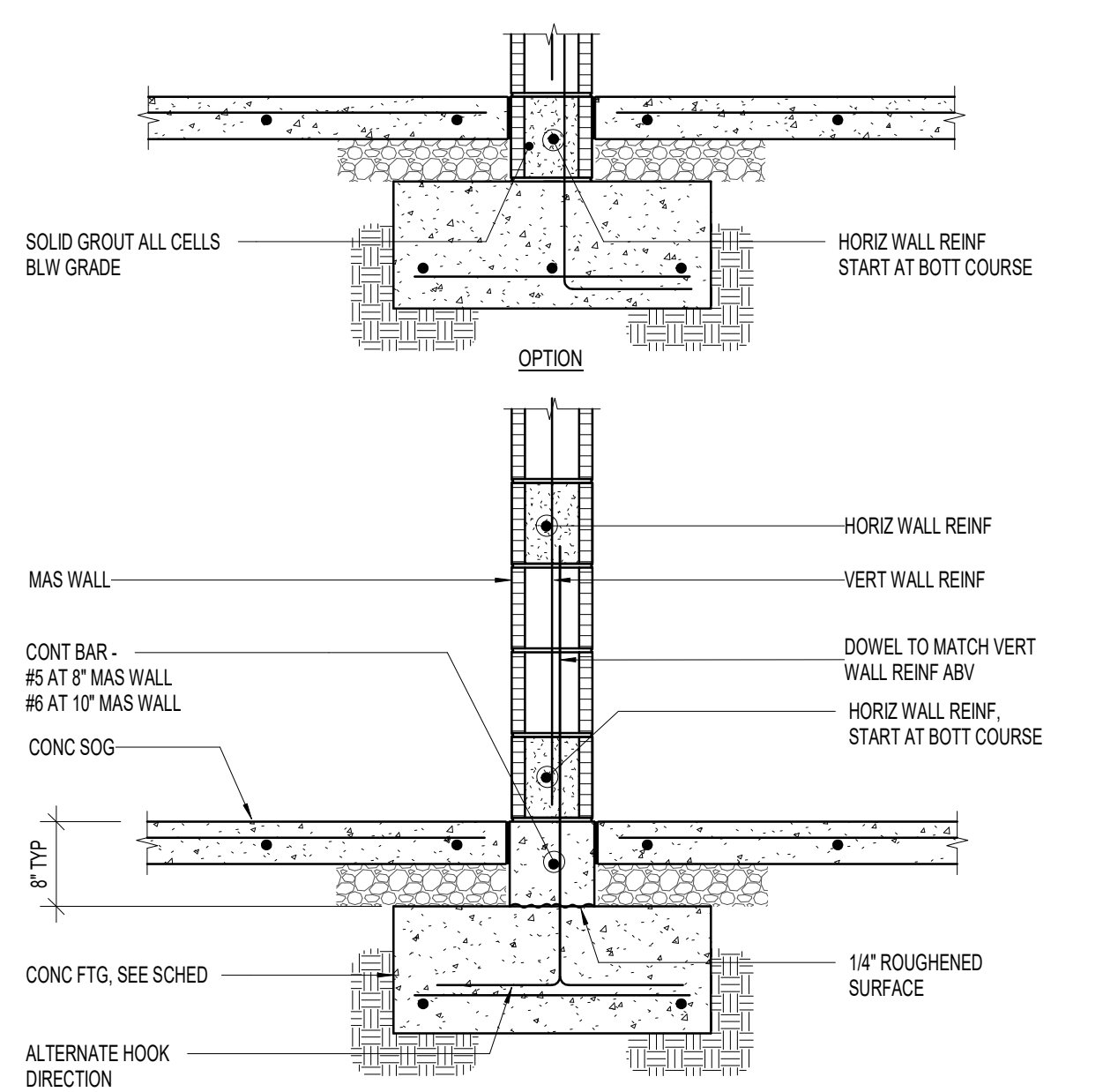
10 FOUNDATION SUPPORT OVER CONDUIT BOX

SE502.1 NO SCALE: 1/8\"/>



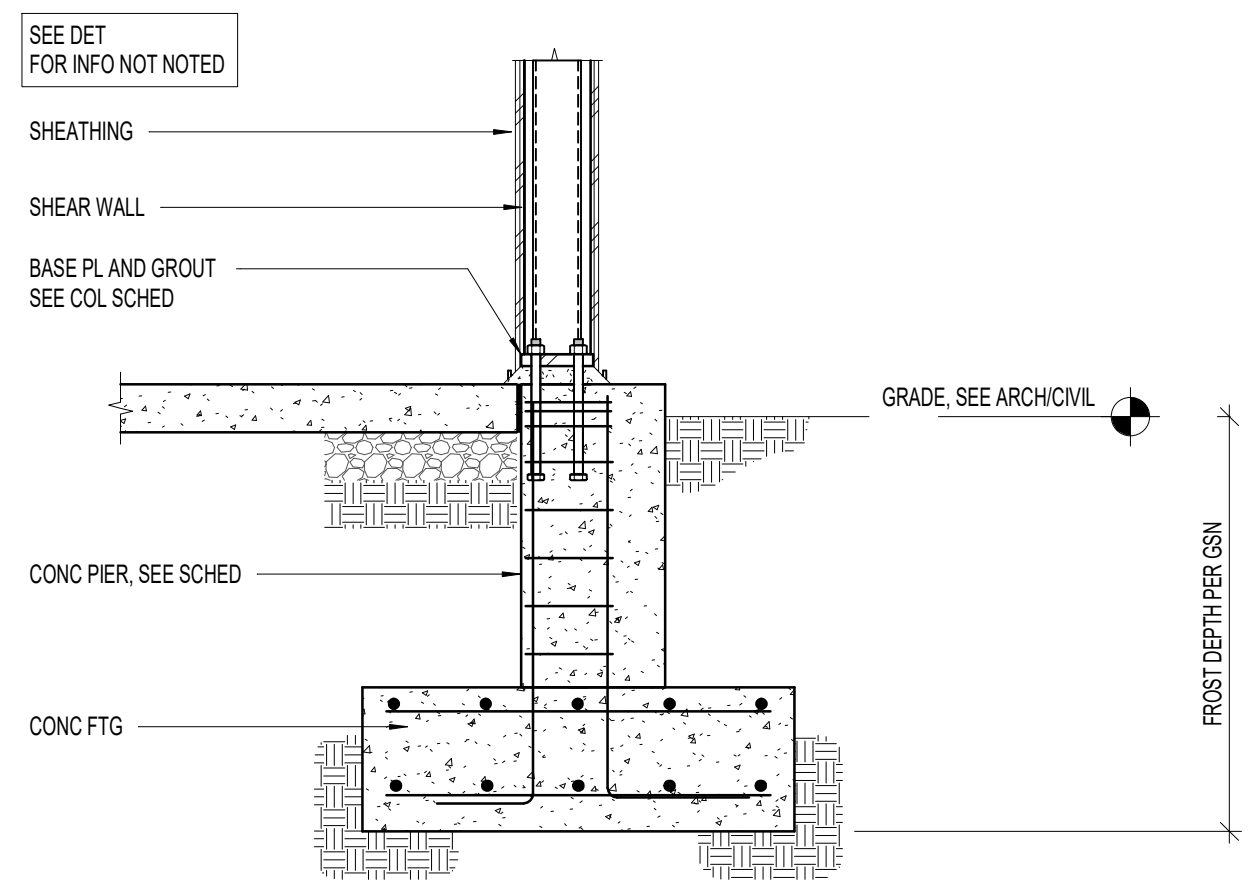
1 TYPICAL CORNER WALL REINFORCING FOR SINGLE REINFORCED MASONRY WALLS (PLAN VIEW)

SE502.1 NO SCALE: 1/8\"/>



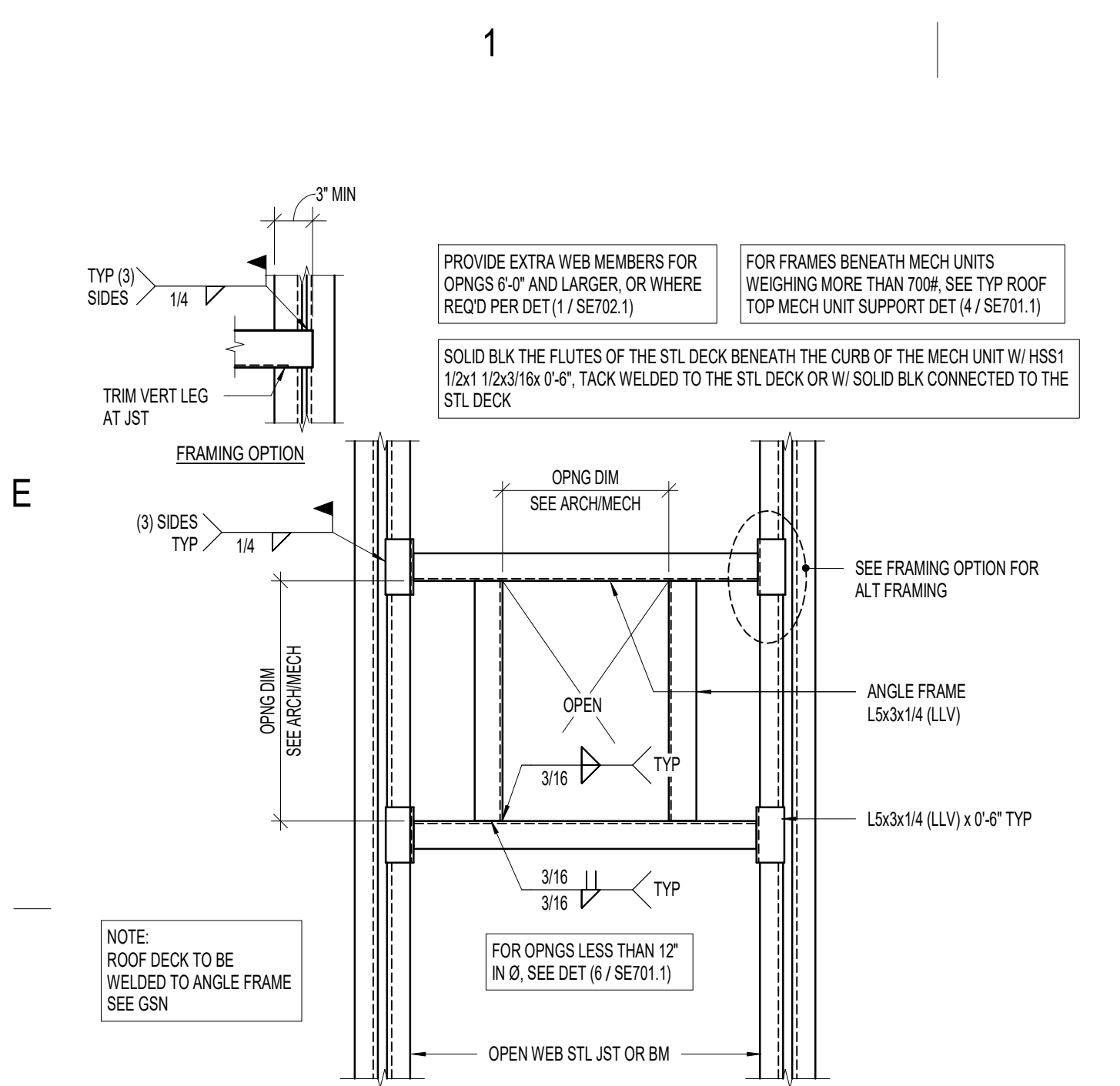
6 TYPICAL SINGLE MAT REINFORCING MASONRY WALL ON INTERIOR CONCRETE FOOTING

SE502.1 NO SCALE: 1/8\"/>

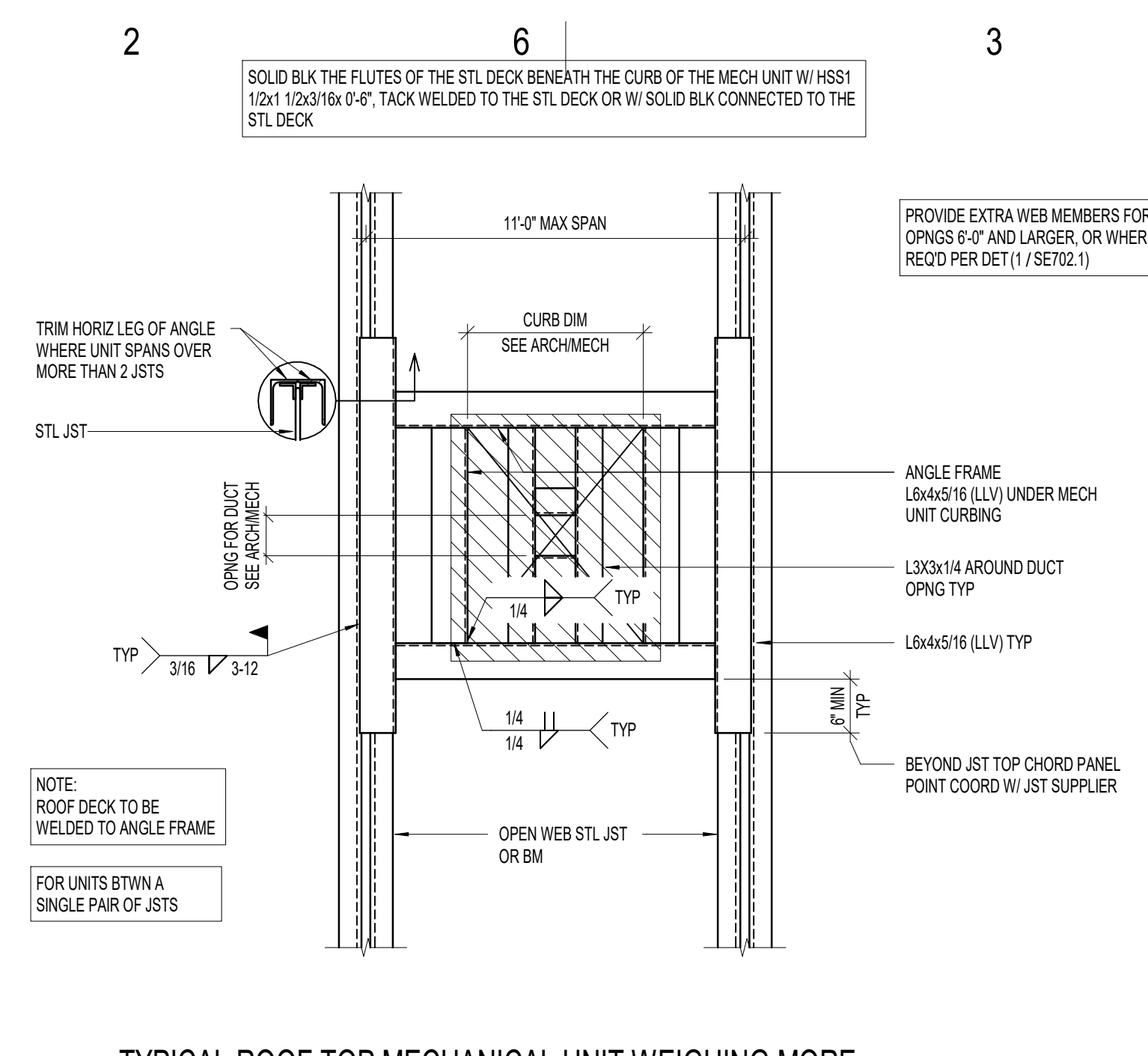


11 DOUBLE WALL AT COLUMN

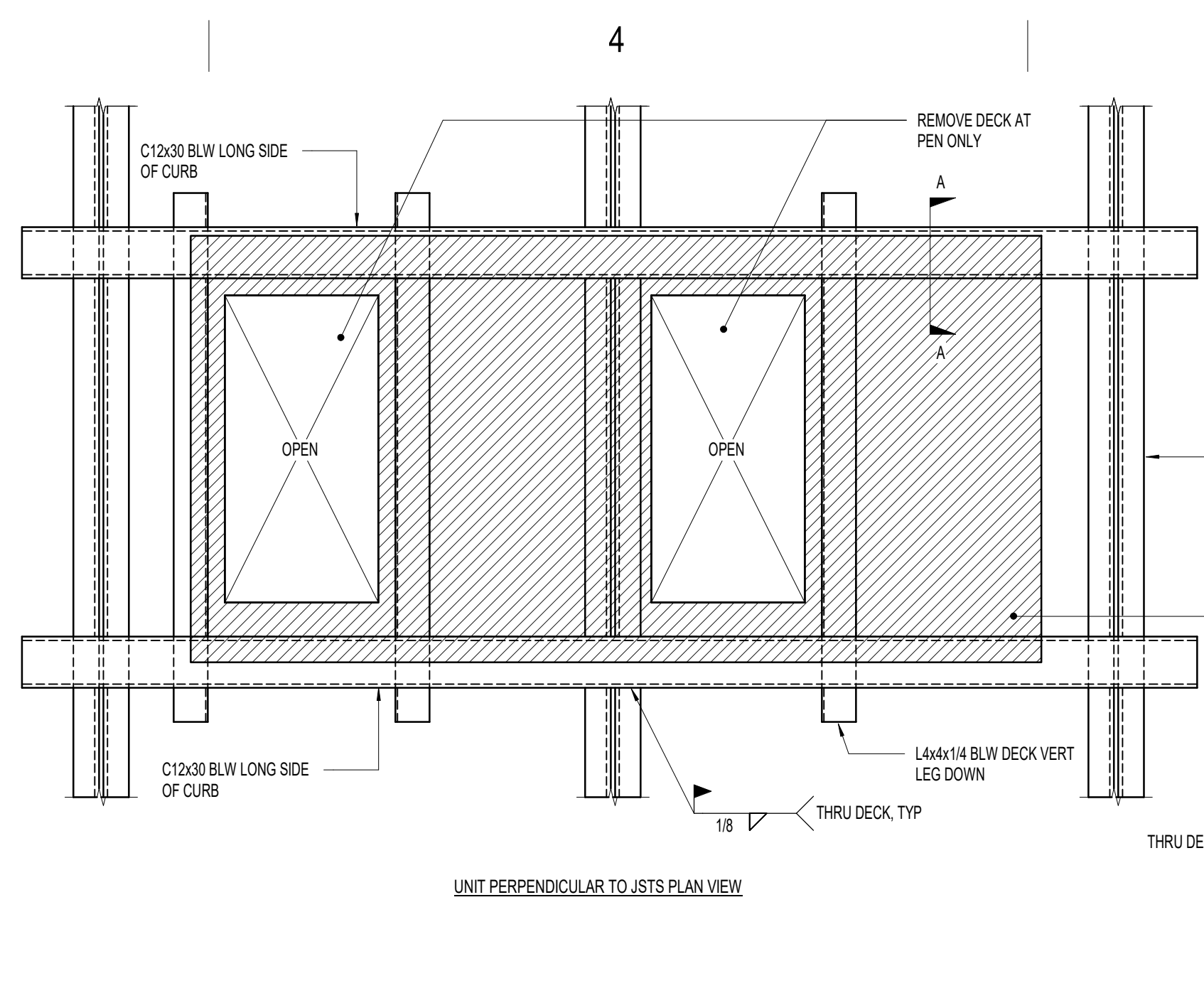
SE502.1 NO SCALE: 1/8\"/>



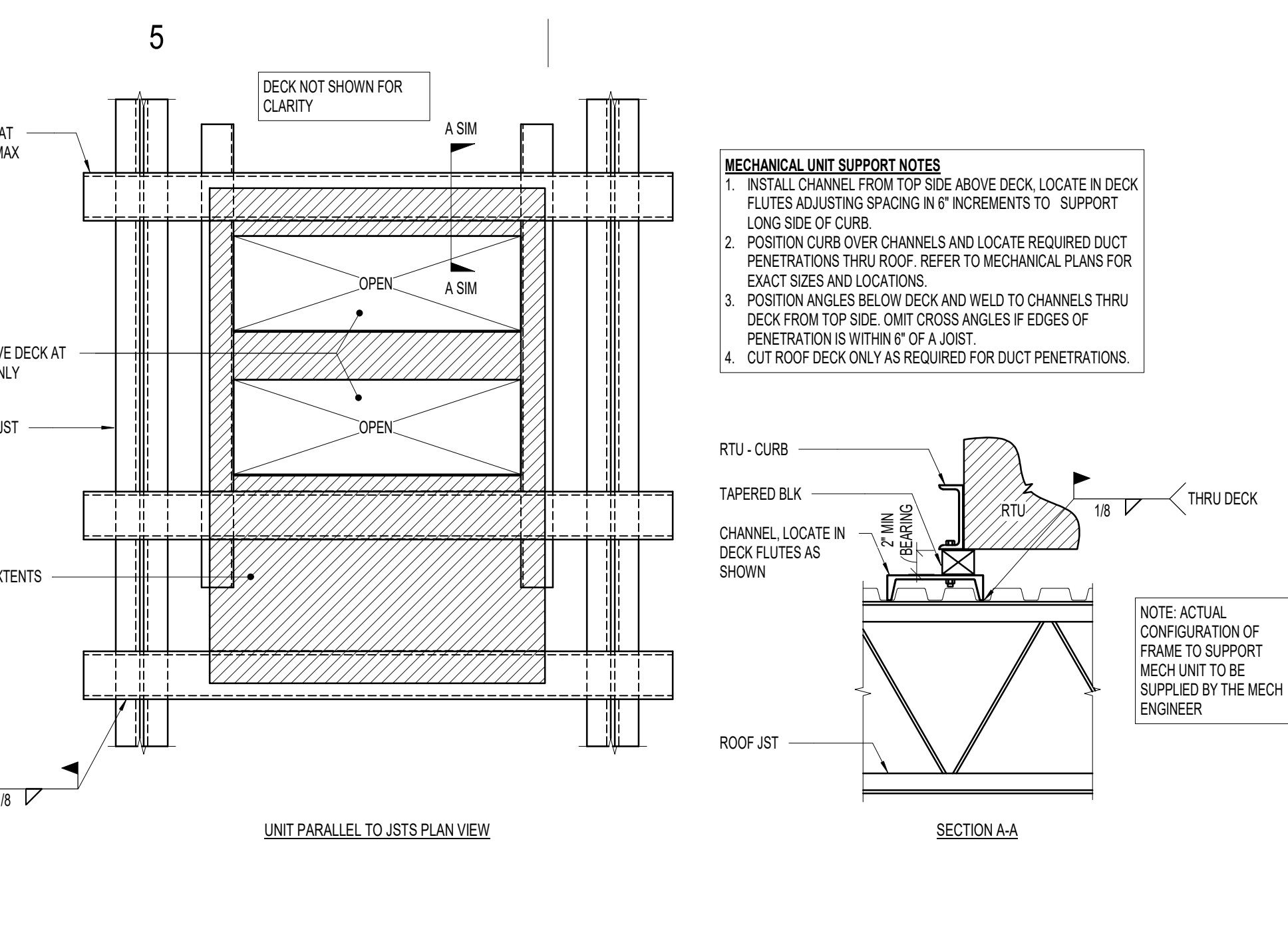
1 TYPICAL ROOF OPENING OR MECHANICAL UNIT WEIGHING LESS THAN 700#
SE701.1 NO SCALE



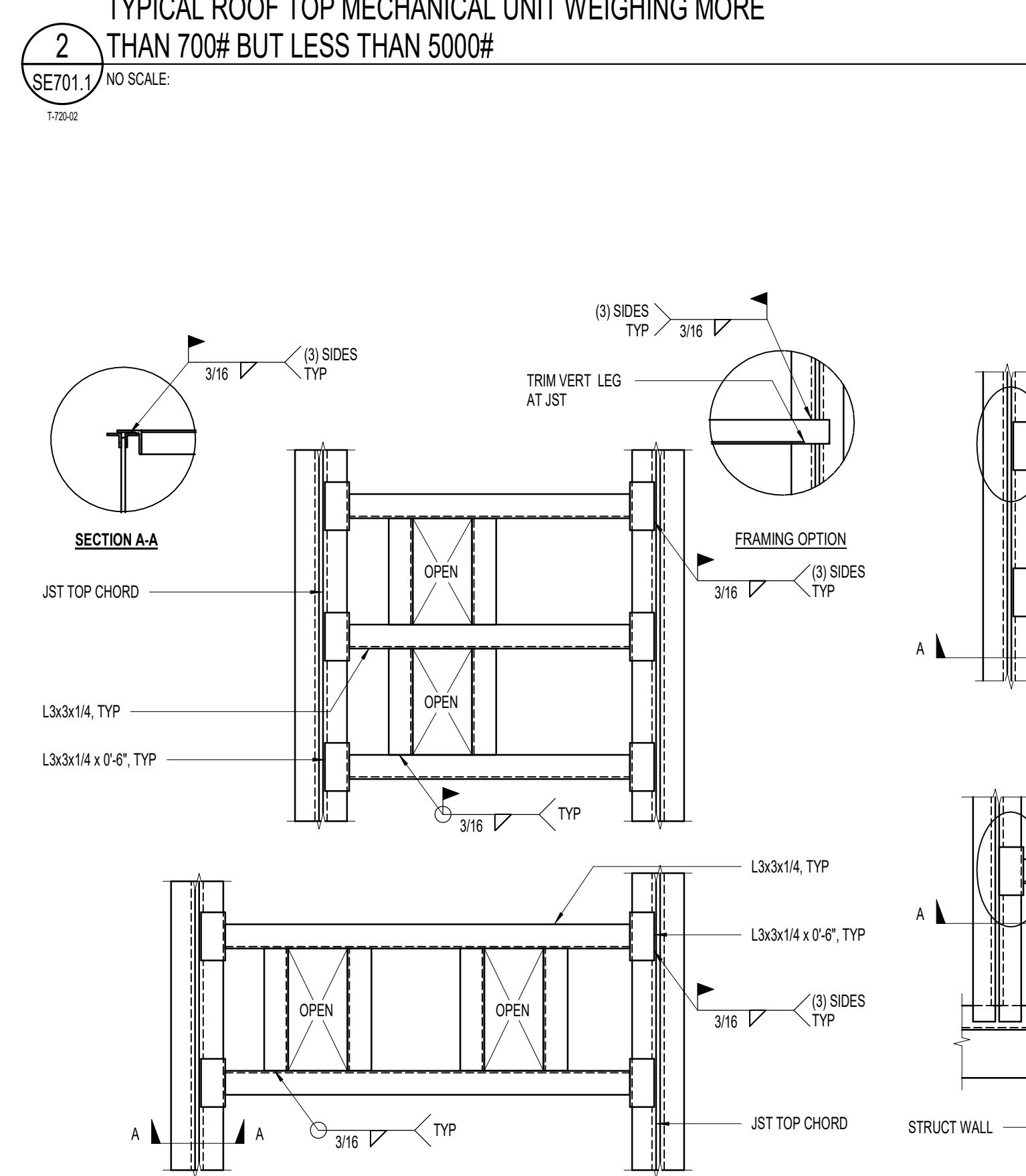
2 TYPICAL ROOF TOP MECHANICAL UNIT WEIGHING MORE THAN 700# BUT LESS THAN 5000#
SE701.1 NO SCALE



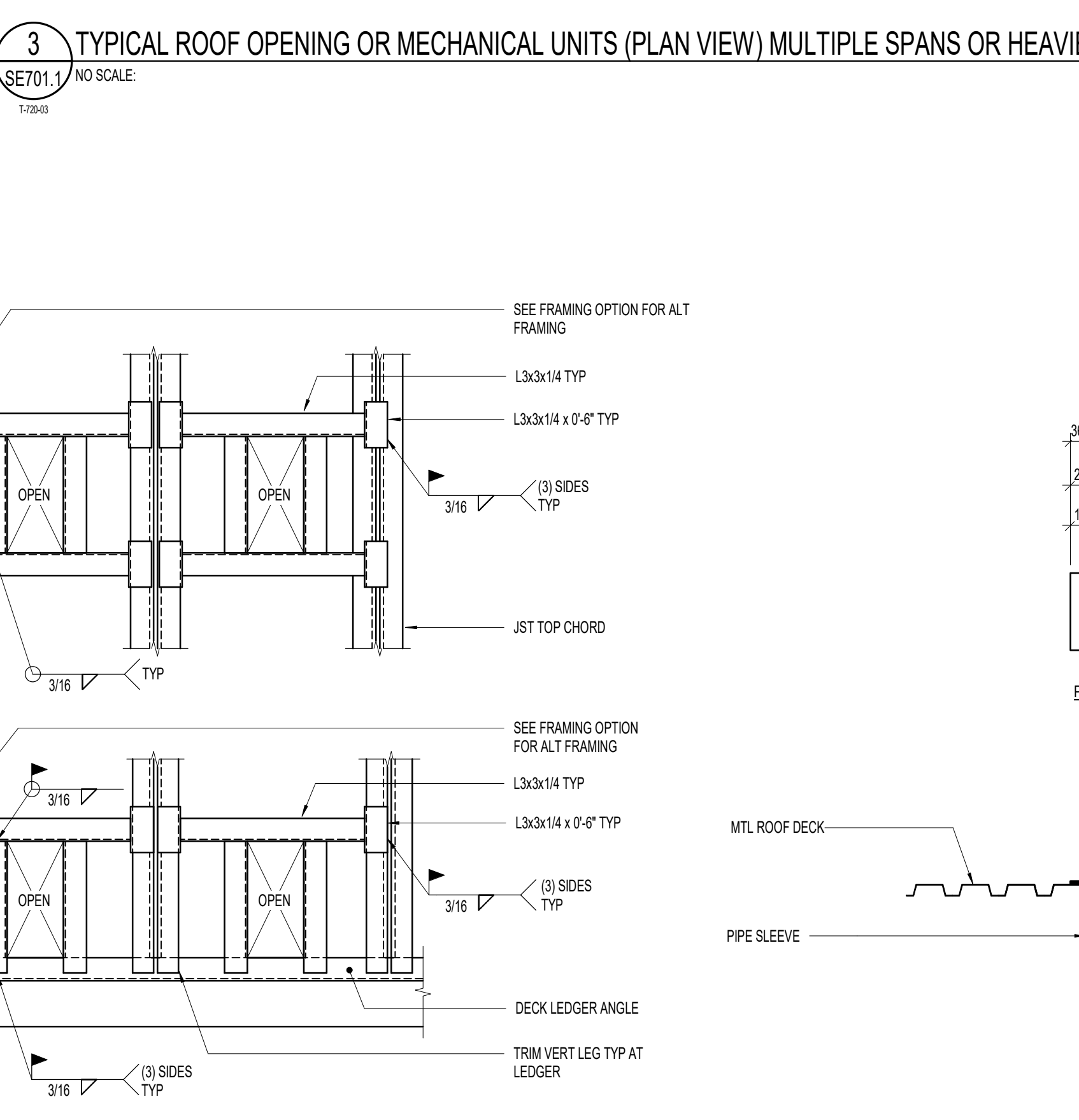
3 TYPICAL ROOF OPENING OR MECHANICAL UNITS (PLAN VIEW) MULTIPLE SPANS OR HEAVIER UNITS
SE701.1 NO SCALE



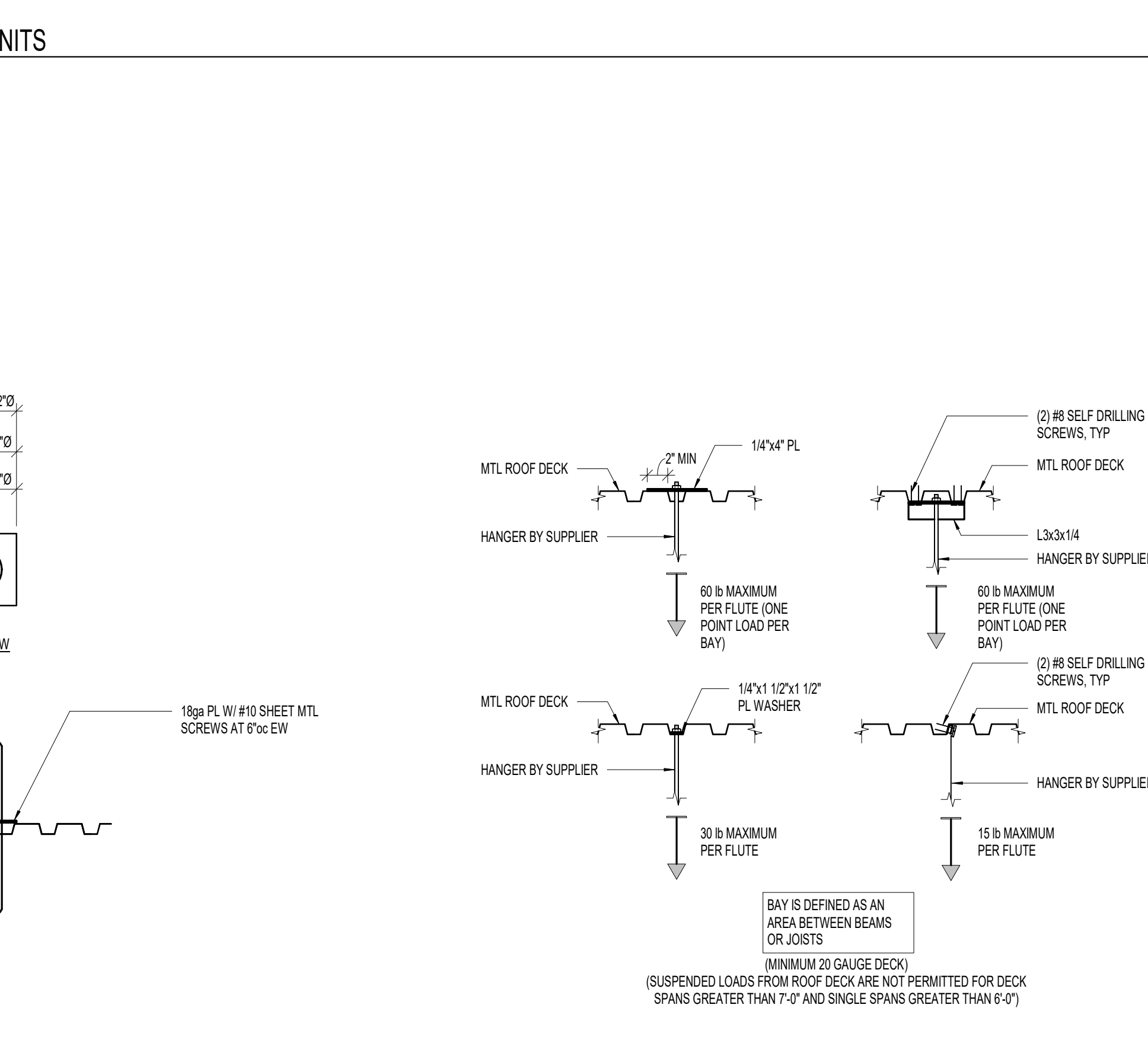
4 HANGING MECHANICAL UNIT
SE701.1 NO SCALE



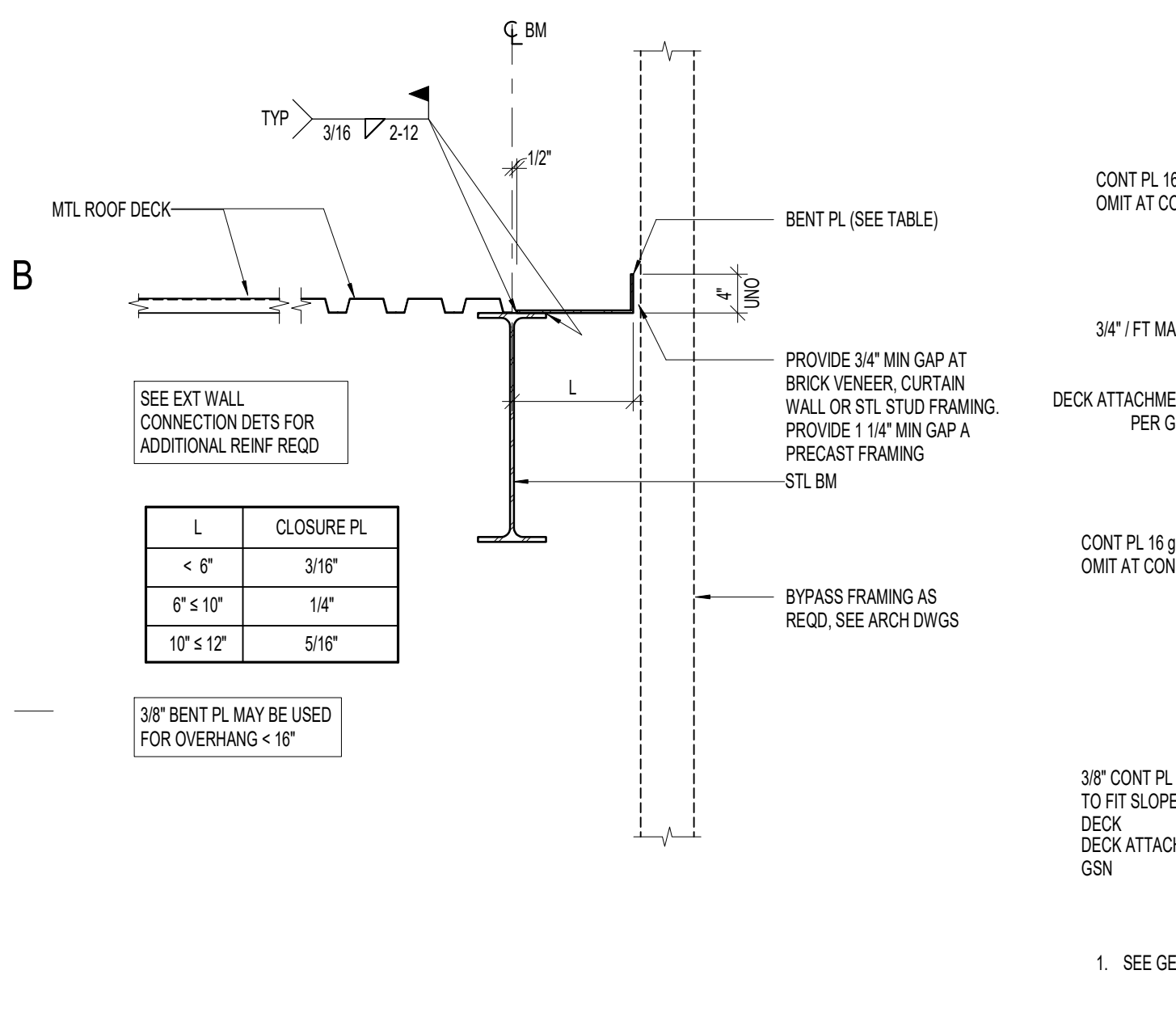
5 TYPICAL ROOF DRAIN SUPPORT (PLAN VIEW)
SE701.1 NO SCALE



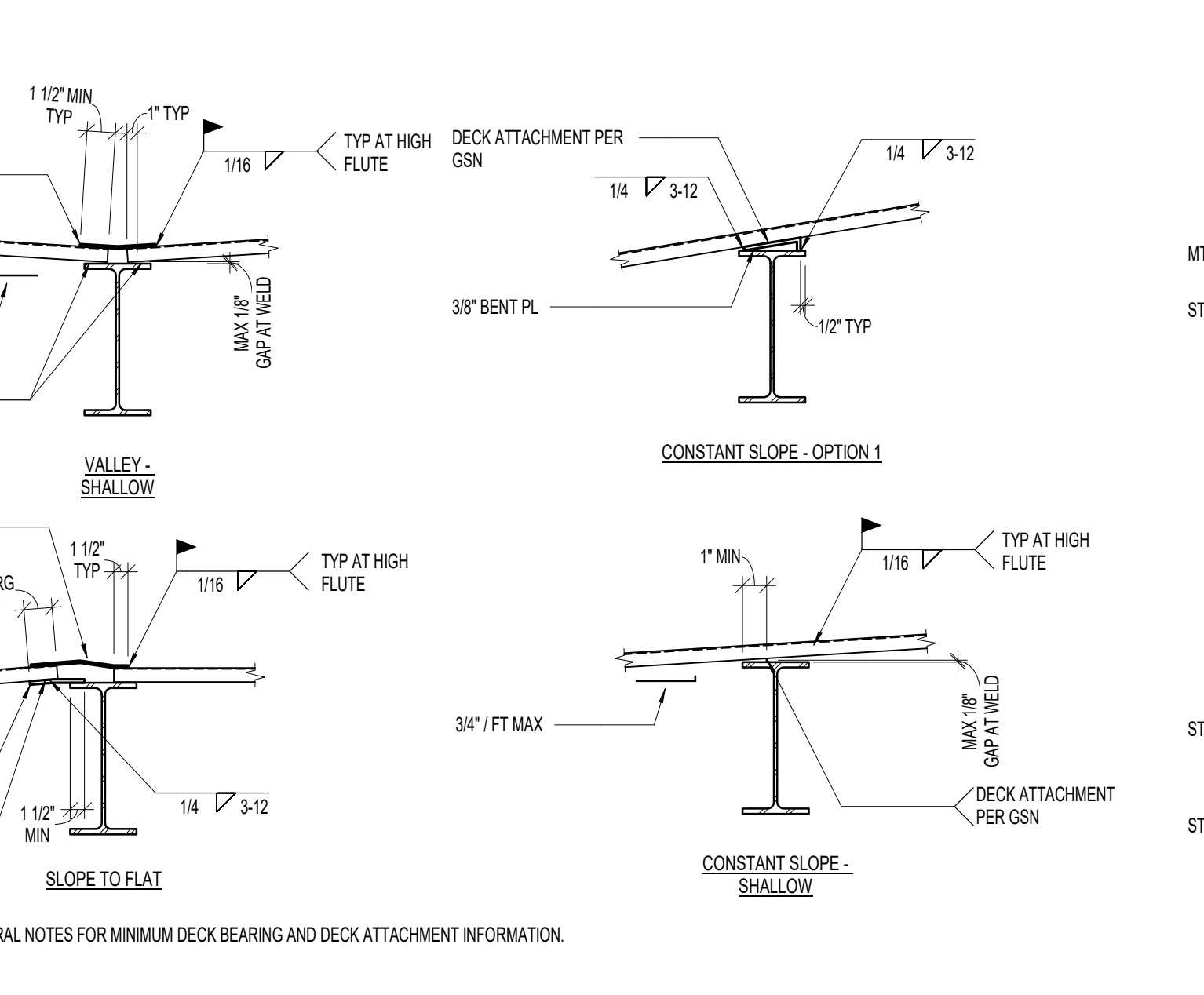
6 TYPICAL PIPE SLEEVE HOLE DETAIL (12"Ø OR LESS) THRU ROOF DECK
SE701.1 NO SCALE



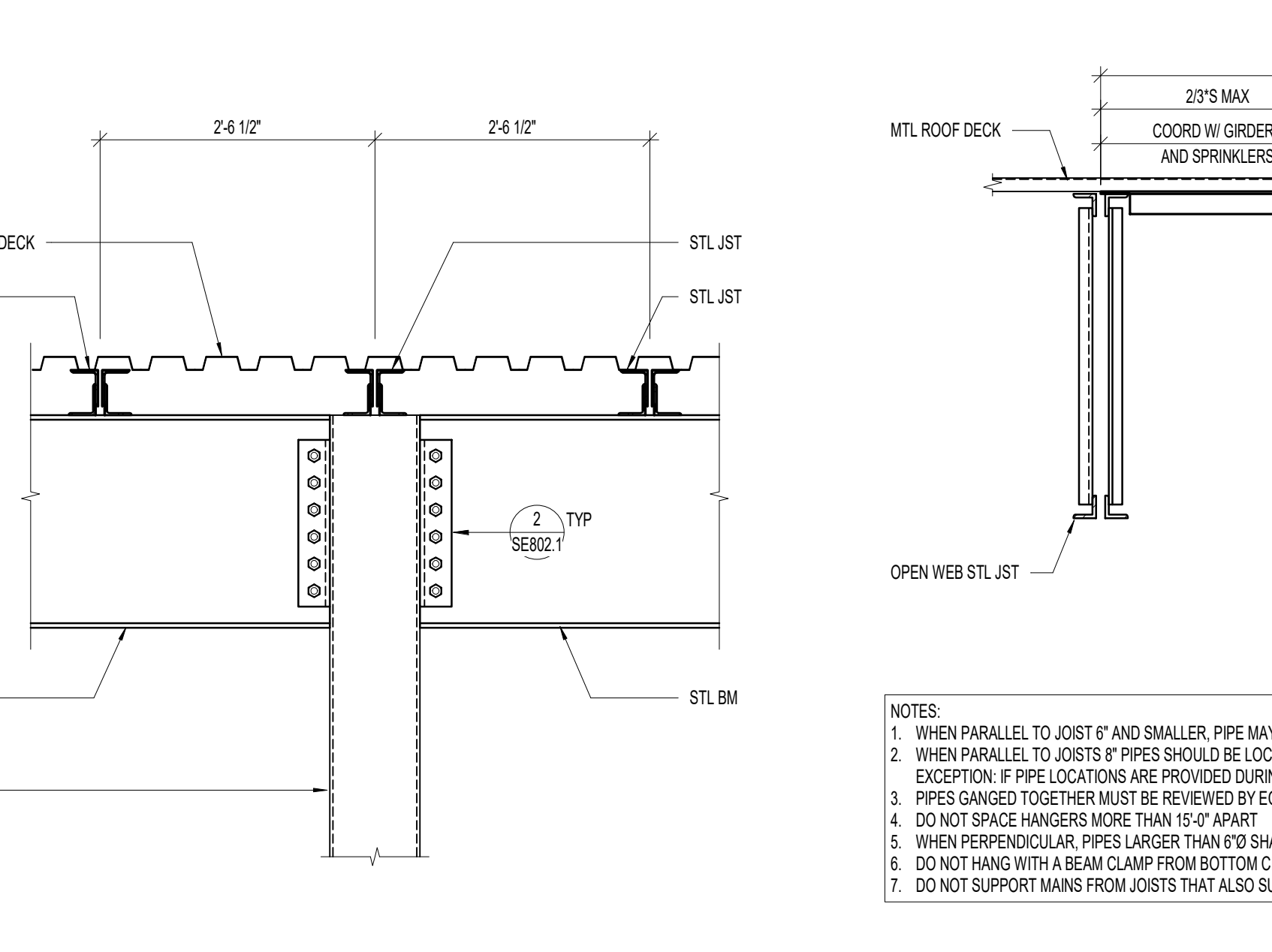
7 SUSPENDED LOADS FROM METAL DECK
SE701.1 NO SCALE



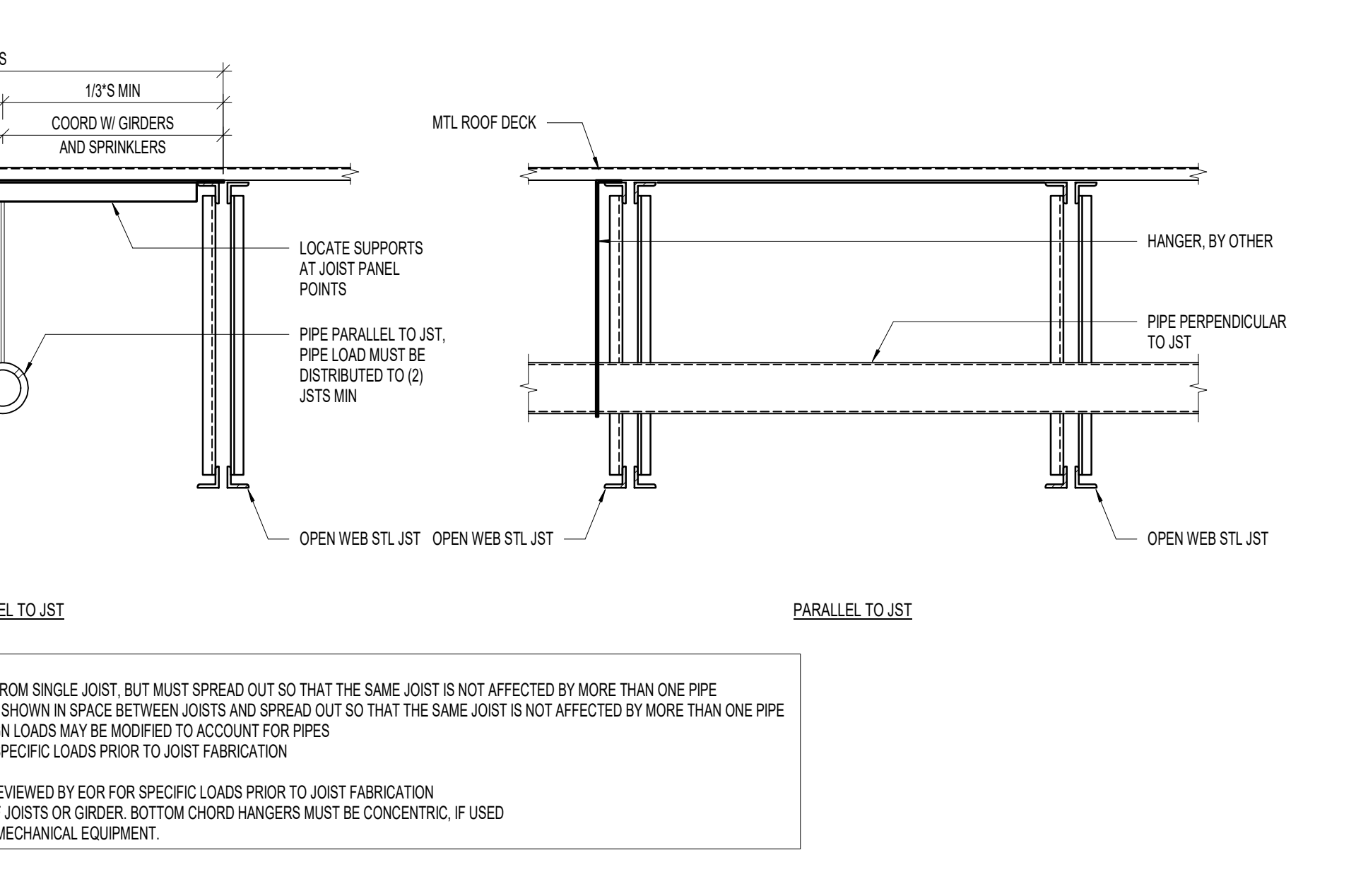
8 TYPICAL CLOSURE PLATE (OVERHANG 'L' ≤ 12")
SE701.1 NO SCALE



9 SUPPORT OF SLOPED STEEL DECK AT BEAMS OR JOISTS
SE701.1 NO SCALE



10 WIDE FLANGE BEAM TO STEEL COLUMN
SE701.1 NO SCALE



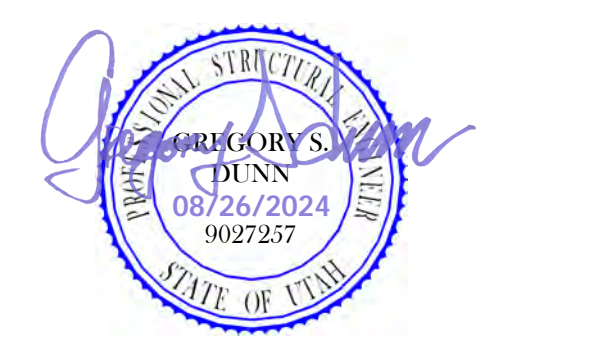
11 FIRE PROTECTION PIPE HANGER DETAIL
SE701.1 NO SCALE

MECHANICAL UNIT SUPPORT NOTES

- INSTALL CHANNEL FROM TOP SIDE ABOVE DECK. LOCATE IN DECK FLUTES ADJUSTING SPACING IN 6" INCREMENTS TO SUPPORT LONG SIDE OF CURB.
- POSITION CURB OVER CHANNELS AND LOCATE REQUIRED DUCT PENETRATIONS THRU ROOF. REFER TO MECHANICAL PLANS FOR EXACT SIZES AND LOCATIONS.
- POSITION ANGLES BELOW DECK AND WELD TO CHANNELS THRU DECK FROM TOP SIDE. OMIT CROSS ANGLES IF EDGES OF PENETRATION IS WITHIN 6" OF A JOIST.
- CUT ROOF DECK ONLY AS REQUIRED FOR DUCT PENETRATIONS.

NOTE: ACTUAL CONFIGURATION OF FRAME TO SUPPORT MECH UNIT TO BE SUPPLIED BY THE MECH ENGINEER.

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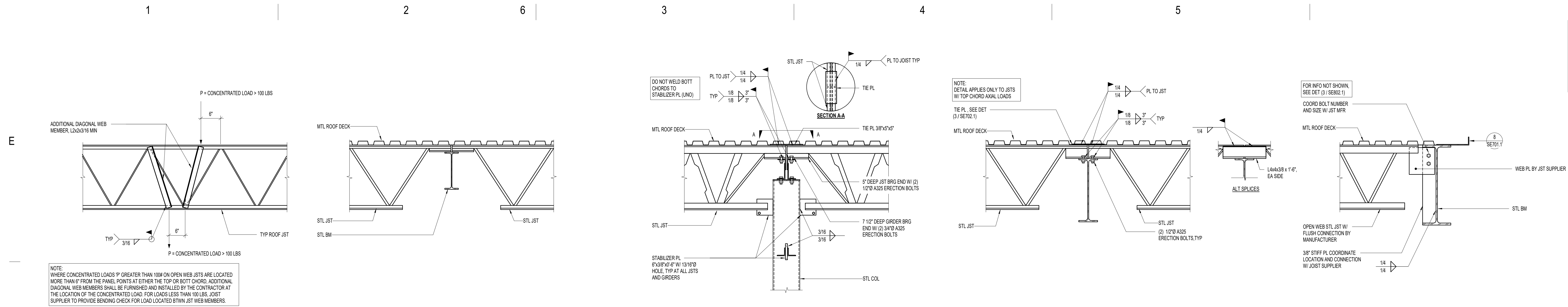


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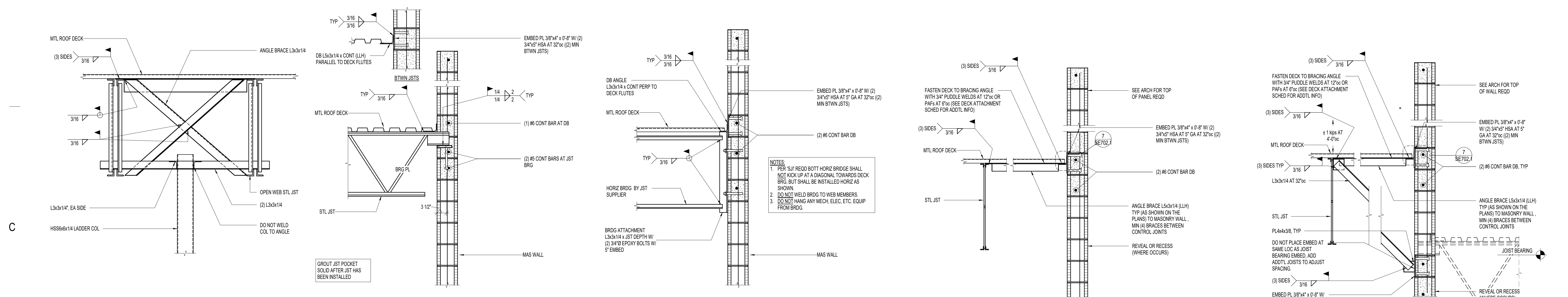
NOTE: THESE STRUCTURAL DRAWINGS ARE BASED ON ARCHITECTURAL DRAWINGS DATED July 23, 2024. DIMENSIONS AND ELEVATIONS AS THEY RELATE TO THE BUILDING IN GENERAL, IN GRID TO GRID DIMENSIONS OR DECK BEARING ELEVATIONS, ARE SUPPLIED BY THE ARCHITECT. THEY ARE PROVIDED ON THE STRUCTURAL PLANS AND DETAILS FOR THE CONVENIENCE OF THE CONTRACTOR. VERIFY DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS.

ROOF FRAMING
DETAILS
SE701.1

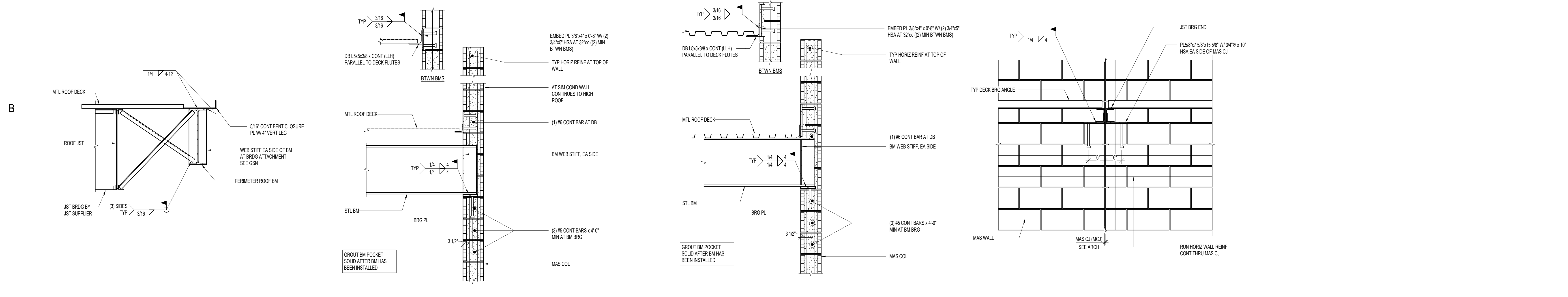
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1 TYPICAL JOIST REINFORCING AT CONCENTRATED LOAD
2 TYPICAL JOISTS BEARING ON STEEL BEAM
3 TYPICAL JOISTS AND GRIDER BEARING ON STEEL COLUMN WITH TIE PLATE
4 TYPICAL JOIST CHORD/TIE/DAG STRUT
5 TYPICAL FLUSH FRAMED JOIST CONNECTION



6 VERTICAL SLIP CONNECTION AT TOP OF LADDER COLUMN
7 TYPICAL JOIST / DECK BEARING DETAIL
8 TYPICAL DECK BEARING WITH BRIDGING DETAIL
9 TYPICAL STEEL BRACE CONNECTION DETAIL TO MASONRY WALL
10 WALL BRACING AT CANOPY



11 TYPICAL BRIDGING DETAIL
12 TYPICAL BEAM / DECK BEARING DETAIL
13 TYPICAL BEAM / DECK BEARING DETAIL
14 JOIST BEARING AT MASONRY CONTROL JOINT DETAIL

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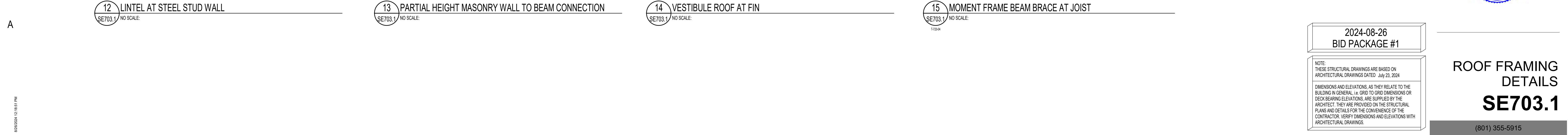
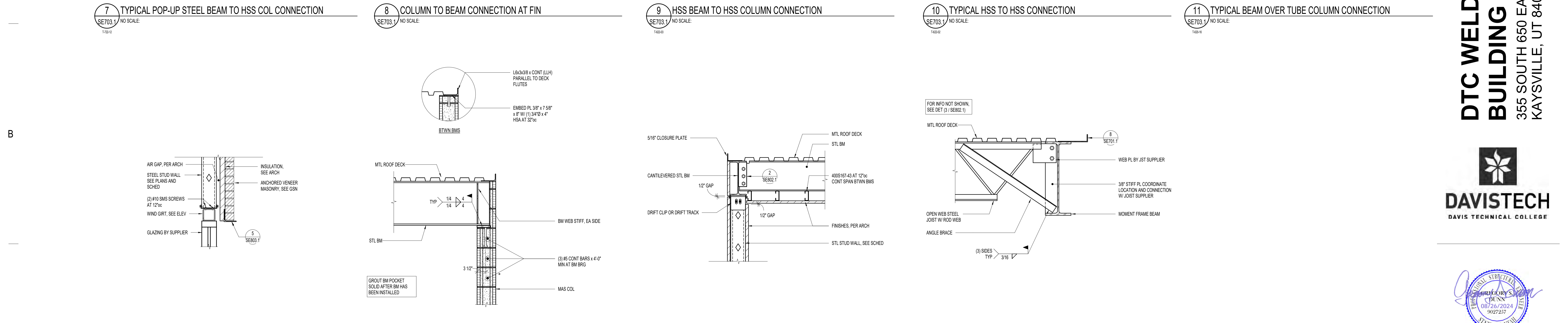
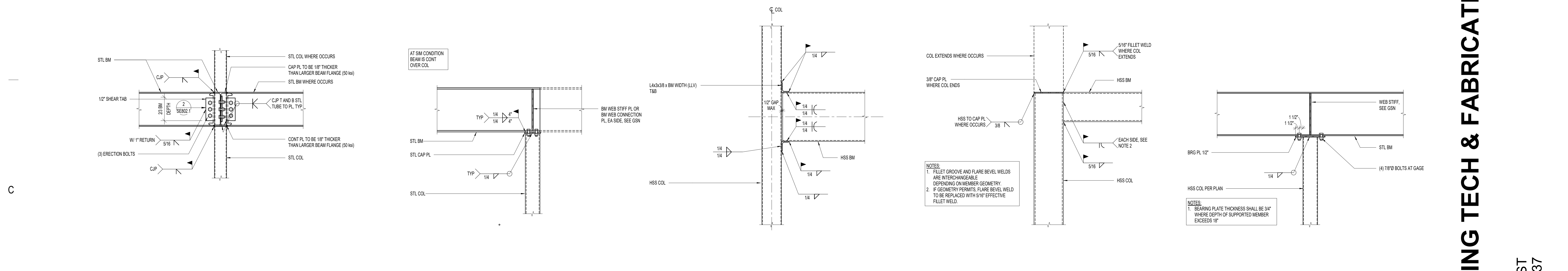
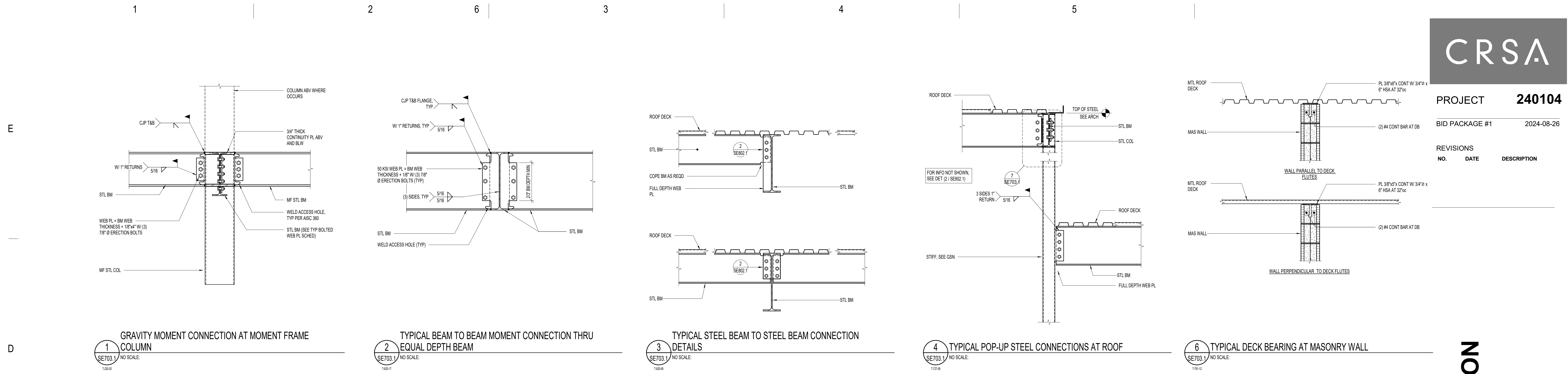


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ROOF FRAMING
 DETAILS
SE702.1

NO.	DATE	DESCRIPTION



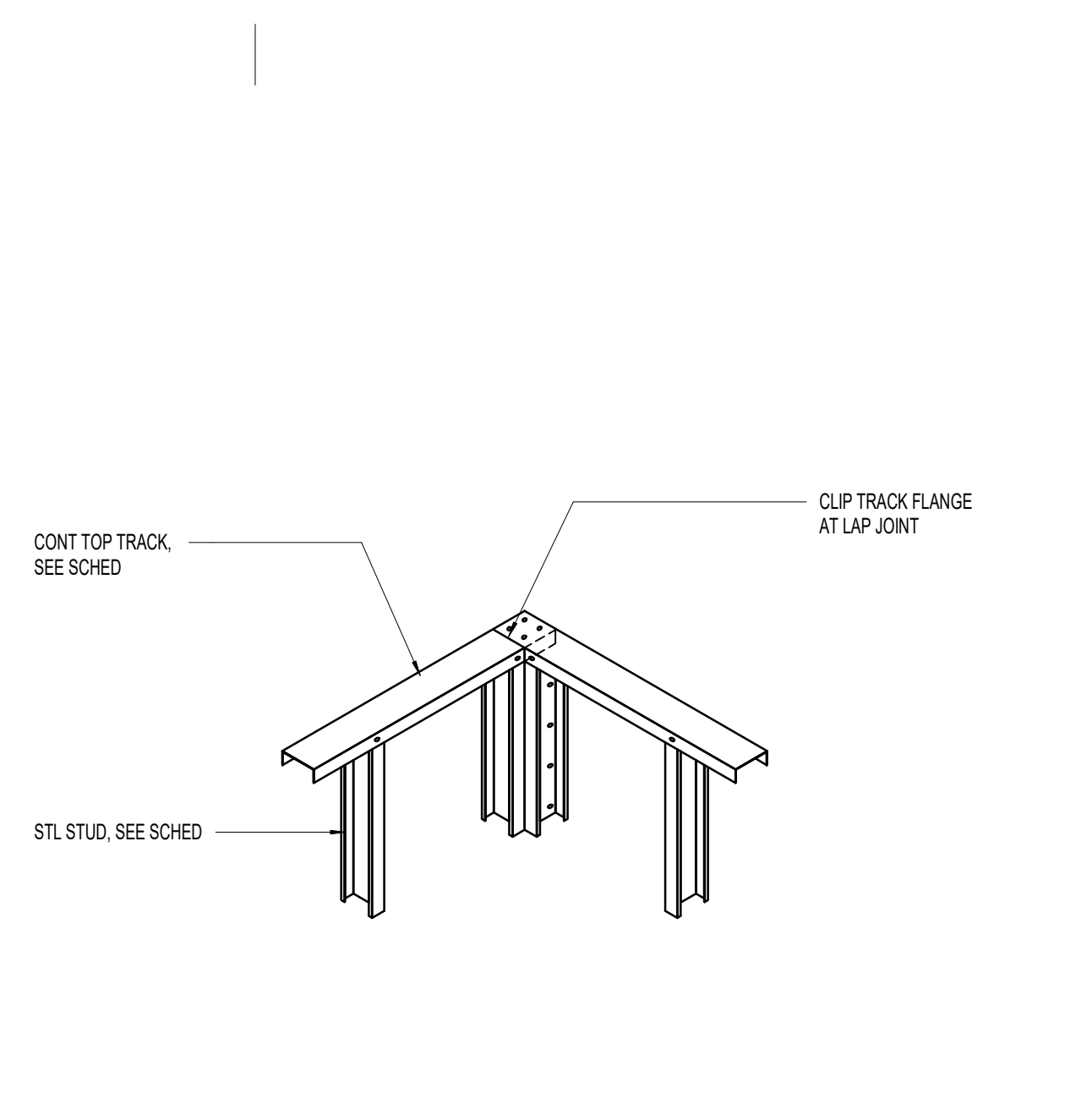
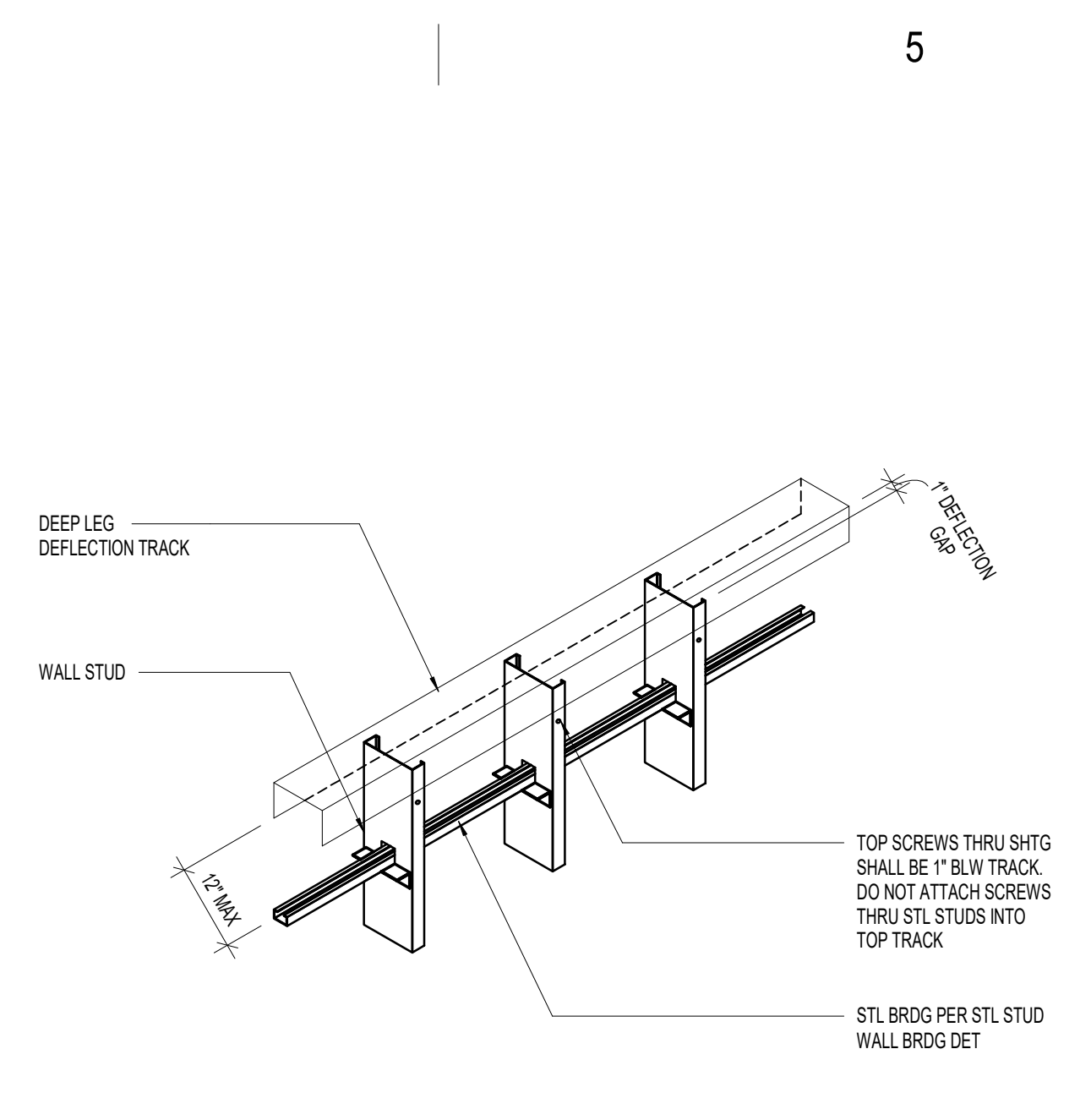
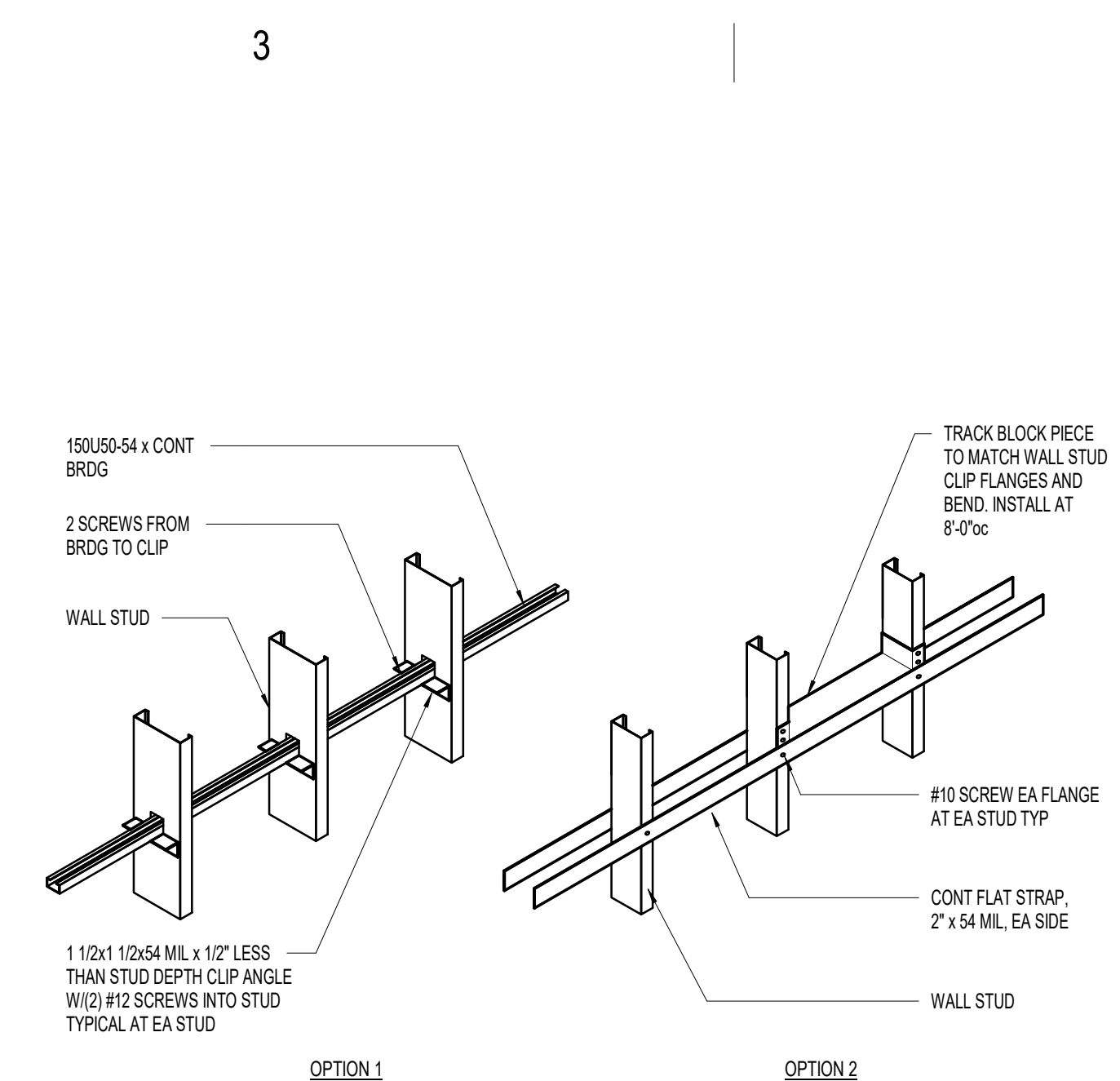
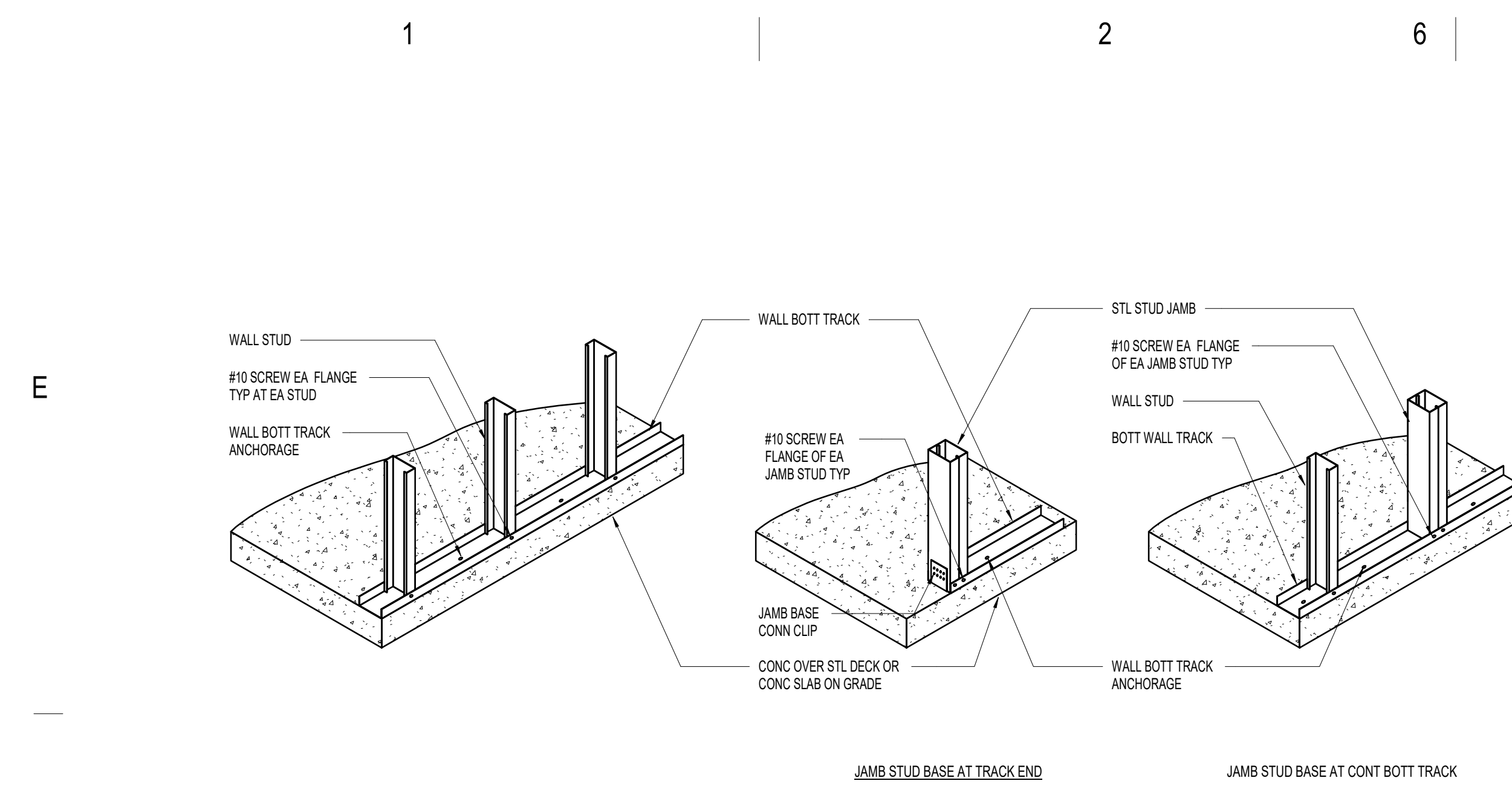
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ROOF FRAMING
 DETAILS
SE703.1
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1 TYPICAL BOTTOM TRACK ANCHORAGE DETAILS

2 TYPICAL STEEL STUD WALL BRIDGING DETAIL

3 TYPICAL STEEL STUD WALL DEFLECTION TRACK ASSEMBLY DETAIL - SINGLE & DOUBLE TRACK

4 TYPICAL WALL FRAMING AT CORNER TRACK LAP CONNECTION

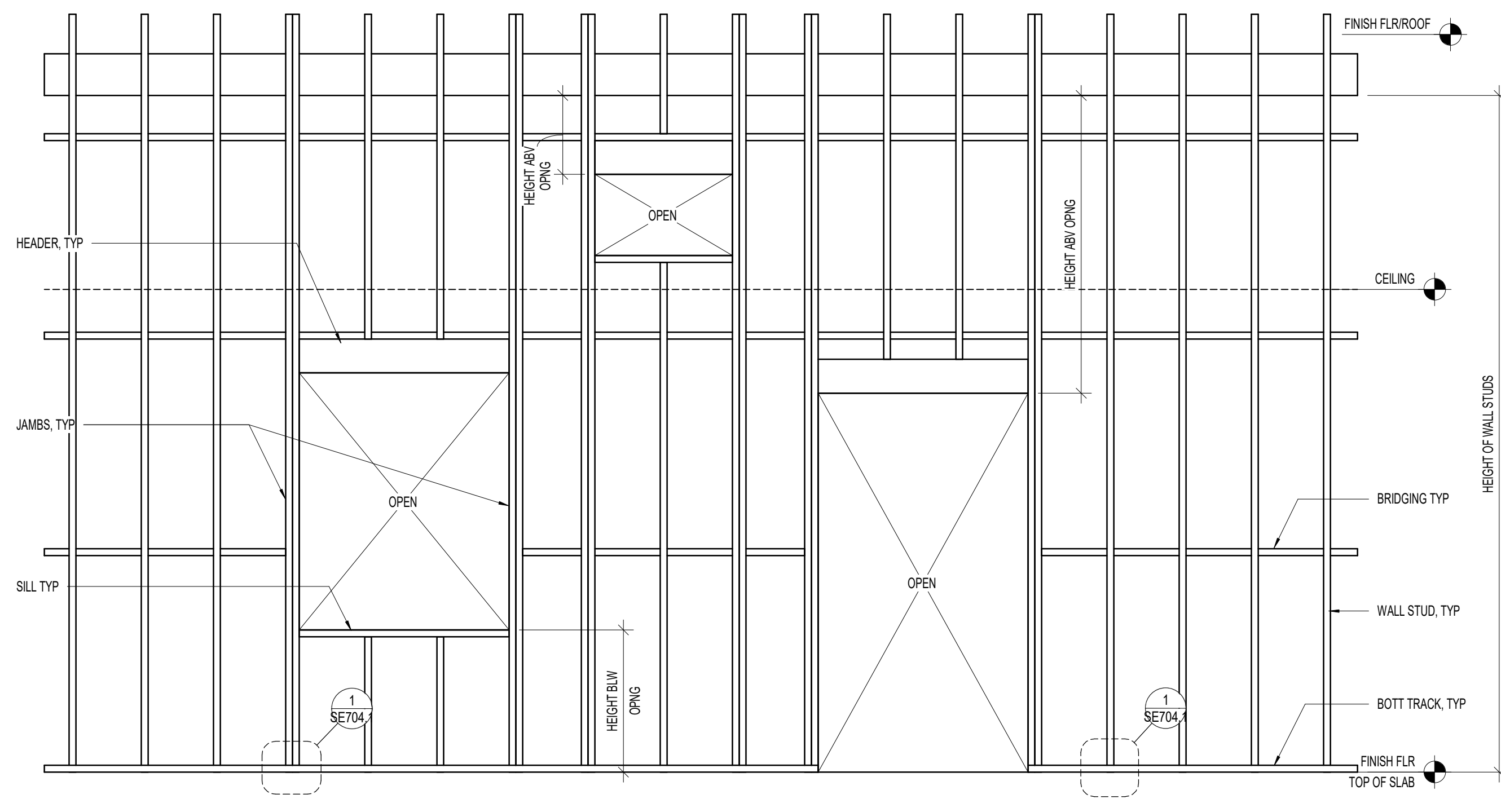
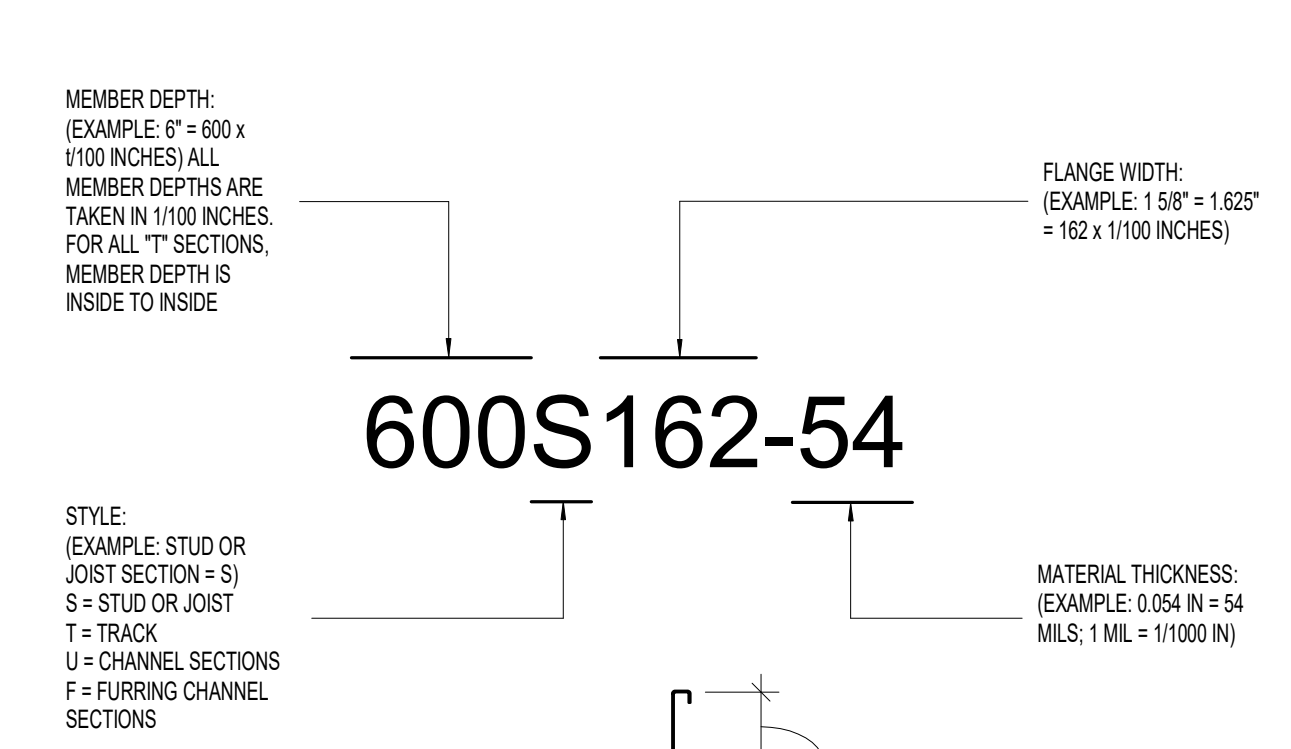
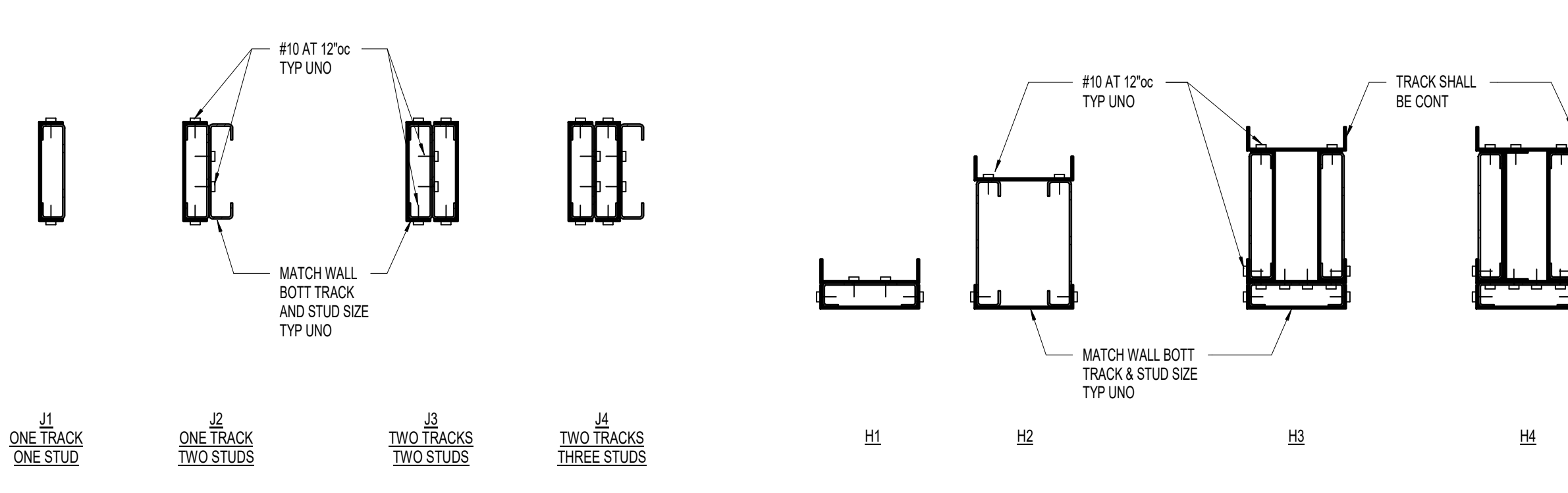
E

D

C

B

A



5 TYPICAL STEEL STUD JAMB DETAILS

6 TYPICAL STUD HEADER DETAILS

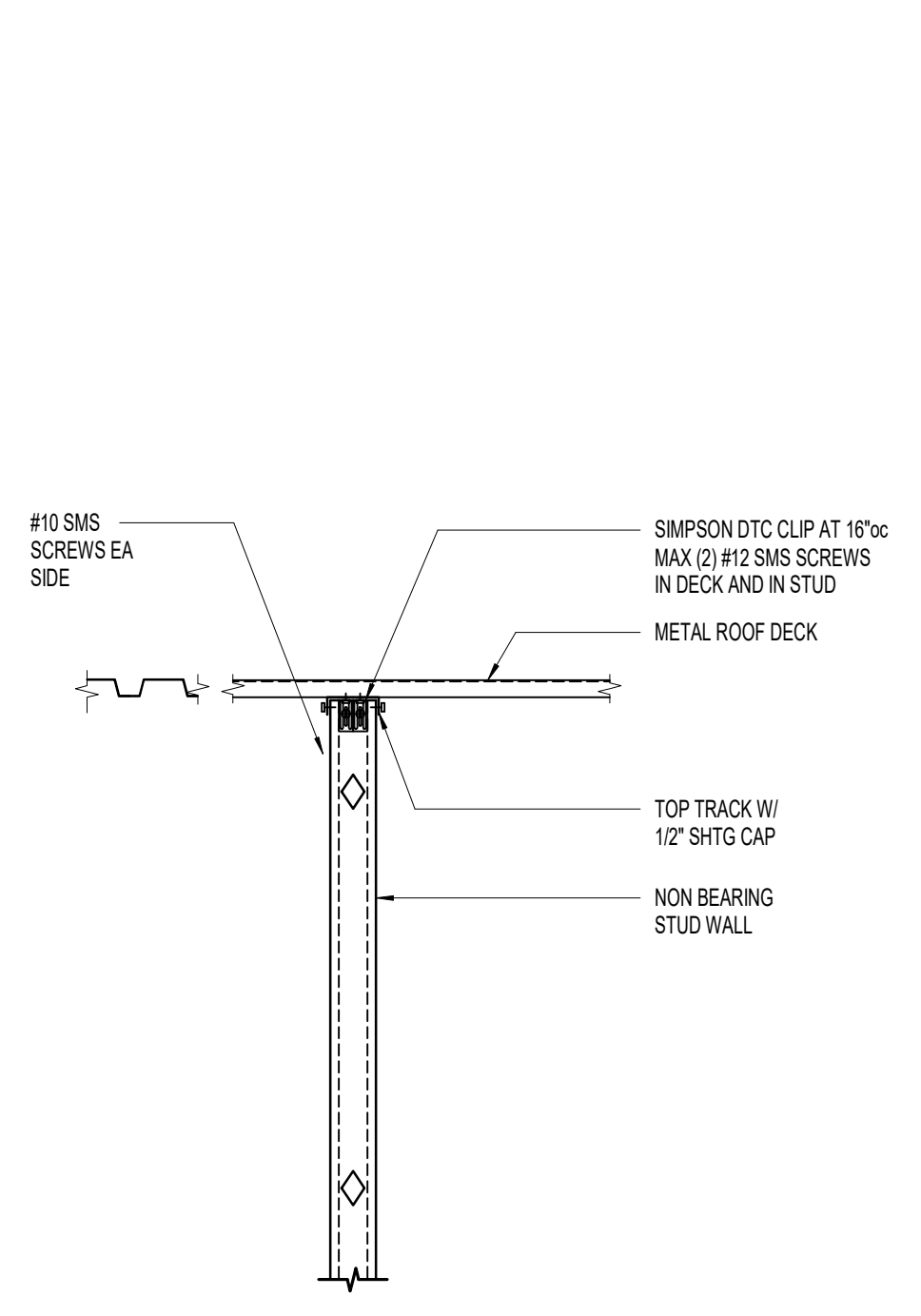
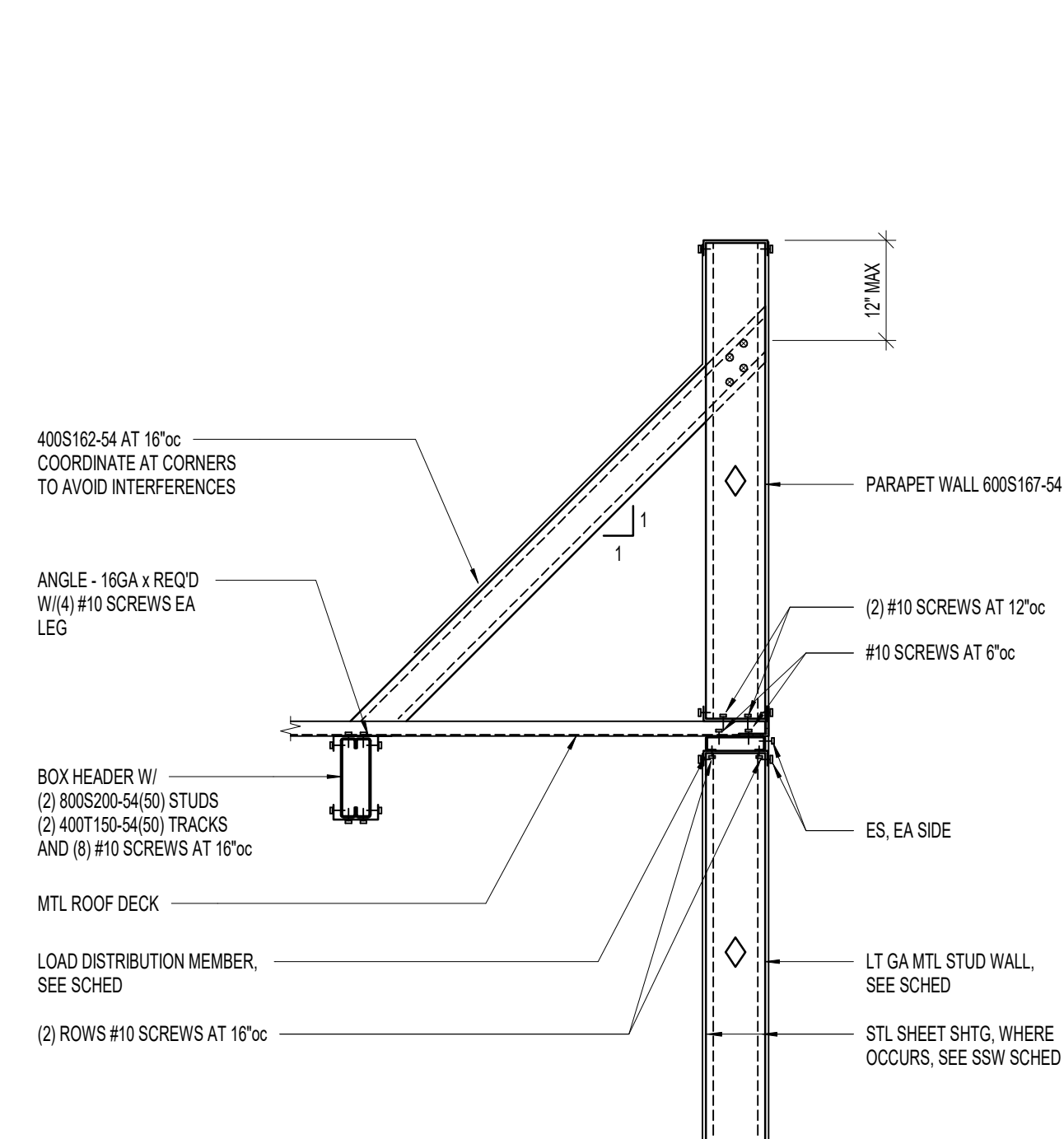
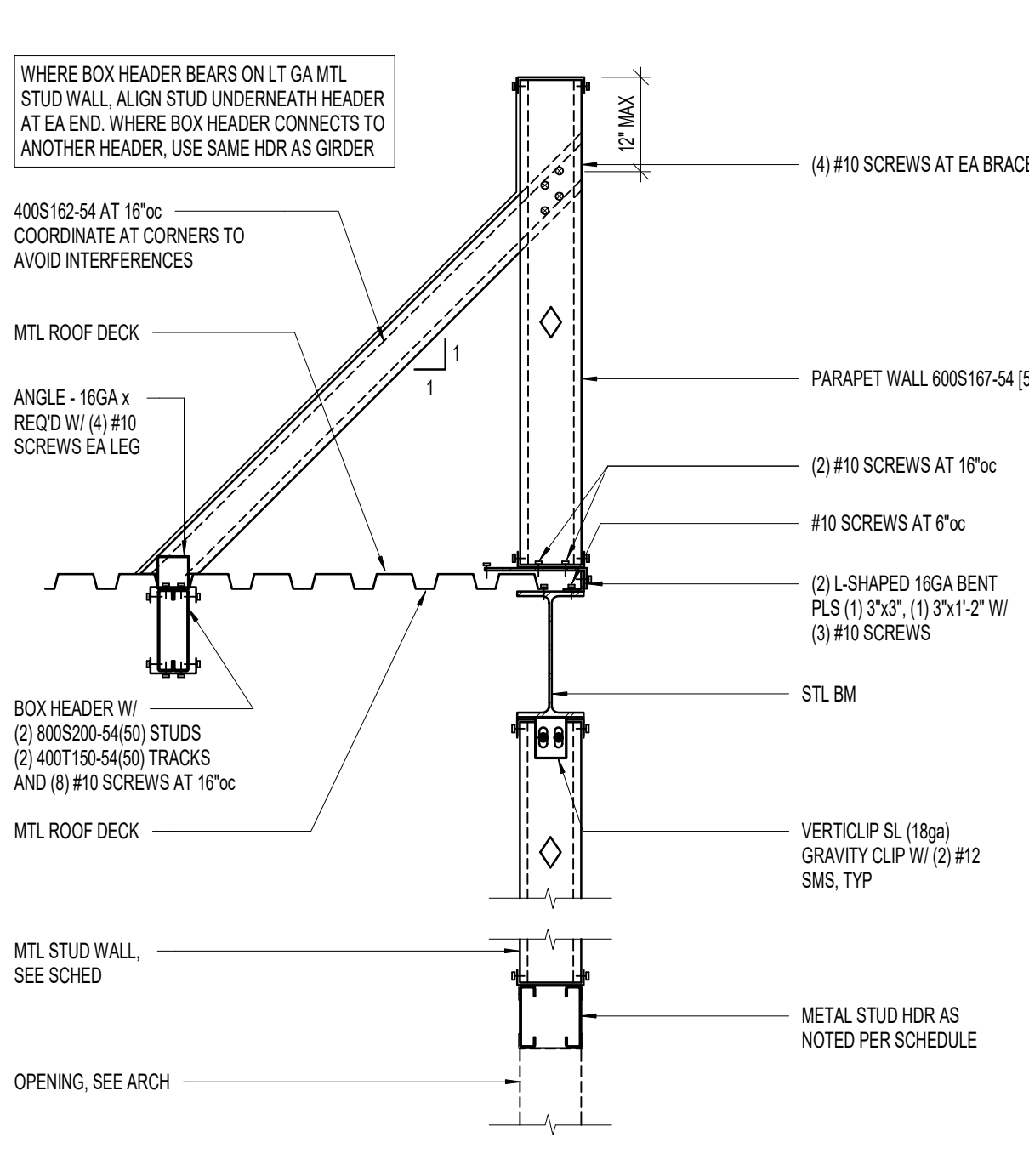
7 STEEL STUD MANUFACTURER'S ASSOCIATION NOMENCLATURE

8 TYPICAL EXTERIOR WALL OPENING FRAMING ELEVATION

C

B

A



9 PARAPET WITH DECK PARALLEL TO WALL

10 PARAPET WITH DECK PERPENDICULAR TO WALL

11 TYPICAL NON-BEARING STEEL STUD TO DECK CONNECTION

B

A

A

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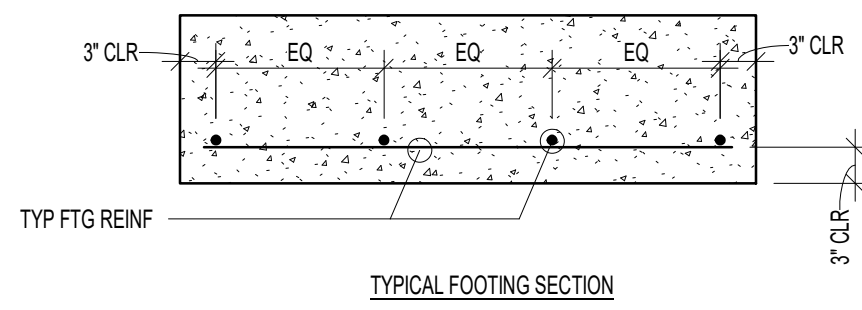
NOTE: THESE STRUCTURAL DRAWINGS ARE BASED ON ARCHITECTURAL DRAWINGS DATED July 23, 2024. DIMENSIONS AND ELEVATIONS AS THEY RELATE TO THE BUILDING IN GENERAL, IN GRID TO GRID DIMENSIONS OR DECK BEARING ELEVATIONS, ARE SUPPLIED BY THE ARCHITECT. THEY ARE PROVIDED ON THE STRUCTURAL PLANS AND DETAILS FOR THE CONVENIENCE OF THE CONTRACTOR. VERIFY DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS.

STEEL STUD
FRAMING DETAILS
SE704.1

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8/20/2024 12:18:03 PM

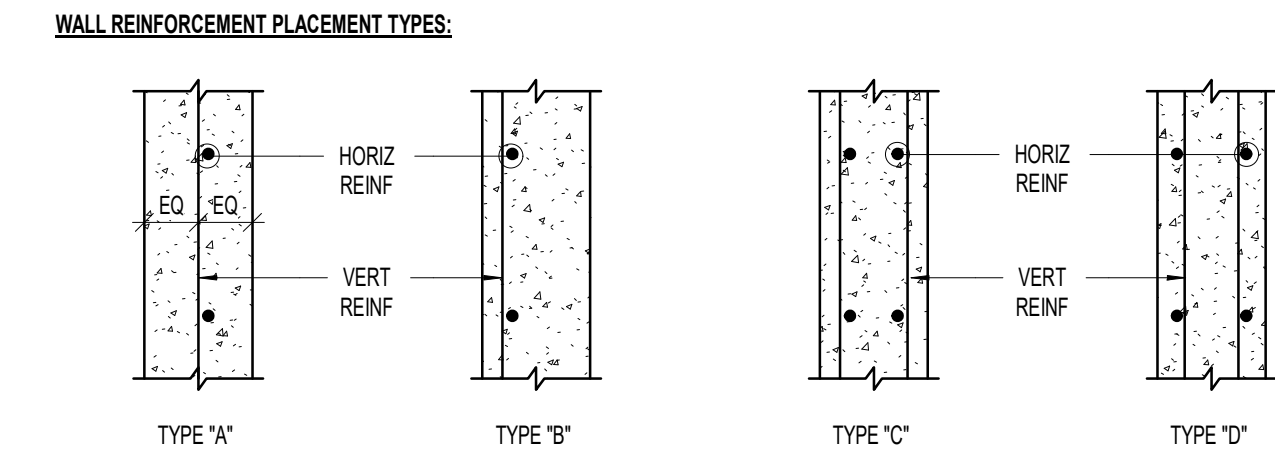
CONCRETE FOOTING SCHEDULE												
MARK	WIDTH	LENGTH	THICKNESS	REINFORCING CROSSWISE				REINFORCING LENGTHWISE				COMMENTS
				NO	SIZE	LENGTH	SPACING	NO	SIZE	LENGTH	SPACING	
FC2.0	2'-0"	12'	12"	CONT	12"			2	#5	CONT	EQ	
FC3.0	3'-0"	12'	12"	CONT	12"	#5	2'-0"	3	#5	CONT	EQ	TOP AND BOTT
FC4.0	4'-0"	12'	12"	CONT	12"	#5	3'-0"	4	#5	CONT	EQ	TOP AND BOTT
FC4.5	4'-6"	12'	12"	CONT	12"	#5	4'-0"	5	#5	CONT	EQ	TOP AND BOTT
FS3.5	3'-6"	3'-6"	12"	3	#5	3'-0"	EQ	3	#5	3'-0"	EQ	
FS5.0	5'-0"	5'-0"	12"	5	#5	4'-0"	EQ	5	#5	4'-0"	EQ	
FS6.5	6'-6"	6'-6"	12"	6	#5	6'-0"	EQ	6	#5	6'-0"	EQ	
FS7.0	7'-0"	7'-0"	12"	7	#5	6'-0"	EQ	7	#5	6'-0"	EQ	



- CONCRETE FOOTING NOTES:**
- PLACE ALL FOOTING REINFORCING IN BOTTOM OF FOOTING WITH 3" CLEAR CONCRETE COVER, UNLESS NOTED OTHERWISE.
 - TOP REINFORCING, WHERE SPECIFIED, SHALL BE PLACED IN THE TOP OF THE FOOTING WITH 2" MINIMUM CONCRETE COVER.
 - IF FOOTINGS ARE EARTH FORMED, FOOTING WIDTH AND LENGTH SHALL BE 6" WIDER AND LONGER THAN SCHEDULED.
 - SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER REQUIREMENTS.
 - NOT ALL FOOTINGS ARE USED. SEE FOUNDATION PLAN FOR FOOTING MARKS.
 - RUN CONTINUOUS BARS IN 'C' FOOTINGS THROUGH INTERSECTED 'S' FOOTINGS, AND TO THE FAR SIDE OF INTERSECTED 'C' FOOTINGS (NO CORNER BARS REQUIRED).
 - EXTEND CONTINUOUS FOOTINGS 1'-0" BEYOND END OF WALL, EXCEPT AT INTERSECTING CORNERS OR UNO ON PLAN.
 - FOOTINGS MAY BE THICKER THAN THE SCHEDULED BERTH IN AREAS SURROUNDING ANCHOR BOLTS OR HOLD DOWNS, SEE ANCHORAGE AND HOLD DOWN DETAILS.
 - IN 'C' FOOTINGS CROSSWISE BAR SHALL BE BELOW THE LENGTHWISE BAR.

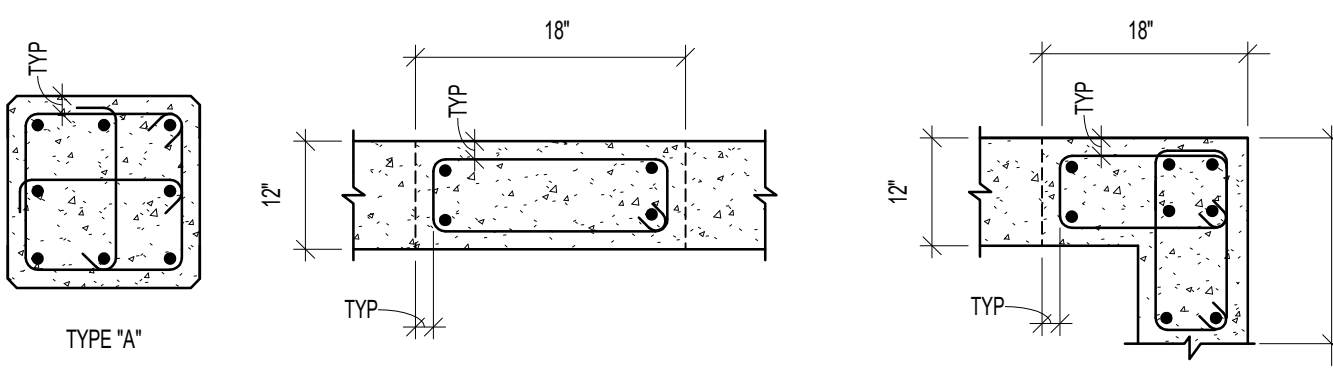
CONCRETE WALL SCHEDULE						
MARK	THICKNESS	REINFORCING			WALL TYPE	COMMENTS
		VERTICAL	HORIZONTAL	TOP AND BOTTOM		
CW-08	8"	(1) #4 AT 16"oc	(1) #4 AT 12"oc	(1) #4	A	TYP WALL
CW-12	12"	(2) #4 AT 16"oc	(2) #4 AT 12"oc	(2) #4	C	AT OFFICES

- CONCRETE WALL NOTES:**
- SEE GENERAL STRUCTURAL NOTES FOR COVER AND OTHER REQUIREMENTS NOT NOTED IN SCHEDULE.
 - CONCRETE WALLS NOT DESIGNATED ON THE PLANS SHALL BE REINFORCED AS FOLLOWS:
- | | | |
|-----------|--------------------------|--------------------------|
| THICKNESS | VERTICAL REINFORCING | HORIZONTAL REINFORCING |
| 8" | #4 BARS AT 16"oc | #4 BARS AT 16"oc |
| 8" | #4 BARS AT 16"oc | #4 BARS AT 12"oc |
| 10" | #4 BARS AT 16"oc | #5 BARS AT 15"oc |
| 12" | #4 BARS AT 16"oc EA FACE | #4 BARS AT 16"oc EA FACE |
- PLACE STEEL IN THE CENTER OF THE WALL (EXCEPT TYPE 'B' AND RETAINING WALLS). WALLS THICKER THAN 10" SHALL HAVE TWO CURTAINS OF REINFORCEMENT (PLACED NEAR EA FACE OF THE WALL), UNLESS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS.



CONCRETE PIER SCHEDULE					
MARK	PIER SIZE	REINFORCING		TYPE	COMMENTS
		VERTICAL	TIES		
CP-1	16' x 16'	(8) #6	(1) #3 AT 4'oc	A	AT CANOPY
CP-2	12' x 18'	(4) #6	(1) #3 AT 4'oc	B	IN WALL
CP-3	12' x 18'	(8) #6	(1) #3 AT 4'oc	C	IN WALL AT CORNER

- CONCRETE PIER NOTES:**
- INSTALL (3) SETS OF TIES WITHIN THE TOP 5' AT THE TOP OF ALL PIERS (UNO).
 - ALTERNATE POSITION OF HOOKS IN PLACING SUCCESSIVE SETS OF TIES.



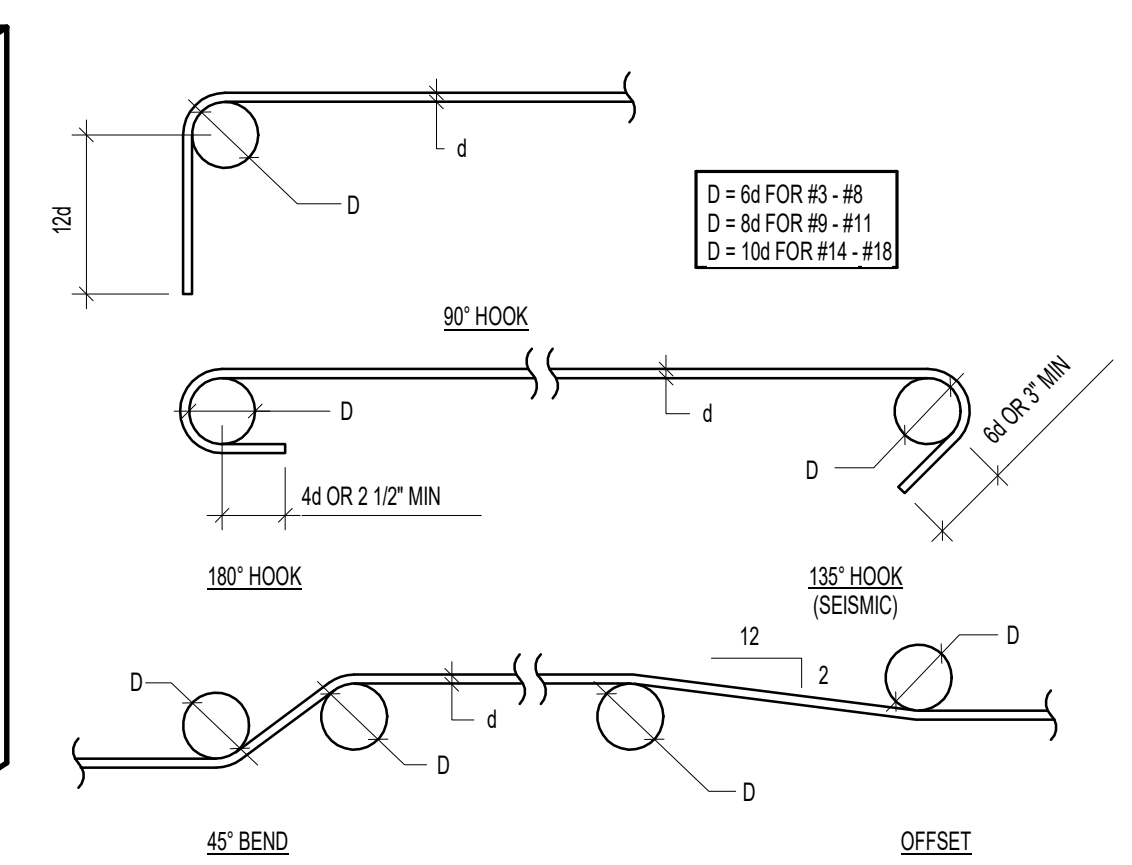
1 CONCRETE FOOTING SCHEDULE
SE801.1 NO SCALE
10/08

2 CONCRETE WALL SCHEDULE
SE801.1 NO SCALE
10/08

3 CONCRETE PIER SCHEDULE
SE801.1 NO SCALE
10/14

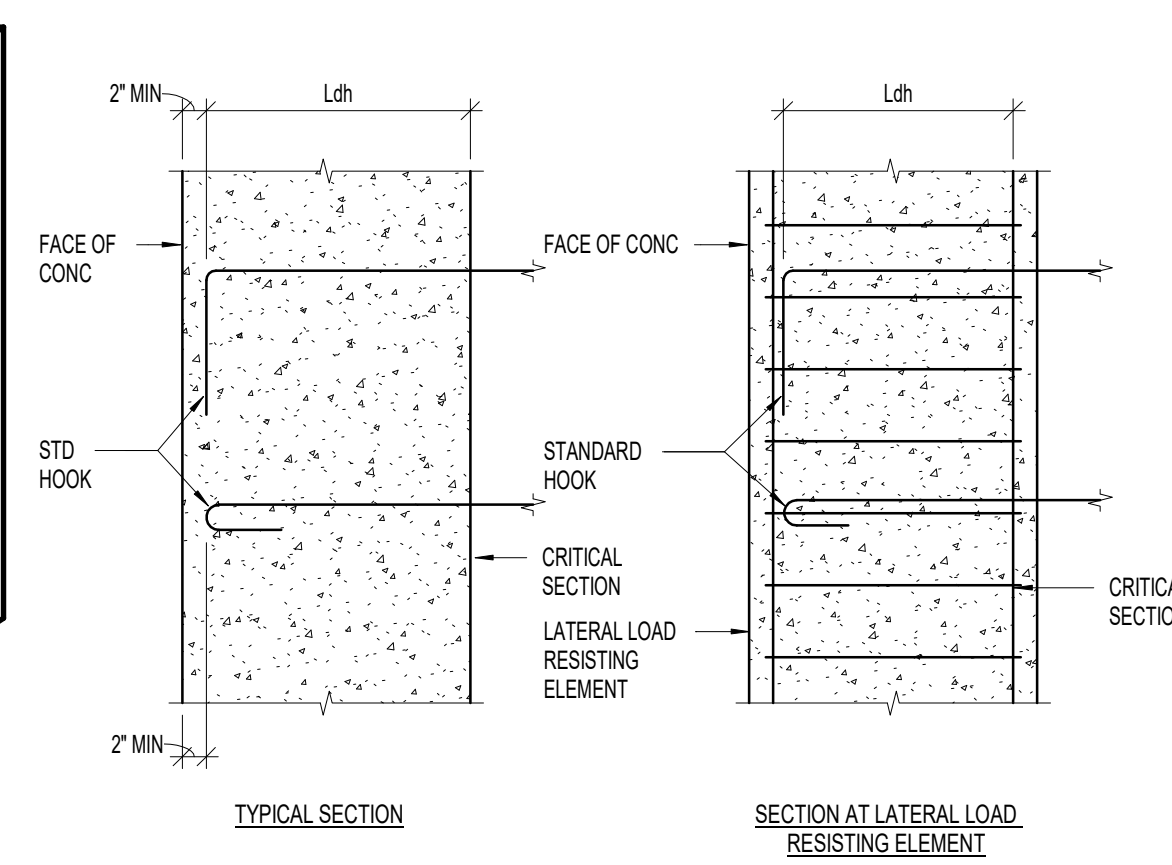
CONCRETE REINFORCING BAR LAP SPlice SCHEDULE																								
BAR SIZE	f _c = 3000 PSI				f _c = 3500 PSI				f _c = 4000 PSI				f _c = 4500 PSI				f _c = 5000 PSI				f _c = 6000 PSI			
	REGULAR		TOP		REGULAR		TOP		REGULAR		TOP		REGULAR		TOP		REGULAR		TOP		REGULAR		TOP	
	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	
#3	17"	22"	22"	28"	16"	21"	21"	26"	15"	19"	25"	14"	18"	19"	23"	17"	22"	12"	16"	16"	20"	16"	20"	
#4	22"	29"	29"	38"	21"	27"	27"	36"	19"	25"	33"	18"	24"	24"	31"	17"	23"	23"	29"	16"	21"	21"	27"	
#5	28"	36"	36"	47"	26"	34"	34"	44"	24"	31"	41"	22"	28"	28"	36"	22"	28"	28"	36"	20"	26"	26"	33"	
#6	33"	43"	43"	56"	31"	40"	40"	52"	29"	37"	47"	27"	35"	35"	46"	26"	34"	34"	44"	24"	31"	31"	40"	
#7	48"	63"	63"	81"	45"	59"	59"	75"	42"	54"	64"	37"	49"	49"	63"	36"	48"	48"	63"	34"	45"	45"	58"	
#8	55"	72"	72"	93"	51"	67"	67"	82"	48"	62"	74"	45"	59"	59"	76"	43"	58"	58"	72"	39"	51"	51"	66"	
#9	62"	81"	81"	105"	58"	75"	75"	98"	54"	70"	84"	51"	66"	66"	86"	48"	63"	63"	81"	44"	57"	57"	74"	
#10	70"	91"	91"	118"	65"	85"	85"	110"	61"	79"	92"	57"	74"	74"	96"	54"	71"	71"	92"	50"	64"	64"	84"	
#11	78"	101"	101"	131"	73"	94"	94"	122"	67"	87"	104"	64"	82"	82"	107"	60"	78"	78"	102"	55"	71"	71"	93"	

- NOTES:**
- THIS SCHEDULE SHALL BE USED FOR ALL SPLICES, UNLESS NOTED OTHERWISE.
 - HORIZONTAL BARS ARE CLASSIFIED AS TOP BARS WHERE 12" OR MORE OF FRESH CONCRETE IS CAST BELOW THE REINFORCING BARS.
 - CLASS 'B' SPLICES SHALL BE USED FOR ALL SPLICES UNLESS NOTED OTHERWISE.
 - TIES AND STRIPPERS SHALL NOT BE SPLICED.
 - FOR ALL LIGHTWEIGHT CONCRETE, LAP LENGTHS SHALL BE MULTIPLIED BY 1.3.
 - FOR ALL EPOXY COATED BARS, LAP LENGTHS SHALL BE MULTIPLIED BY 1.5 FOR BARS WITH CLEAR COVER LESS THAN 3 BAR DIAMETERS OR CLEAR SPACING LESS THAN 6 BAR DIAMETERS, OTHERWISE MULTIPLY BY 1.2.
 - LAP LENGTHS SHALL BE MULTIPLIED BY 1.5 AT SHEAR WALL BOUNDARY ELEMENTS.
 - DEVELOPMENT LENGTH l_d IS EQUAL TO CLASS 'A' SPICE.
 - IF REINFORCING HAS CLEAR COVER LESS THAN ONE BAR DIAMETER, LAP LENGTHS SHALL BE MULTIPLIED BY 1.5.
 - IF REINFORCING IS NOT ENCLOSED IN TIES OR STRIPPERS AND IS SPACED TIGHTER THAN 2 BAR DIAMETERS ON CENTER, LAP LENGTHS SHALL BE MULTIPLIED BY 1.5.
 - LAP LENGTHS SHALL BE MULTIPLIED BY 1.33 FOR GRADE 60 REBAR.
 - WHERE BARS OF DIFFERENT SIZES ARE LAPPED, THE SPICE LENGTH SHALL BE THE LARGER OF l_d OF THE LARGER BARS AND THE SPICE LENGTH OF THE SMALLER BAR.



HOOKED BAR DEVELOPMENT LENGTHS, l_{dh}						
BAR SIZE	f _c = 3000 PSI	f _c = 4000 PSI	f _c = 4500 PSI	f _c = 5000 PSI	f _c = 6000 PSI	
#3	9"	8"	7"	7"	6"	
#4	11"	10"	9"	9"	8"	
#5	14"	12"	12"	11"	10"	
#6	17"	15"	14"	13"	12"	
#7	20"	17"	16"	15"	14"	
#8	22"	19"	18"	17"	16"	
#9	25"	22"	21"	20"	18"	
#10	28"	25"	23"	22"	20"	
#11	31"	27"	26"	24"	22"	

- NOTES:**
- FOR GRADE 60 REBAR, MULTIPLY LENGTHS BY 1.33.
 - FOR LIGHTWEIGHT CONCRETE, MULTIPLY LENGTHS BY 1.3.
 - FOR EPOXY COATED REINFORCEMENT, MULTIPLY LENGTHS BY 1.2.
 - FOR HOOKS WITH 2" MINIMUM SIZE COVER PERPENDICULAR TO PLANE OF HOOK, MULTIPLY LENGTHS BY 0.7.
 - FOR LATERAL LOAD RESISTING ELEMENTS, CRITICAL SECTIONS SHALL BE TAKEN AS THE FACE OF THE HOOK AT CORNER CORES OF COLUMN JOINTS OR SHEAR WALL BOUNDARY ZONE.



4 CONCRETE REINFORCING BAR LAP SCHEDULES AND DIAGRAMS
SE801.1 NO SCALE
10/08



PROJECT **240104**
BID PACKAGE #1 2024-08-26

REVISIONS
NO. DATE DESCRIPTION

DTC WELDING TECH & FABRICATION
BUILDING
355 SOUTH 650 EAST
KAYSVILLE, UT 84037



2024-08-26
BID PACKAGE #1

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CONCRETE SCHEDULES
SE801.1

(801) 355-5915

STEEL COLUMN SCHEDULE

STEEL COLUMN NOTES:

- ALL ANCHOR BOLTS SHALL BE ASTM F1554 GRADE 55 (AT CONTRACTOR'S OPTION, GRADE 56 ANCHORS MAY BE SUBSTITUTED WITH GRADE 56 ANCHORS), UNLESS NOTED OTHERWISE. ALL COLUMNS SHALL BE INSTALLED WITH HEADED (OR DOUBLE NUT) ANCHOR BOLTS. PROJECT ANCHOR BOLTS 4" MINIMUM ABOVE THE TOP OF THE BASE PLATE.
- ANCHOR BOLTS SHALL NOT BE WELDED (INCLUDING TACK WELDS).
- IF DESIRED SPLICE LOCATIONS DIFFER FROM THOSE LEVELS SHOWN ON PLAN, NOTIFY STRUCTURAL ENGINEER PRIOR TO FABRICATION. WRITTEN APPROVAL REQUIRED.
- ALL CAP PLATE BOLTS SHALL BE 3/4" BOLT SIZE, TYPICAL, UNLESS NOTED OTHERWISE.
- ALL CAP PLATES TO BE 3/4" THICK UNO.
- SEE (2) (SEE 9.1) FOR COLUMNS WRAPPED IN CONCRETE.
- SEE GENERAL STRUCTURAL NOTES FOR OTHER REQUIREMENTS.
- ERECTION AIDS TO BE REMOVED AFTER COLUMN SPLICING.
- FOR HSS 14x14 AND HSS 18x18 COLUMNS, ANCHOR BOLTS SHOULD BE 1" DIAMETER WITH 12" EMBEDS WITH 3/8"x2" PLATE WASHERS. FOR HSS 12x12 COLUMNS OR SMALLER, ANCHOR BOLTS SHOULD BE 3/4" WITH 1" EMBEDS WITH 1/4"x2" PLATE WASHERS. FOR HSS 10x10 COLUMNS OR SMALLER HSS COLUMNS, ASTM F1554 GRADE 36 RODS MAY BE SUBSTITUTED.
- OVERSIZED HOLES MAY BE USED IN BASE PLATES PROVIDED THEY DO NOT EXCEED THE FOLLOWING SIZES:
3/4" BOLT ≤ 1 1/8"
1" BOLT ≤ 1 1/2"
1 1/4" BOLT ≤ 1 7/8"
1 1/2" BOLT ≤ 2 1/8"
- HOLES IN PLATE WASHERS SHOULD BE 1/8" GREATER THAN BOLT DIMENSION.
- NON-SHRINK GROUT UNDER BASE PLATES SHALL BE 1 1/2" THICK UNO.
- COLUMN LOCATIONS SHOWN ON SCHEDULES ARE APPROXIMATE. PLEASE SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS.
- DO NOT WELD IN WIDE FLANGE "K" ZONES. WELDS ARE NOT REQUIRED AT "K" ZONES.
- WELDING OF PLATE WASHERS TO BASE PLATES MAY BE OMITTED AT SINGLE STORY COLUMNS AND HSS 8x8 COLUMNS OR SMALLER.

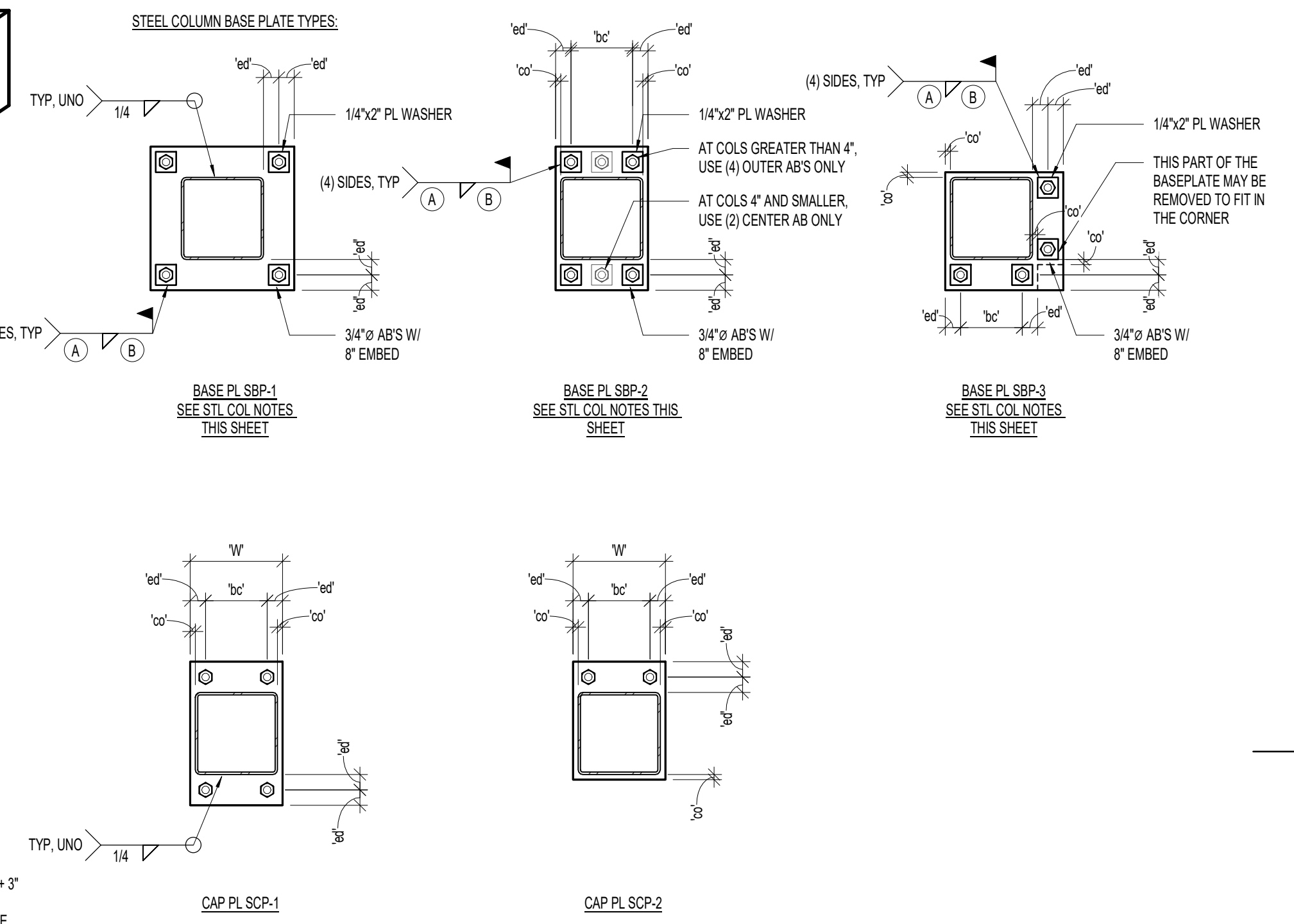
BASE PL LEGEND:

1" = 1/2" MINIMUM
1 1/2" = 1 1/2" MINIMUM
1 1/2" = 2" MINIMUM
1 1/2" = BEAM OR GIRDER GAGE + 3"
OR
BEAM OR GIRDER FLANGE WIDTH + 1"
OR
COLUMN DEPTH + 1"
WHICHEVER IS GREATER

CAP PL LEGEND:

1" = BEAM OR GIRDER GAGE + 3"
OR
BEAM OR GIRDER FLANGE WIDTH + 1"
OR
COLUMN WIDTH + 1"
WHICHEVER IS GREATER

BASE PL ELEVATION



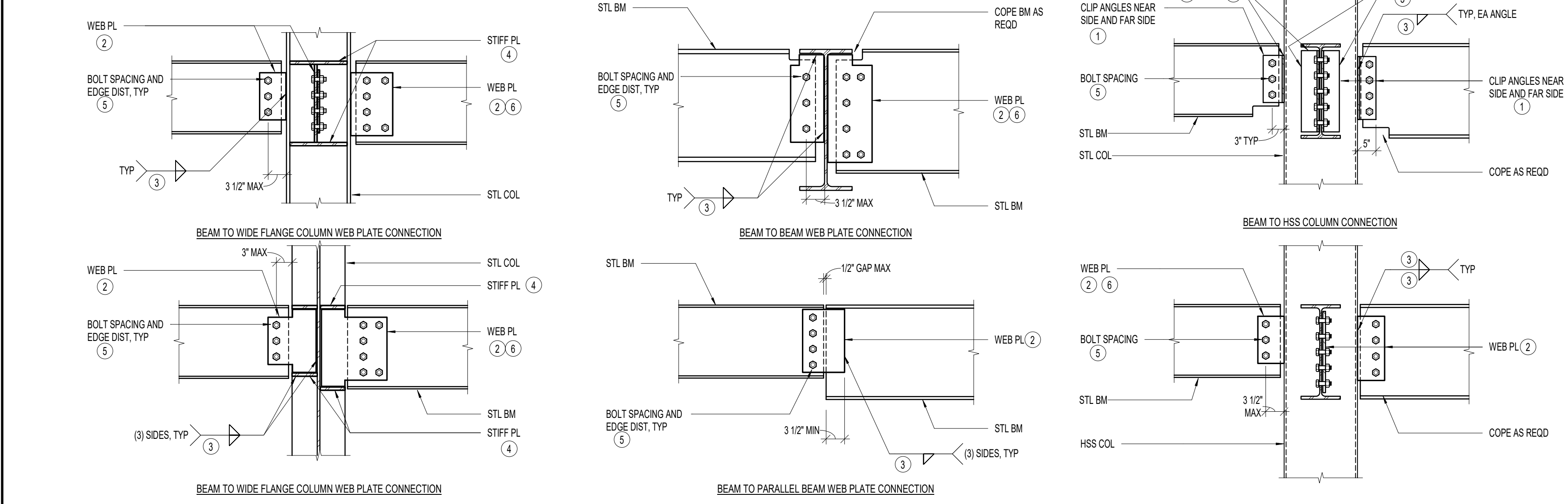
1 STEEL COLUMN SCHEDULE FOR SEISMIC DESIGN CATEGORIES C THRU F

A-325 BOLT SCHEDULE

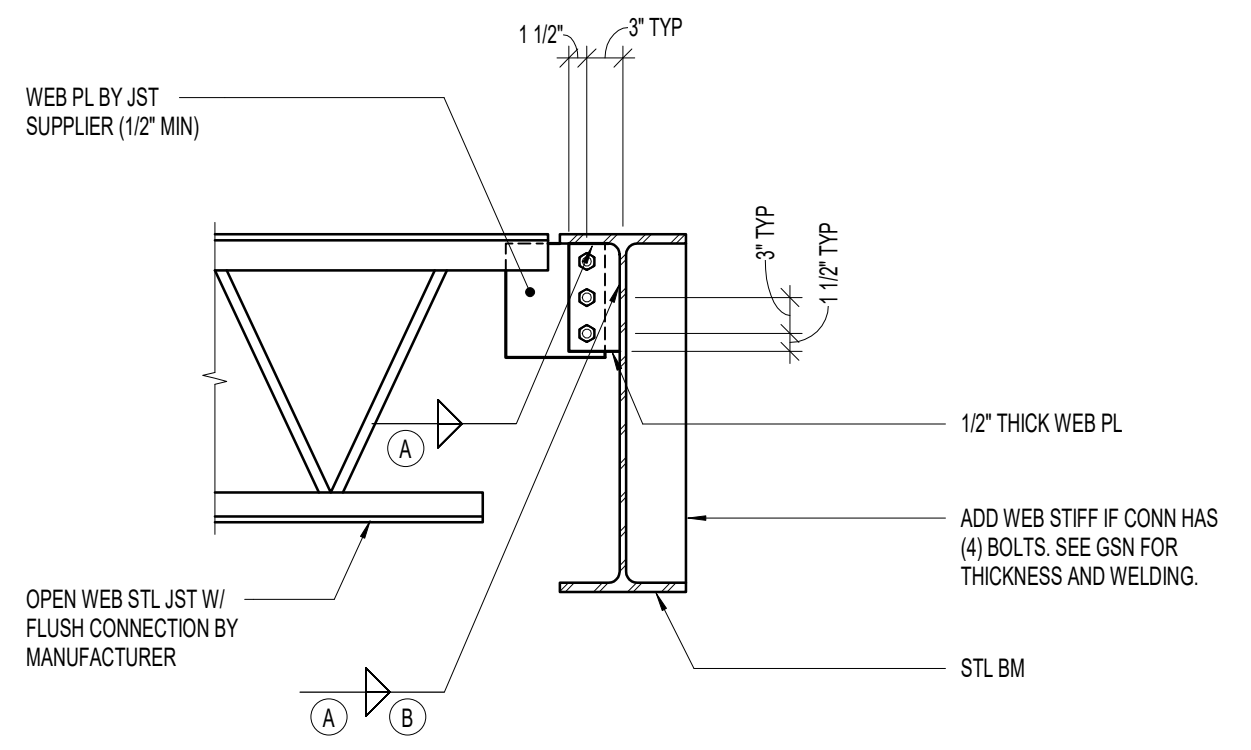
MAXIMUM BEAM SIZE IN EACH BEAM DEPTH GROUP	A-325 BOLTS	SIZE
W8	2	7/8"
W10	2	7/8"
W12	3	7/8"
W14	3	7/8"
W16	4	7/8"
W18	5	7/8"
W21	6	7/8"
W24	6	7/8"
W27	7	7/8"

NOTES:

- CLIP ANGLES: 1/2" THICKNESS SHALL BE EQUAL TO ONE HALF THE BEAM WEB THICKNESS PLUS 1/16" (1/4" MIN.) FOR TWO ROWS OF BOLTS OR SKEWED CONNECTIONS. USE BOLT PLATES WHERE COLUMN WIDTH IS SMALLER THAN THE CONNECTING CLIP ANGLES. ANGLE LESS SHALL BE REDUCED TO MATCH WIDTH OF COLUMN.
- BEAM WEB CONNECTION PLATE THICKNESS EQUALS 3/8" MINIMUM THICK FOR W8 BEAMS OR SMALLER, 1/2" MINIMUM THICK FOR W10 BEAMS OR LARGER, 3/4" MINIMUM THICK FOR BEAMS WITH WEB GREATER THAN 1" THICK.
- FILLET WELDS SHALL BE AS FOLLOWS:
1/4" FOR 3/8" PLATES
5/16" FOR 1/2" PLATES
7/16" FOR 3/4" PLATES
- THICKNESS EQUALS BEAM FLANGE THICKNESS OF BEAM FRAMING INTO COLUMN WEB (3/8" MINIMUM).
- BOLT EDGE DISTANCE SHALL BE 1 1/2" MINIMUM AT ALL EDGES. BOLT SPACING SHALL BE AT 3" BOLT SPACING MAY BE REDUCED TO 3x THE BOLT DIAMETER IF IT IS REQUIRED FOR A SINGLE ROW OF BOLTS. A SINGLE ROW OF BOLTS IS PREFERRED.
- WHEN MORE THAN ONE COLUMN OF BOLTS IS NEEDED, THE FIRST COLUMN SHALL BE COMPLETE WITH THE REMAINDER OF THE BOLTS PLACED IN THE SECOND COLUMN.
- 1/2" PLATE THICKNESS + 5/16"



2 TYPICAL BOLTED WEB PLATE CONNECTIONS WITH BOLT SCHEDULE (SINGLE SHEAR)



FLUSH FRAME OPEN WEB STEEL JOIST TO BEAM CONNECTION SCHEDULE

LH SIZE	NUMBER OF BOLTS	SIZE OF WELD (A)	LENGTH OF WELD (B)
18LH to 24LH	2	3/16	4
24LH to 32LH	3	3/16	6
36LH to 48LH	4	3/16	9

1. THE ABOVE SCHEDULE IS BASED ON HIGHEST CATALOG VALUES. IF LOADING EXCEEDS THAT OF THE CATALOG OR IS A SPECIAL JOIST TYPE, THE ABOVE VALUES DO NOT APPLY.
2. SCHEDULE VALUES AND CAPACITIES BASED ON 70x10 AISC BOLTS.
3. CONTRACTOR TO COORDINATE CONNECTION BETWEEN JOIST SUPPLIER AND STEEL SUPPLIER.

3 TYPICAL FLUSH FRAMED JOIST CONNECTION

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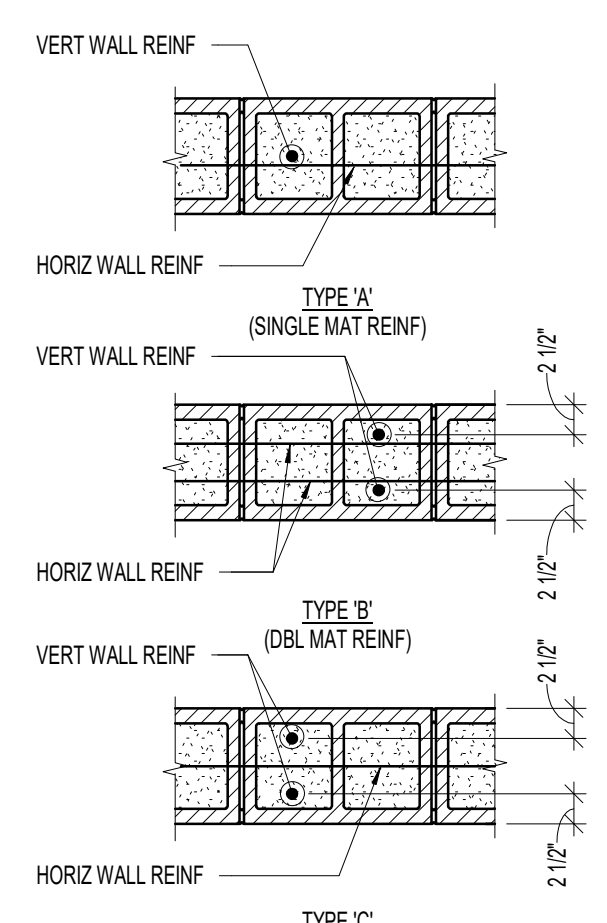
STEEL SCHEDULES
SE802.1

(801) 355-5915

MASONRY WALL SCHEDULE							
MARK	THICKNESS	MATERIALS	SOLID GROUT	REINFORCING			
				VERTICAL	HORIZONTAL	TYPE	JOINT REINF
MW-08	8"	CMU	NO	SEE MAS WALL ELEV	SEE MAS WALL ELEV	A	NO

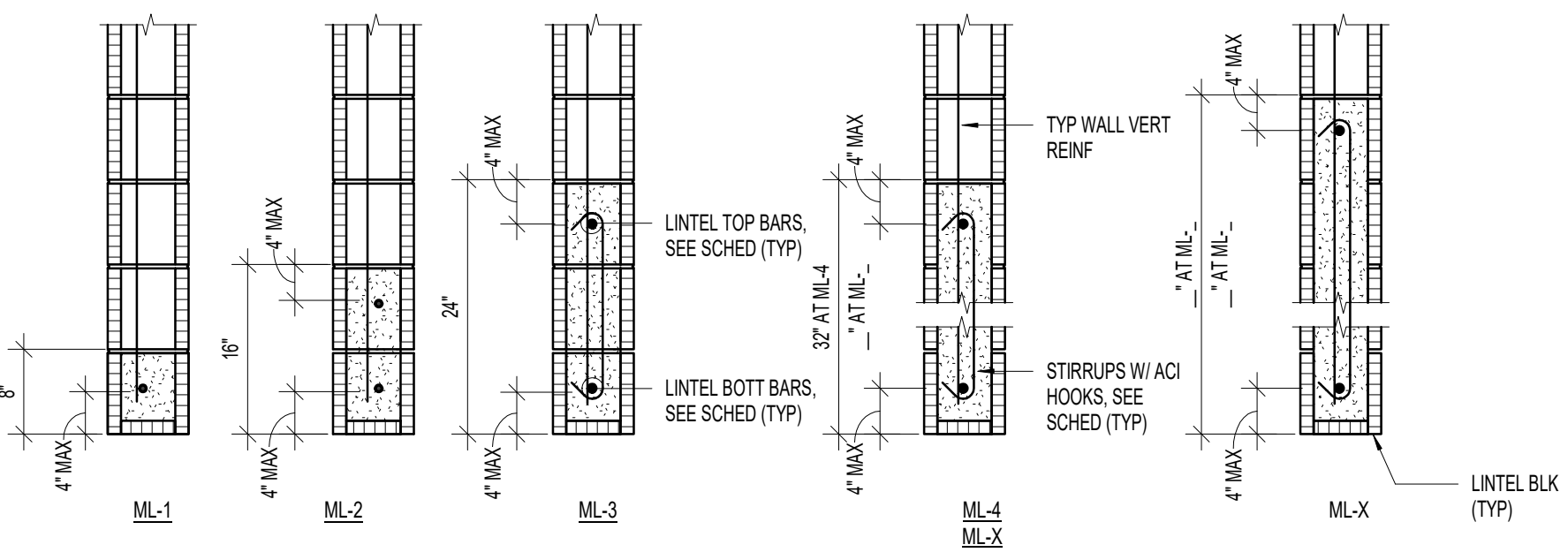
- MASONRY WALL NOTES**
- COORDINATE WITH ARCHITECTURAL DRAWINGS, MASONRY WALL FINISHES, TYPES OF MATERIAL, COURSING, ETC.
 - DO NOT SOLID GROUT WALLS UNLESS NOTED OTHERWISE.
 - ALL MASONRY BELOW GRADE SHALL BE GROUTED SOLID.
 - VERTICAL REINFORCING SHALL BE CENTERED IN THE WALL UNLESS NOTED OTHERWISE.
 - PROVIDE (1) VERTICAL BAR MINIMUM AT ALL CORNERS AND END OF WALLS.
 - HORIZONTAL WALL REINFORCING SHALL BE PLACED BETWEEN VERTICAL MASONRY COLUMN REINFORCING BARS.
 - HORIZONTAL WALL REINFORCING SHALL CONTINUE THRU MASONRY LINTELS, WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING OCCUR IN THE SAME COURSE, USE THE LARGER REINFORCING.
 - HORIZONTAL WALL REINFORCEMENT SPACING SHALL NOT EXCEED 48" IN SPECIAL REINFORCED MASONRY SHEAR WALLS. MAX SPACING OF HORIZONTAL AND VERTICAL BARS SHALL NOT EXCEED THE LESSER OF 48" OR WALL LENGTH / 3, OR WALL HEIGHT / 3.
 - MASONRY WALLS NOT DESIGNATED ON THE PLANS SHALL BE REINFORCED AS FOLLOWS:
 - IF JOINT REINFORCING IS REQUIRED, PROVIDE 3/8" DIAMETER GALVANIZED LADDER TO TRUSS TYPE REINFORCING.

THICKNESS	VERTICAL REINFORCING	HORIZONTAL REINFORCING
8"	#5 BARS AT 32"oc	#4 BARS AT 48"oc
8"	#5 BARS AT 32"oc	#5 BARS AT 48"oc
10"	#5 BARS AT 32"oc	#6 BARS AT 48"oc
12"	#6 BARS AT 32"oc	(2) #5 BARS AT 48"oc



MASONRY LINTEL SCHEDULE					
MARK	LINTEL DEPTH	LINTEL SPAN (MAX)	REINFORCING		COMMENTS
			HORIZONTAL	STIRRUPS	
ML-1	16"	8'-0"	(1) #7BAR CONT T&B	NONE	
ML-2	24"	8'-0"	(1) #7BAR CONT T&B	#4 AT 8"oc	
ML-3	32"	10'-0"	(1) #7BAR CONT T&B	#4 AT 8"oc	
ML-4	40"	16'-0"	(1) #7BAR CONT T&B	#4 AT 8"oc	

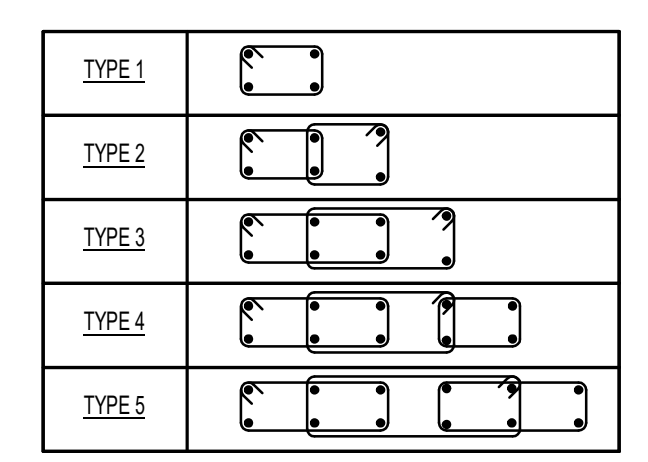
- MASONRY LINTEL NOTES**
- LINTEL WIDTH AND MATERIAL TYPES SHALL BE THE SAME AS THE WALL IN WHICH THE LINTEL IS CONSTRUCTED.
 - GROUT MASONRY LINTELS MONOLITHICALLY WITH THE SUPPORT WALL OR COLUMN AT EA END.
 - MASONRY LINTELS ML-1 THRU ML-4 SHALL BE USED OVER OPENINGS IN MASONRY WALLS WHEN A SPECIFIC MASONRY LINTEL IS NOT OTHERWISE SPECIFIED. WHEN A LINTEL IS SPECIFIED ON THE PLANS, THE MAXIMUM SPAN AS NOTED IN THIS SCHEDULE SHALL NOT APPLY. CONSULT THE STRUCTURAL ENGINEER FOR LINTELS NOT SPECIFIED ON THE PLANS WHICH HAVE A SPAN GREATER THAN 10'-0".
 - MASONRY LINTELS ML-1 THRU ML-4 SHALL NOT BE LOCATED DIRECTLY BELOW FLOOR OR ROOF BEAMS OR CRIBS UNLESS NOTED OTHERWISE ON THE PLANS. JOISTS SHALL NOT BEAR ON ANY LINTEL LESS THAN 16" DEEP. CONSULT THE STRUCTURAL ENGINEER FOR LINTELS NOT SHOWN ON THE PLANS WHICH ARE LOCATED DIRECTLY BELOW FLOOR OR ROOF BEAMS OR CRIBS.
 - EXTEND ALL HORIZONTAL REINFORCING BEYOND THE EDGE OF ALL OPENINGS. IF HORIZONTAL REINFORCING CANNOT EXTEND LAP SPICE LENGTH BEYOND EDGE OF OPENING, PROVIDE 90° STANDARD HOOK.
 - SPLICE TOP BARS AT MID-SPAN OF LINTEL ONLY AND BOTTOM BARS OVER SUPPORTS ONLY.
 - HORIZONTAL WALL REINFORCING SHALL CONTINUE THRU MASONRY LINTELS, WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING OCCUR IN THE SAME COURSE, USE THE LARGER REINFORCING.
 - DOWEL VERTICAL REINFORCING OF WALL ABOVE LINTEL INTO THE FULL DEPTH OF LINTEL.
 - SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.



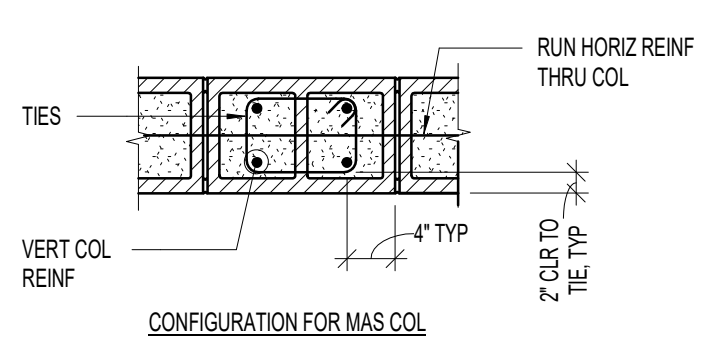
1 MASONRY WALL SCHEDULE
SE803.1 NO SCALE
TAB-01

2 MASONRY LINTEL SCHEDULE
SE803.1 NO SCALE
TAB-02

MASONRY COLUMN SCHEDULE				
MARK	COLUMN SIZE	REINFORCING		
		VERTICAL	TIES	TYPE
MC-1	8" x 16"	(4) #5	#3 AT 8"oc	1
MC-2	8" x 24"	(6) #5	#3 AT 8"oc	3
MC-3	8" x 48"	(12) #5	#3 AT 8"oc	5



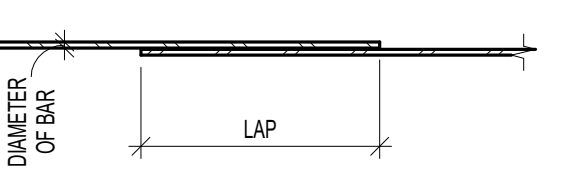
- MASONRY COLUMN NOTES**
- HORIZONTAL WALL REINFORCEMENT SHALL BE LOCATED TO THE INSIDE OF VERTICAL BARS. THE CENTERLINE OF VERTICAL BARS SHALL BE LOCATED 2 1/2" FROM FACE OF THE MASONRY.
 - VERTICAL REINFORCING AND TIES SHALL EXTEND TO FULL WALL HEIGHT, UNO.
 - VERTICAL MASONRY COLUMN REINFORCING SHALL EXTEND INTO FOOTING AND TERMINATE WITH A STANDARD 90° HOOK. FOR CONCRETE FOUNDATION WALLS OVER 8'-0" TALL, VERTICAL COLUMN REINFORCING SHALL DOWEL 4" MINIMUM INTO THE FOUNDATION WALL.
 - IN CONCRETE FOUNDATION WALLS, VERTICAL MASONRY COLUMN REINFORCING SHALL BE TIED WITH #3 TIES AT THE SAME SPACING AND CONFIGURATION AS MASONRY COLUMNS ABOVE.
 - #3 TIES MAY BE SUBSTITUTED WITH #2 TIES IN SEISMIC DESIGN CATEGORIES A, B AND C.



3 MASONRY COLUMN SCHEDULE
SE803.1 NO SCALE
TAB-04

MASONRY REINFORCING BAR LAP SPICE SCHEDULE										
REBAR SIZE	SINGLE BAR CENTERED IN CELL									
	THICKNESS		THICKNESS		THICKNESS		THICKNESS		THICKNESS	
	8"	10"	12"	8"	10"	12"	8"	10"	12"	8"
#3	12"	12"	12"	12"	12"	12"	12"	12"	12"	12"
#4	13"	12"	12"	12"	12"	12"	12"	12"	12"	12"
#5	20"	16"	13"	19"	14"	12"	16"	13"	12"	14"
#6	38"	29"	24"	34"	26"	21"	31"	24"	20"	29"
#7	52"	40"	33"	47"	36"	29"	42"	33"	27"	39"
#8	72"	61"	50"	71"	55"	45"	65"	50"	41"	60"
#9	81"	78"	64"	81"	70"	57"	81"	64"	52"	78"

- MASONRY REINFORCING BAR LAP SPICE NOTES**
- MECHANICAL SPLICES ARE REQUIRED FOR BARS IN MASONRY GREATER THAN #8 BARS.
 - MECHANICAL SPLICES MAY BE USED IN LIEU OF LAP SPLICES SHOWN.
 - MECHANICAL SPLICES SHALL DEVELOP 125% OF SPECIFIED YIELD STRENGTH OF BAR.
 - MASONRY DEVELOPMENT LENGTHS SHOWN SHALL BE INCREASED BY 50% WHERE REBAR IS COATED WITH EPOXY. * BARS MAY BE DIFFICULT TO LAP SPICE DUE TO CONGESTION. COUPLERS RECOMMENDED.



4 MASONRY REINFORCING BAR LAP SPICE SCHEDULE
SE803.1 NO SCALE
TAB-03

STEEL ANGLE LINTEL SCHEDULE (NON-STRUCTURAL)	
CLEAR OPENING	(VERT x HORIZ x THICKNESS)
UP TO 5'-0"	3 1/2" x 3" x 1/4"
5'-1" TO 7'-0"	3 1/2" x 3 1/2" x 1/4"
7'-1" TO 9'-0"	5" x 3 1/2" x 1/4"
9'-1" TO 10'-0"	5" x 3 1/2" x 5/16"
10'-1" TO 11'-0"	5" x 3 1/2" x 3/8"
11'-1" TO 12'-0"	6" x 4" x 3/8"
12'-1" AND OVER	REQUIRES SPECIAL ANALYSIS

- NOTE**
- LINTELS CARRY VENEER ONLY, WHERE FLOORS, ROOFS OF CONCENTRATED LOADS OCCUR. FURTHER ANALYSIS IS NECESSARY.
 - PROVIDE 1" OF BEARING EA END FOR EA FOOT OF SPAN. MINIMUM BEARING OF 6" EA SIDE OF OPENING.
 - USE THIS SCHEDULE UNLESS NOTED OTHERWISE.
 - LINTELS ARE TO BE GALVANIZED.

5 STEEL ANGLE LINTEL SCHEDULE (NON-STRUCTURAL)
SE803.1 NO SCALE
TAB-05

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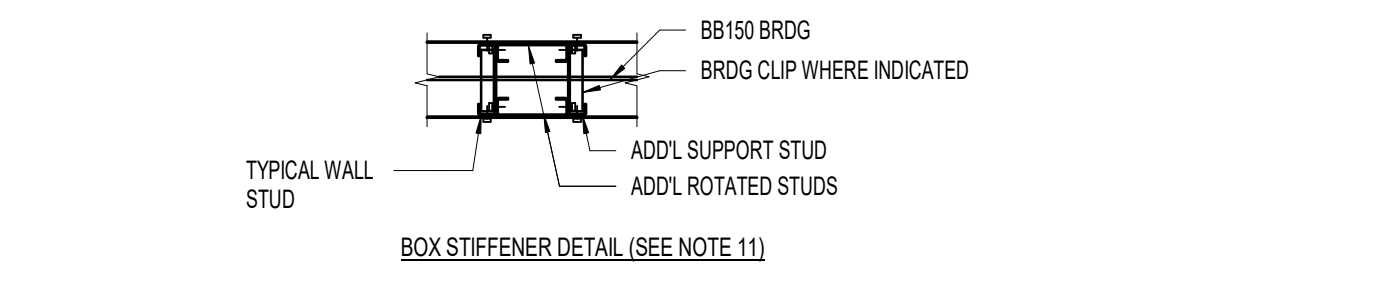
MASONRY SCHEDULES
SE803.1

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NO.	DATE	DESCRIPTION

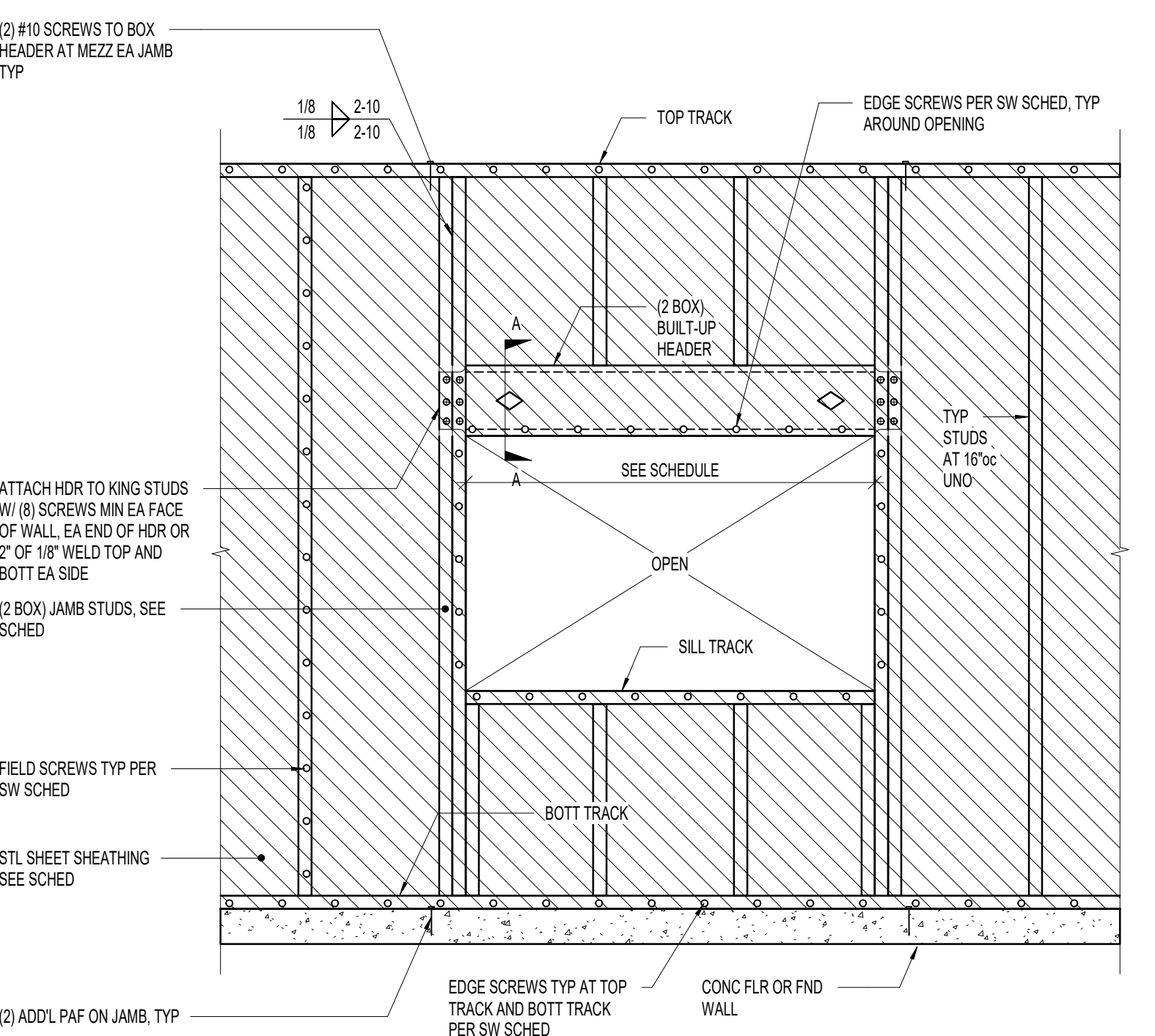
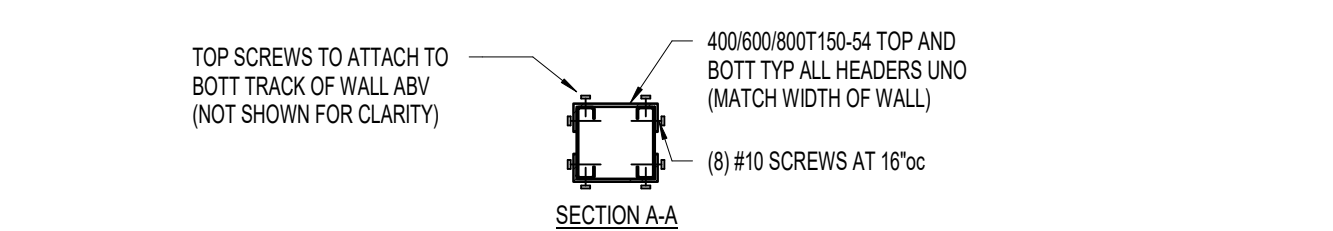
MARK	SIZE AND SPAN	TOP & BOTT TRACK	LOAD DISTRIBUTION MEMBER	LOCATION (UNO)	BRIDGING CLIP AT EA STUD
SS-6	8'0"X20'-54" AT 16"OC	6007200-54	(2) 6005200-54	ALL WALLS UNO	BCBCB8150

- NOTES:**
- ALL NON-BEARING WALLS 400600S125-33 (33) AT 16"OC TYPICAL UNO.
 - ALL TRACKS SHALL BE 400600T150-54 (50) TYPICAL UNO.
 - SEE SHEAR WALL SCHEDULE FOR SHEAR WALL SHEATHING, STUD THICKNESS, AND ATTACHMENTS.
 - ATTACH ALL TRACKS TO EACH STUD AT EACH STUD FLANGE WITH #10 SCREWS.
 - ATTACH ALL LOAD DISTRIBUTION MEMBERS TO TOP TRACK WITH #10 SCREWS AT 16"OC.
 - ATTACH ALL LOAD DISTRIBUTION MEMBERS TO BOTTOM TRACK WITH (2) #10 SCREWS AT 12"OC TYPICAL AT DECK PERPENDICULAR TO WALL AND (2) #10 SCREWS AT 16"OC TYPICAL AT DECK PARALLEL TO WALL.
 - PROVIDE DECK CLOSURE ELEMENTS AS REQUIRED TO ALLOW FULL DEPTH CONCRETE OVER TOP OF WALL, TYPICAL.
 - ATTACH BOTTOM TRACK TO CONCRETE SLAB ON DECK, SUSPENDED SLAB ON GRADE, AND/OR FOUNDATION WALLS WITH (1) 1/2"X3/4" PAF AT 24"OC AT INTERIOR WALLS AND 12"OC AT EXTERIOR WALLS.
 - PROVIDE BR150 BRIDGING AT A MAXIMUM SPACING OF 48"OC TYPICAL.
 - IN ADDITION TO BRIDGING, PROVIDE IN-PLANE BRACING IN ALL LOAD BEARING WALLS. PROVIDE BOX SHAPE AT 10'-0" ON CENTER MAX. (6-9) WALLS SHORTER THAN 10'-0" REQUIRE (1) STIFFENER, WALLS 10'-0" TO 20'-0" LONG REQUIRE (2) STIFFENERS, AND SO ON. (1) USE (2) STUDS (SAME SIZE AS WALL STUDS) 90 DEGREES FROM TYPICAL STUDS, ONE EACH FACE OF WALL. STIFFENERS AND SO ON. (1) USE (2) STUDS PERPENDICULAR STUDS. ATTACH WITH (8) #10 SCREWS AT 16"OC. SEE SECTION A-A BELOW.
 - EDGE DISTANCE OF ALL SCREWS SHALL BE 3/8" MINIMUM.
 - ALL OPENINGS IN UTILITY/ELEVATOR SHAFT WALLS TO BE FRAMED USING 400600US-34 (33), 400600UR-34 (33), AND 400600ES-34 (33) AS REQ'D.



SPAN LENGTH	HEADER SIZE	JAMB SIZE	JAMB CONNECTOR TOP	HEADER TYPE
0'-0" - 3'-0"	(2) 600S300-54 BOXED	(2) 600S200-43 BACK TO BACK	(2) SIMPSON SCB45.5	SSHDR-1
3'-0" - 6'-0"	(2) 600S300-54 BOXED	(2) 600S200-43 BOXED	(2) SIMPSON SCB45.5	SSHDR-2
6'-0" - 9'-0"	(2) 600S300-54 BOXED	(4) 600S200-43 BACK TO BACK	(2) SIMPSON SCB45.5	SSHDR-3
9'-0" - 10'-0"	(2) 1000S300-54 BOXED	(4) 600S200-43 BACK TO BACK	DIRECT BRG ON (2) BACK TO BACK STUDS	SSHDR-4

- NOTE:**
- ALL HEADERS SHALL BE BUILT WITH (2) STUDS SIZE AS SHOWN IN THE SCHEDULE, AND (2) 400600/800T150-54 TRACKS W/ A DEPTH TO MATCH WALL THICKNESS.
 - PLACE ONE STUD AT EACH FACE OF THE WALL, PLACE ONE TRACK ON TOP AND ONE TRACK ON BOTTOM. SCREW TOGETHER WITH (8) #10 SCREWS AT 16"OC.
 - THE WEBS OF EACH OF THE STUD MEMBERS SHALL EXTEND PAST THE EDGE OF THE OPENING TO THE FAR SIDE OF THE JAMB, EACH SIDE. CORNER FLANGES AND TRACKS AS REQUIRED.
 - ALL JAMBS SHALL BE BUILT WITH (2) 400600 STUDS AS SHOWN IN THE SCHEDULE. WHEN A BOX IS SPECIFIED MAKE THE FLANGES BUTT TOGETHER.
 - WELD JAMBS TOGETHER WITH A 1/8" BY 2' LONG WELD AT 12"OC EACH SIDE.
 - JAMBS SHALL EXTEND FULL HEIGHT OF WALL.
 - ATTACH EACH WEB OF EACH HEADER STUD TO EACH FACE OF THE JAMB WITH (8) #10 SCREWS AT EACH END OF THE HEADER.
 - EDGE DISTANCE OF ALL SCREWS SHALL BE 3/8" MINIMUM.
 - BACK TO BACK JAMBS TO BE SCREWED ALONG THEIR WEB WITH #8 SCREWS AT 12"OC.



MARK	SHEATHING THICKNESS	SINGLE OR DOUBLE SHEATHED	EDGE SCREWS	FIELD SCREWS	BOTT TRACK TO SLAB
SW-1	0.027"	DOUBLE	#8 AT 6"OC	#8 AT 12"OC	5/8"Ø THRU BOLT AT 16"OC
SW-2	0.027"	DOUBLE	#8 AT 2"OC	#8 AT 12"OC	5/8"Ø THRU BOLT AT 16"OC

- NOTES:**
- ALL WALL SHEATHING SHALL BE FLAT STEEL PANEL SHEETS CONFORMING TO ASTM A1003 STRUCTURAL GRADE 33 TYPE H WITH THICKNESS AS SHOWN IN THE SCHEDULE. STANDARD WIDTH OF PANELS SHALL BE 4'-0". MINIMUM WIDTH OF ALL PANELS SHALL BE 12".
 - ALL STUDS SHALL BE AT LEAST 1-5/8" WIDE, 3-1/2" DEEP AND 43 MILS THICK. ALL TRACKS SHALL BE A MINIMUM OF 1-1/2" WIDE, 3-1/2" DEEP AND 43 MILS THICK. ALL BLOCKING AND STRAPS SHALL BE A MINIMUM OF 1-1/2" WIDE AND 43 MILS THICK.
 - USE ASTM C1513 #10-18 METAL SCREWS. ALL SCREWS SHALL HAVE A MINIMUM OF THREE THREADS EXTENSION ON THE FAR SIDE OF THE CONNECTION. INSTALL SCREWS TIGHT TO THE SURFACE OF THE SHEATHING.
 - ALL PANEL EDGES ARE DESIGNED TO BE LAPPED. AT CONTRACTORS OPTION PANELS MAY BE BLOCKED AT JOINTS IN LIEU OF LAPS.
 - PROVIDE FIELD SCREWS AT 12"OC TYPICAL FOR ALL SHEATHED WALLS.
 - PLACE STEEL PANELS IN EITHER HORIZONTAL OR VERTICAL DIRECTION. BUTT ALL JOINTS AT COMMON STUD, STRAP, AND/OR BLOCKING. DO NOT LAP SHEETS.
 - AT WALLS WITH SHEATHING ON EACH FACE OF THE WALL, STAGGER ALL PANEL JOINTS ON ONE FACE OF THE WALL FROM THE PANEL JOINTS ON THE OTHER FACE OF THE WALL, BOTH HORIZONTAL AND VERTICAL.
 - ATTACH ALL BOTTOM PLATES OF WALLS ABOVE TO TOP PLATES OF WALLS BELOW WITH THRU-BOLTS. SIZE AND SPACING AS SHOWN IN THE SCHEDULE.
 - ALL TOP PLATES AT ROOF SHALL BE ATTACHED WITH (2) ROWS OF #10 SCREWS AT EDGE SPACING SHOWN IN THE SCHEDULE.
 - ALL BOTTOM PLATES AT CONCRETE SOLE PLATES SHALL BE ATTACHED TO CONCRETE WALLS AND/OR FOOTINGS WITH CAST IN PLACE ANCHOR BOLTS. SIZE AND SPACING AS SHOWN IN THE SCHEDULE.
 - PROVIDE HOLD-DOWN AT EACH END OF EACH SHEAR WALL. SEE THE TYPICAL SHEAR WALL AND HOLD-DOWN DETAILS. PLACE DOUBLE BACK TO BACK STUDS OF HOLD-DOWNS ADJACENT TO THE JAMB STUDS AT SIDES OF OPENINGS. ENDS OF SHEAR WALLS THAT ARE BOUND BY AN HSS COLUMN DO NOT REQUIRE A HOLD-DOWN PROVIDED SHEATHING IS FASTENED TO THE HSS MEMBER WITH EDGE SCREWS AS INDICATED IN THE SCHEDULE.
 - PROVIDE BR150 BRIDGING AT A MAXIMUM SPACING OF 48"OC TYPICAL. SEE LIGHT GAUGE STEEL STUD WALL SCHEDULE FOR CLIP INFO.
 - FOR BRIDGING SEE TYPICAL BEARING SCHEDULE.
 - FOR HOLD-DOWNS SEE PLAN AND HOLD-DOWN SCHEDULE.
 - BACK TO BACK CHORDS TO BE INTERCONNECTED WITH A SPACING OF 12".
 - SOLE BOLTING AT THE TOP AND THE BOTTOM OF THE WALLS TO BE 5/8"Ø BOLTS WITH 6" EMBED AT 16"OC.

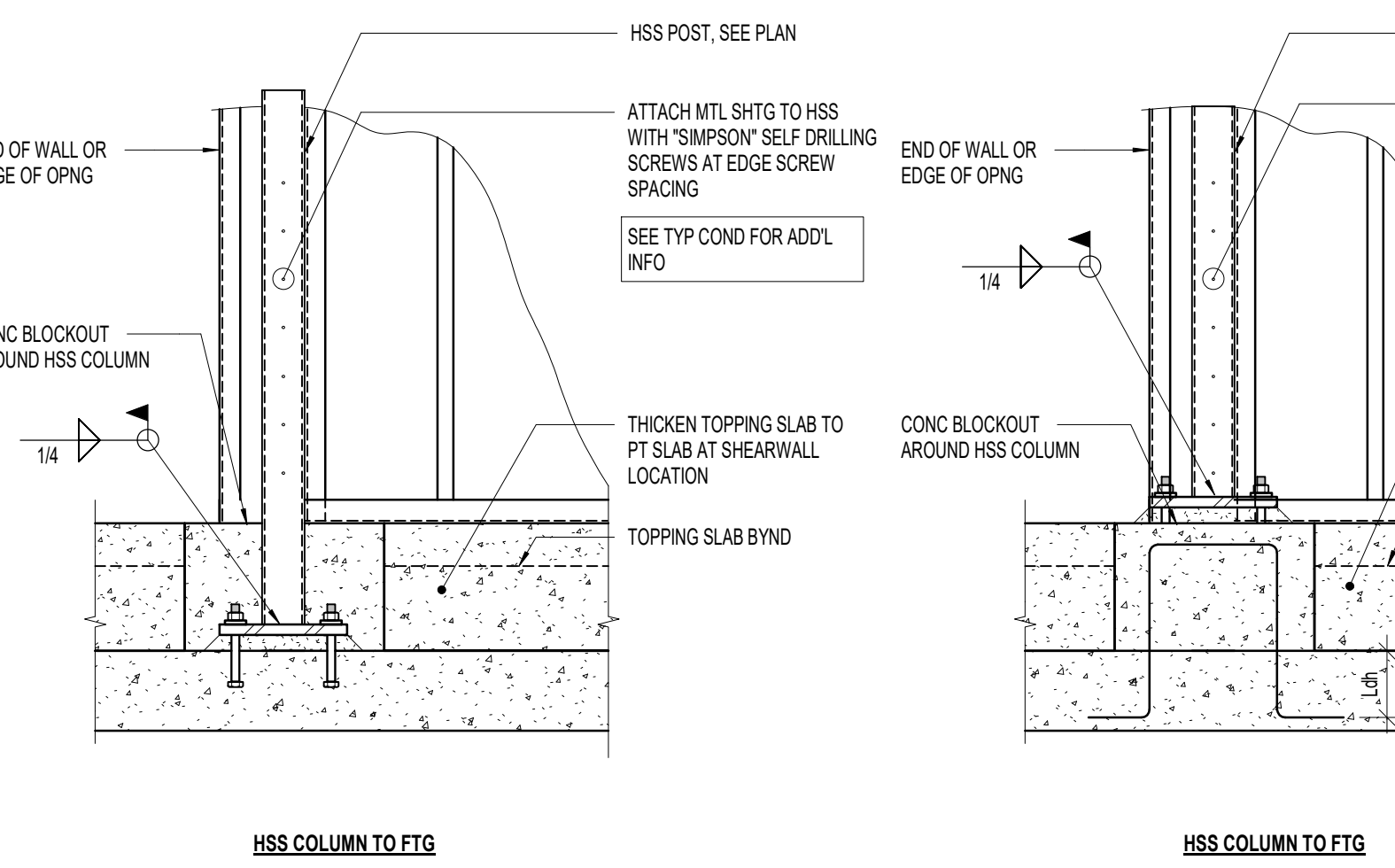
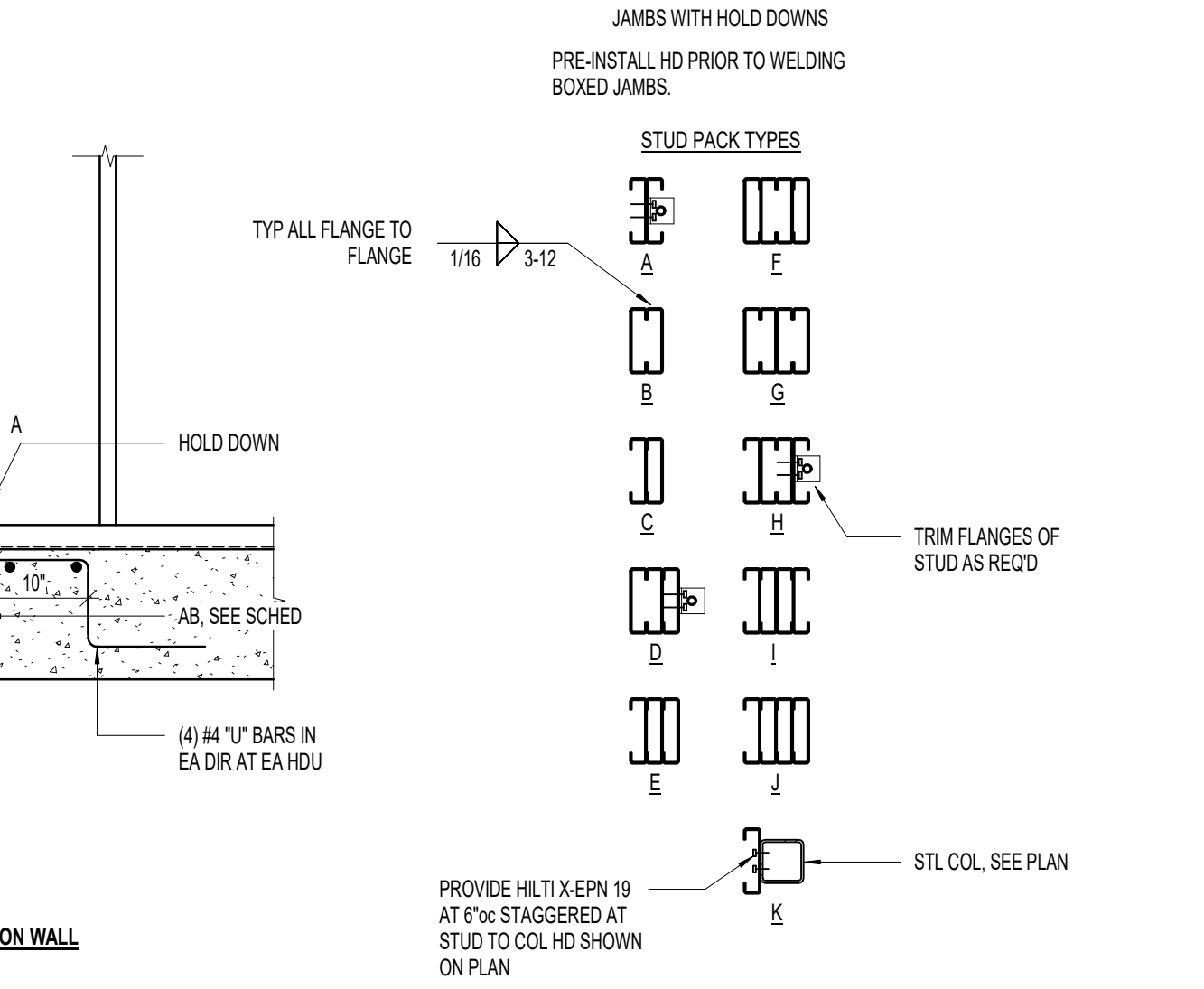
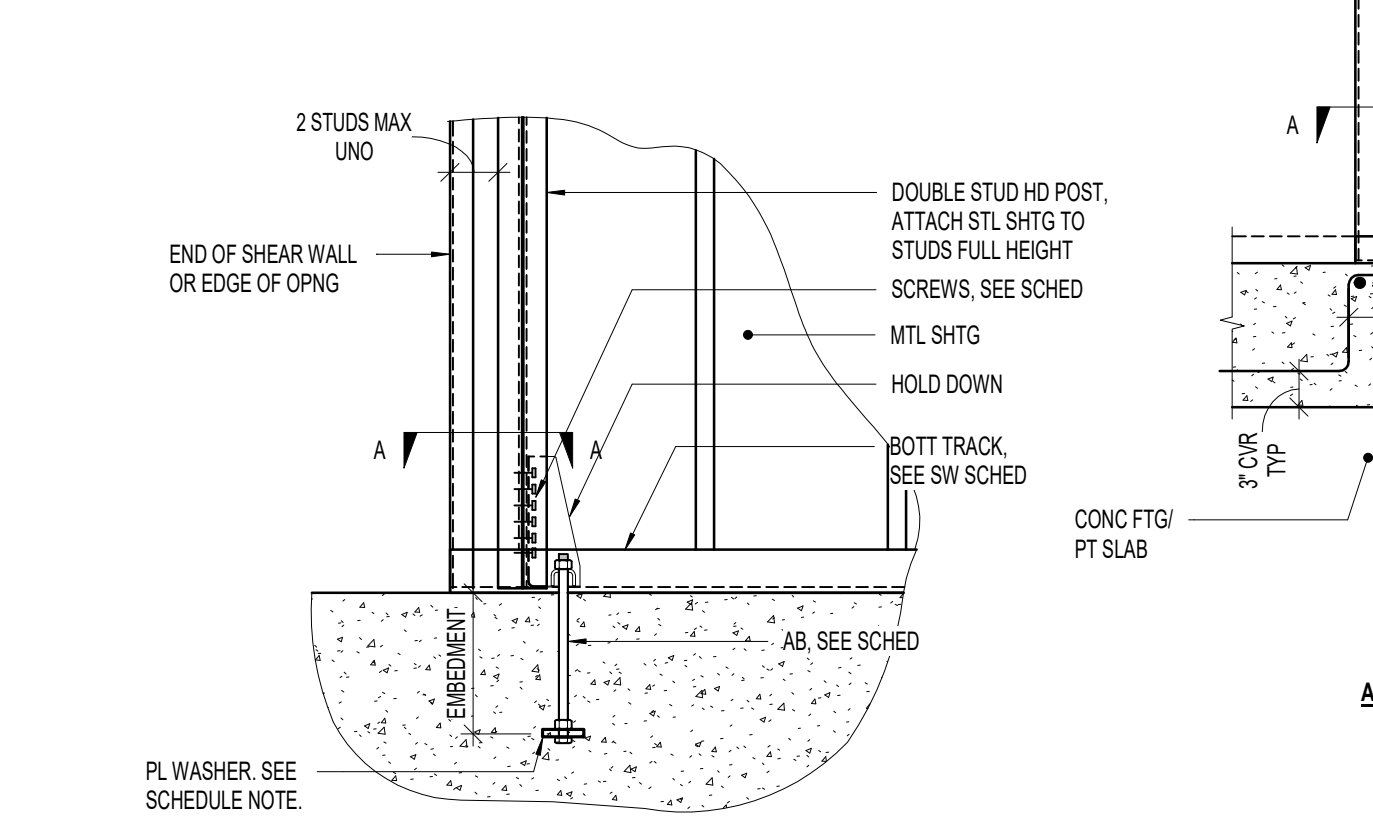
1 LIGHT GAUGE STEEL STUD WALL SCHEDULE
SECTION 1 NO SCALE
18801

2 LIGHT GAUGE HEADER/JAMB SCHEDULE
SECTION 1 NO SCALE
18802

3 LIGHT GAUGE STEEL STUD SHEARWALL SCHEDULE (STEEL SHEATHED)
SECTION 1 NO SCALE
18803

MARK	HOLD DOWN TYPE	COMP STUD TYPE	SCREWS	ANCHOR BOLT Ø	EMBED DEPTH
HD-1	SHDU6	D	(18) #10	7/8"	9"
HD-2	SHDU9	H	(26) #10	7/8"	9"
HD-3	SHDU11	A	(8) #14	1/2"	9"

- HOLD-DOWN NOTES:**
- ANCHOR BOLTS SHALL INCLUDE A DOUBLE NUT AND 4 x 4 x 1/2" PLATE WASHER.
 - INCREASE FOOTING DEPTH WHERE EMBEDMENT LENGTH PLUS 3" IS GREATER THAN FOOTING DEPTH SPECIFIED.
 - HOLD-DOWN COMPRESSION POST SIZE TO MATCH WALL THICKNESS.
 - HOLD-DOWN TYPES ARE SIMPSON STRONG-TIE BRAND.



4 LIGHT GAUGE STEEL HOLD-DOWN SCHEDULE
SECTION 1 NO SCALE
18804

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2024-08-26
BID PACKAGE #1

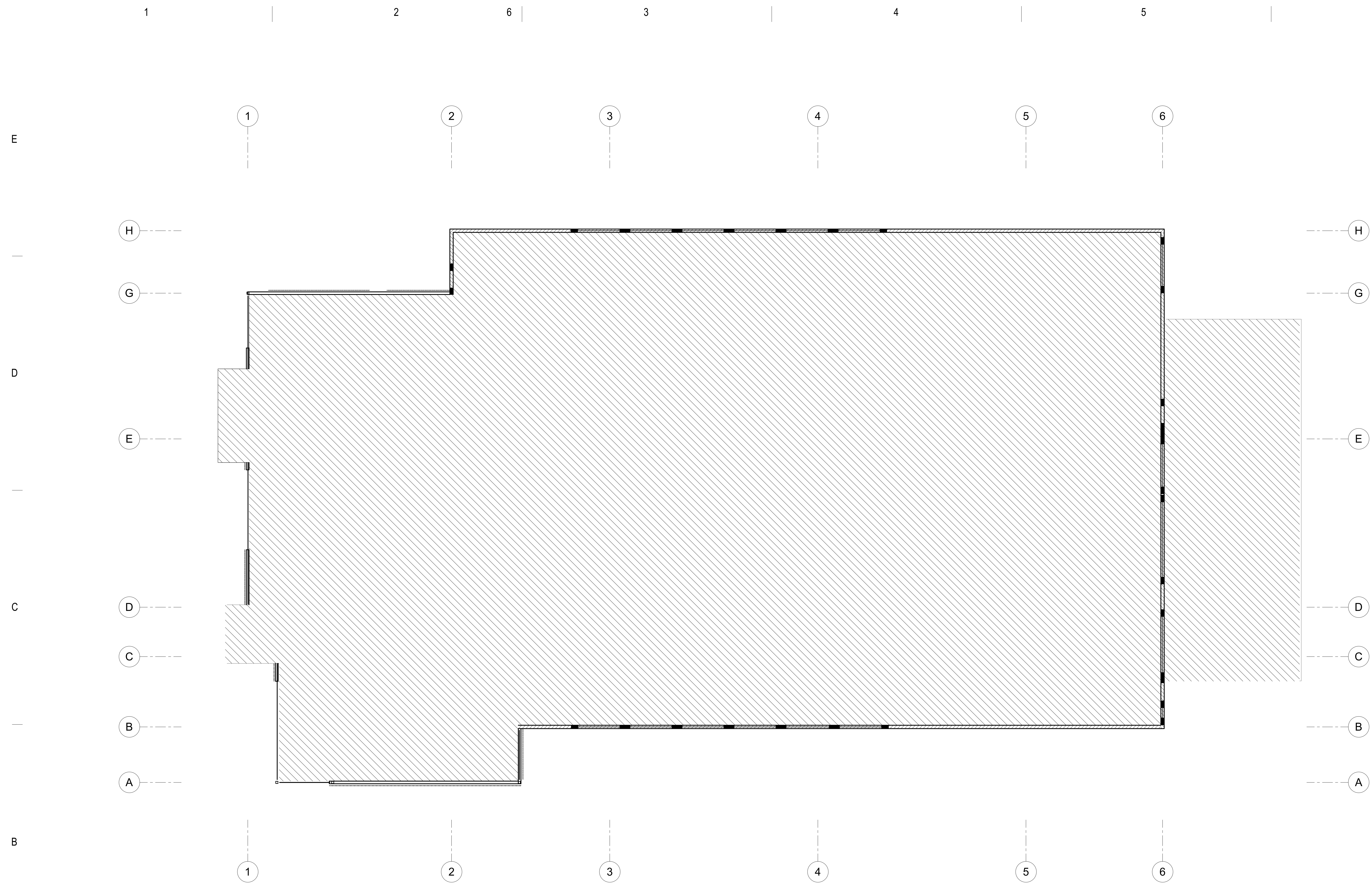
NOTE:
THESE STRUCTURAL DRAWINGS ARE BASED ON ARCHITECTURAL DRAWINGS DATED July 23, 2024.
DIMENSIONS AND ELEVATIONS, AS THEY RELATE TO THE BUILDING IN GENERAL, IN GRID TO GRID DIMENSIONS OR DECK BEARING ELEVATIONS, ARE SUPPLIED BY THE ARCHITECT. THEY ARE PROVIDED ON THE STRUCTURAL PLANS AND DETAILS FOR THE CONVENIENCE OF THE CONTRACTOR. VERIFY DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS.

STEEL STUD SCHEDULES
SE804.1
(801) 355-5915

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BUILDING**
355 SOUTH 650 EAST
KAYSVILLE, UT 84037

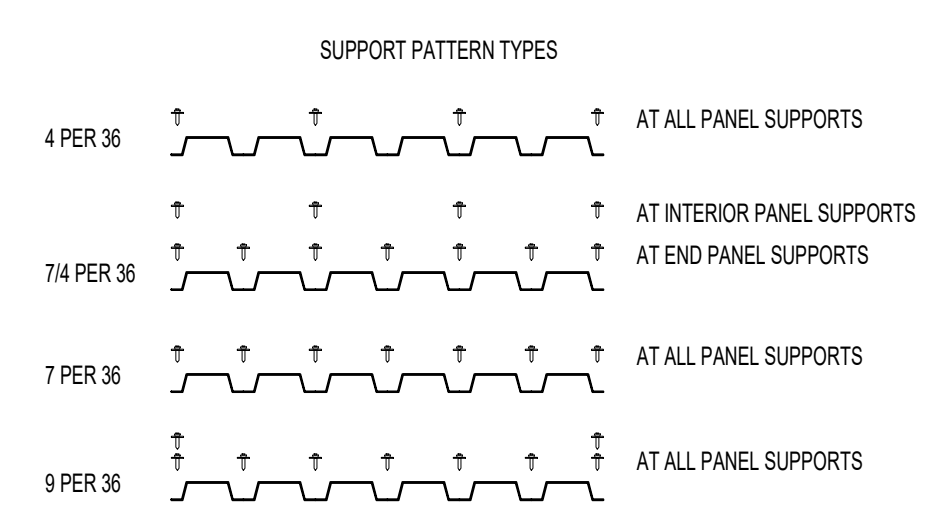


**DIAPHRAGM
SCHEDULE
SE805.1**



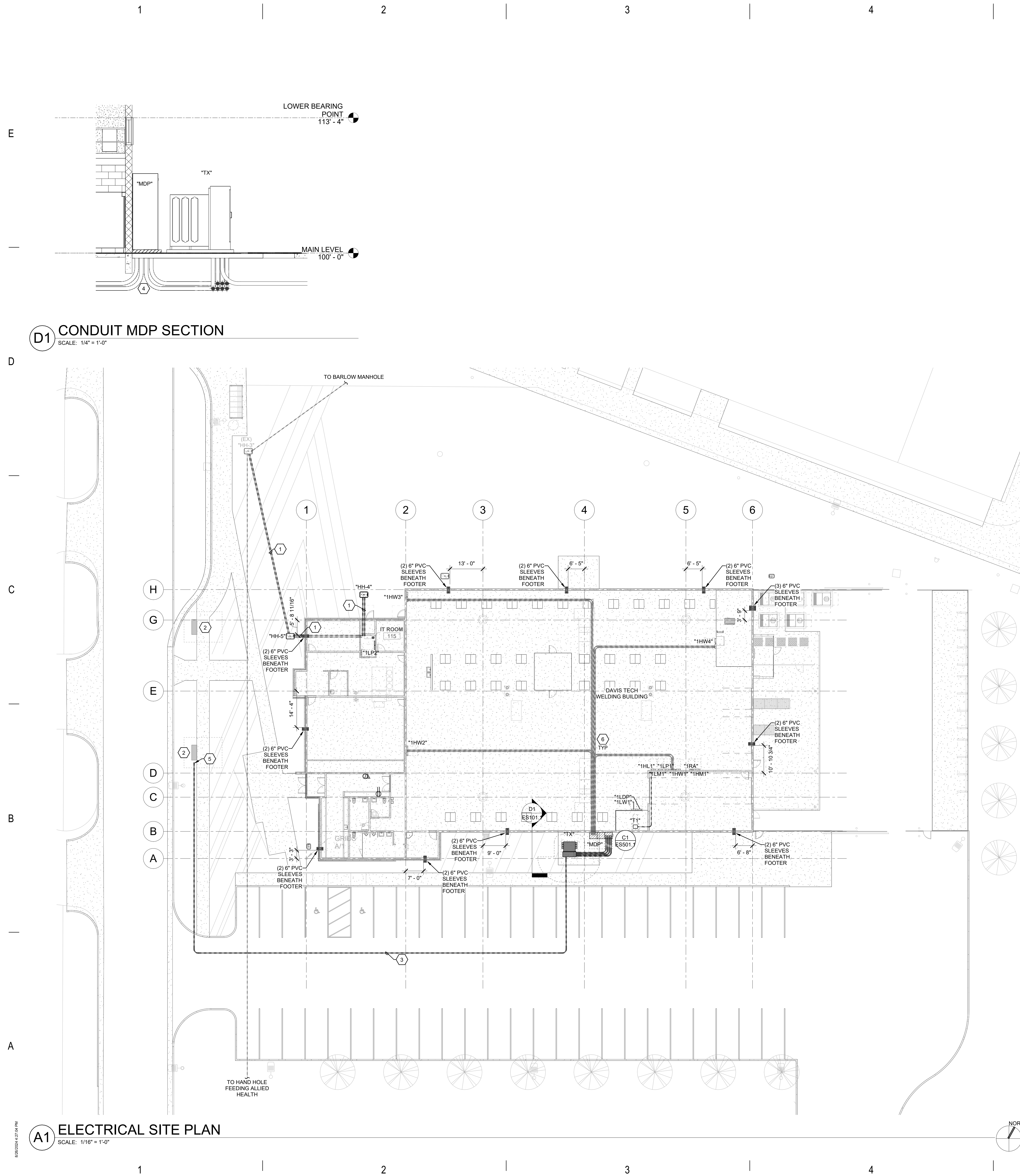
1 UPLIFT PLAN
SE805.1 NO SCALE

STEEL DECK DIAPHRAGM GAUGE CONNECTION SCHEDULE					
HATCH PATTERN	ALLOWABLE SHEAR	DECK GAGE	SUPPORT CONNECTION MULTIPANEL/TEK FASTENERS	SIDE SEAM ATTACHMENT VSC-ZDELTA/RIP SPACING	COMMENTS
	1438 psf	20 gage	7 PER 36"	6"oc	



2024-08-26
BID PACKAGE #1

NOTE:
THESE STRUCTURAL DRAWINGS ARE BASED ON ARCHITECTURAL DRAWINGS DATED July 23, 2024. DIMENSIONS AND ELEVATIONS AS THEY RELATE TO THE BUILDING IN GENERAL, IN GRID TO GRID DIMENSIONS OR DECK BEARING ELEVATIONS, ARE SUPPLIED BY THE ARCHITECT. THEY ARE PROVIDED ON THE STRUCTURAL PLANS AND DETAILS FOR THE CONVENIENCE OF THE CONTRACTOR. VERIFY DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS.



GENERAL SHEET NOTES

- 1 THE ELECTRICAL CONTRACTOR SHALL MEET WITH AND COORDINATE WITH ALL SERVICE PROVIDERS (POWER, COMMUNICATION, CABLE/SATELLITE, ETC.) TO THE FACILITY ON SITE PRIOR TO ANY WORK BEING PERFORMED. CONFIRM WITH EACH SERVICE PROVIDER EXACT LOCATIONS OF EQUIPMENT AND ROUTING. COMPLY WITH ALL SERVICE PROVIDER'S CURRENT STANDARDS AND REQUIREMENTS. PROVIDE THE REQUIRED EQUIPMENT, RACEWAYS, BOXES, CABLE, ETC. AS REQUIRED BY THE SERVICE PROVIDER WHETHER SHOWN ON THE DRAWINGS OR NOT.
- 2 CONTRACTOR IS RESPONSIBLE FOR ALL TRENCHING, BACKFILL, AND COMPACTION ASSOCIATED TO ALL ELECTRICAL UNDERGROUND RACEWAYS AND CABLES. COORDINATE WITH ARCHITECTURAL AND CIVIL DRAWINGS. SEE UNDERGROUND RACEWAY DETAILS FOR REQUIREMENTS FOR EACH TRENCH.
- 3 THE ELECTRICAL CONTRACTOR SHALL HAVE ANY AND ALL CONCRETE POLE BASES AND SLABS REVIEWED BY A STRUCTURAL ENGINEER AND SHALL MODIFY DESIGN PER STRUCTURAL ENGINEER'S AND OR AHJ'S RECOMMENDATIONS.
- 4 THE ELECTRICAL CONTRACTOR IS RESPONSIBLE FOR ALL CONCRETE/ASPHALT CUTTING AND REPLACEMENT OF CONCRETE/ASPHALT TO MATCH EXISTING ASSOCIATED WITH UNDERGROUND RACEWAYS PROVIDED AS PART OF THIS PROJECT.
- 5 REFER TO PLANS FOR CONSTRAINTS ON PHYSICAL DIMENSIONS AND CLEARANCE REQUIREMENTS OF EQUIPMENT. PROVIDE EQUIPMENT DIMENSIONS THAT FALL WITHIN THE CONSTRAINTS OF EACH SPECIFIC LOCATION.
- 6 SERVICE EQUIPMENT SHALL BE LEGIBLY MARKED IN THE FIELD WITH THE MAXIMUM AVAILABLE FAULT CURRENT. VERIFY OR RE-CALCULATE THE AVAILABLE FAULT CURRENT AT THE SERVICE WHERE MODIFICATIONS TO THE ELECTRICAL INSTALLATION OCCUR. PLEASE INCLUDE NOTES IN THE ELECTRICAL DRAWINGS OR SUPPLY CALCULATIONS WHERE APPLICABLE. SEE NEC 110.24 (B).
- 7 ALL UNDERGROUND RACEWAYS SHALL UTILIZE GRADUAL SWEEPS WHERE POSSIBLE. PROVIDE FACTORY LONG SWEEP BENDS ONLY WHERE ABSOLUTELY NECESSARY OR NOTED.
- 8 CONTRACTOR SHALL ADJUST DEPTH OF RACEWAYS DEEPER AS NECESSARY TO AVOID CONFLICTS WITH OTHER UTILITIES AND MAINTAIN A SEPARATION OF NO LESS THAN 1 FOOT FROM ALL OTHER UTILITIES CROSSINGS UNLESS NOTED OTHERWISE.
- 9 ALL ELECTRICAL GEAR AND EQUIPMENT (GENERATOR, TRANSFORMER, SWITCHBOARDS, PANELBOARDS, DISCONNECTS, ENCLOSURES, ETC.) LOCATED OUTDOOR OR EXPOSED TO WEATHER SHALL BE NEMA 3R RATED UNLESS NOTED OTHERWISE.

SHEET KEYNOTES

- 1 PROVIDE 2 EA. 4" CONDUITS WITH (1) 7 WAY 12.7X10 MM MICRODUCT, (2) WITH 3 EA. 1.25" INNERDUCT IN EACH CONDUIT.
- 2 EXISTING UTILITY MEDIUM VOLTAGE EQUIPMENT TO REMAIN AND BE PRESERVED.
- 3 APPROXIMATE ROUTING OF PRIMARY CONDUIT TO TRANSFORMER.
- 4 DROPPED FOOTING FOR FUTURE ACCESS OF ALL CONDUITS FROM SWITCHBOARD TO EQUIPMENT. COORDINATE WITH STRUCTURAL.
- 5 CONTRACTOR TO PROVIDE CONDUIT 1 FOOT AWAY FROM EXISTING SECTIONALIZER. CONTRACTOR TO COORDINATE WITH KAYSVILLE CITY POWER ONCE COMPLETED. KAYSVILLE CITY POWER TO INSTALL ELBOW INTO SECTIONALIZER.
- 6 HALFTONED RACEWAYS AND EQUIPMENT ARE SHOWN FOR REFERENCE ONLY AND NOT INCLUDED IN BID PACKAGE 1.



PROJECT **24-038**

BID PACKAGE #1 2024-08-26

REVISIONS
NO. DATE DESCRIPTION



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Salt Lake City, UT 84111
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SE Project # 240296

DTC WELDING TECH & FABRICATION BUILDING
355 SOUTH 650 EAST
KAYSVILLE, UT 84037



ELECTRICAL SITE PLAN

ES101.1

(801) 355-5915

GENERAL SHEET NOTES

- THE ELECTRICAL CONTRACTOR SHALL MEET WITH AND COORDINATE WITH ALL SERVICE PROVIDERS (POWER, COMMUNICATION, CABLE/SATELLITE, ETC.) TO THE FACILITY ON SITE PRIOR TO ANY WORK BEING PERFORMED. CONFIRM WITH EACH SERVICE PROVIDER EXACT LOCATIONS OF EQUIPMENT AND ROUTING. COMPLY WITH ALL SERVICE PROVIDER'S CURRENT STANDARDS AND REQUIREMENTS. PROVIDE THE REQUIRED EQUIPMENT, RACEWAYS, BOXES, CABLE, ETC. AS REQUIRED BY THE SERVICE PROVIDER WHETHER SHOWN ON THE DRAWINGS OR NOT.
- CONTRACTOR IS RESPONSIBLE FOR ALL TRENCHING, BACKFILL, AND COMPACTION ASSOCIATED TO ALL ELECTRICAL UNDERGROUND RACEWAYS AND CABLES. COORDINATE WITH ARCHITECTURAL AND CIVIL DRAWINGS. SEE UNDERGROUND RACEWAY DETAILS FOR REQUIREMENTS FOR EACH TRENCH.
- THE ELECTRICAL CONTRACTOR SHALL HAVE ANY AND ALL CONCRETE POLE BASES AND SLABS REVIEWED BY A STRUCTURAL ENGINEER AND SHALL MODIFY DESIGN PER STRUCTURAL ENGINEER'S AND OR AHJ'S RECOMMENDATIONS.
- THE ELECTRICAL CONTRACTOR IS RESPONSIBLE FOR ALL CONCRETE/ASPHALT CUTTING AND REPLACEMENT OF CONCRETE/ASPHALT TO MATCH EXISTING ASSOCIATED WITH UNDERGROUND RACEWAYS PROVIDED AS PART OF THIS PROJECT.
- REFER TO PLANS FOR CONSTRAINTS ON PHYSICAL DIMENSIONS AND CLEARANCE REQUIREMENTS OF EQUIPMENT. PROVIDE EQUIPMENT DIMENSIONS THAT FALL WITHIN THE CONSTRAINTS OF EACH SPECIFIC LOCATION.
- SERVICE EQUIPMENT SHALL BE LEGIBLY MARKED IN THE FIELD WITH THE MAXIMUM AVAILABLE FAULT CURRENT. VERIFY OR RE-CALCULATE THE AVAILABLE FAULT CURRENT AT THE SERVICE WHERE MODIFICATIONS TO THE ELECTRICAL INSTALLATION OCCUR. PLEASE INCLUDE NOTES IN THE ELECTRICAL DRAWINGS OR SUPPLY CALCULATIONS WHERE APPLICABLE. SEE NEC 110.24 (B).
- ALL UNDERGROUND RACEWAYS SHALL UTILIZE GRADUAL SWEEPS WHERE POSSIBLE. PROVIDE FACTORY LONG SWEEP BENDS ONLY WHERE ABSOLUTELY NECESSARY OR NOTED.
- CONTRACTOR SHALL ADJUST DEPTH OF RACEWAYS DEEPER AS NECESSARY TO AVOID CONFLICTS WITH OTHER UTILITIES AND MAINTAIN A SEPARATION OF NO LESS THAN 1 FOOT FROM ALL OTHER UTILITIES CROSSINGS UNLESS NOTED OTHERWISE.
- ALL ELECTRICAL GEAR AND EQUIPMENT (GENERATOR, TRANSFORMER, SWITCHBOARDS, PANELBOARDS, DISCONNECTS, ENCLOSURES, ETC.) LOCATED OUTDOOR OR EXPOSED TO WEATHER SHALL BE NEMA 3R RATED UNLESS NOTED OTHERWISE.

CRSA

PROJECT **24-038**

BID PACKAGE #1 2024-08-26

REVISIONS
NO. DATE DESCRIPTION

SPECTRUM ENGINEERS

324 S. State St., Suite 400
Salt Lake City, UT 84111
801-328-5151
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SE Project # 240296

SHEET KEYNOTES

DTC WELDING TECH & FABRICATION BUILDING
355 SOUTH 650 EAST
KAYSVILLE, UT 84037

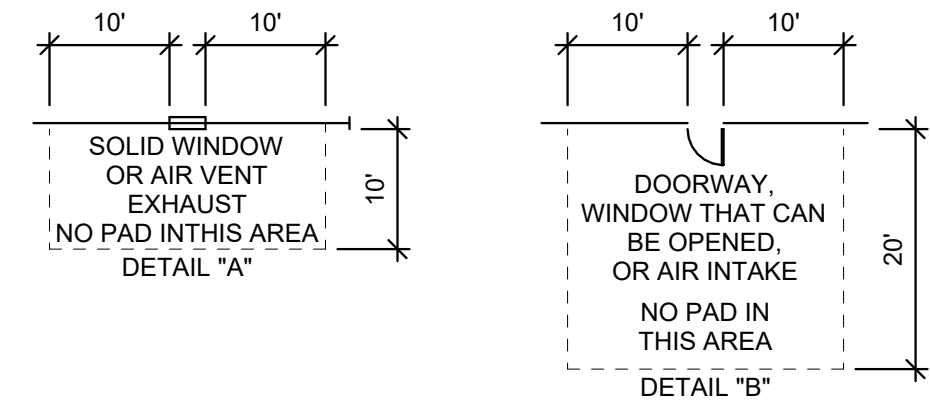


ELECTRICAL DETAILS
ES501.1

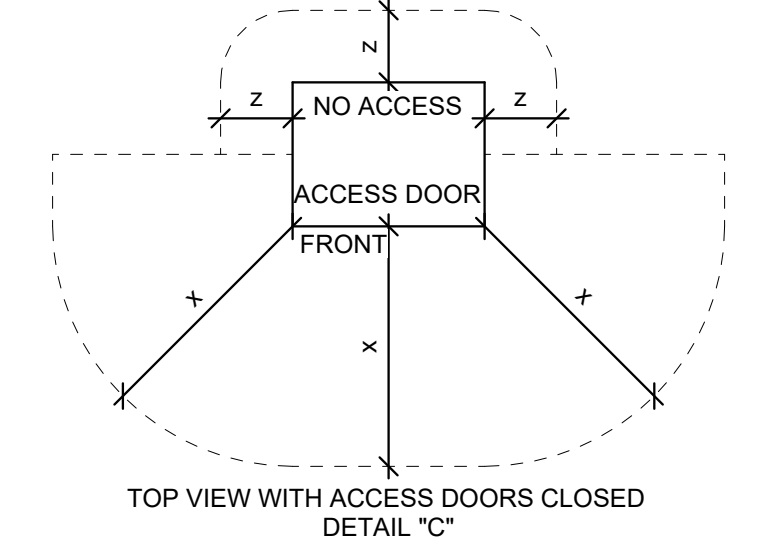
(801) 355-5915

1 2 3 4 5 6

E



- CLEARANCE GENERAL NOTES:
- CLEARANCES: THE FRONT OF THE PAD SHOULD ALWAYS FACE AWAY FROM ADJACENT STRUCTURES AND BE FREE OF OBSTRUCTIONS. AT LEAST 3 FEET MUST SEPARATE THE EDGES OF THE PAD FROM ANY ADJACENT STRUCTURE. THE EDGES OF THE PAD MUST BE AT LEAST 10 FEET FROM ANY COMBUSTIBLE STRUCTURE. IF AN ADJACENT STRUCTURE HAS ANY OVERHANG OR EAVE WITHIN 27 VERTICAL FEET OF THE TOP OF THE PAD, CLEARANCES MUST BE MEASURED FROM THE OUTSIDE OF THE OVERHANG. THE PAD MUST NOT BE PLACED IN AN AREA 10 FEET IN LINE WITH OR 10 FEET TO EITHER SIDE OF ANY WINDOW IN AN ADJACENT STRUCTURE (SEE DETAIL "A"). CLEARANCE FOR A DOOR MUST BE 20 FEET IN LINE WITH IT AND 10 FEET ON THE SIDES (SEE DETAIL "B"). PADS MUST NOT BE PLACED WITHIN 15 FEET OF ANY VALVE OR WITHIN 25 FEET OF ANY PLUMBING OR STORAGE FACILITY CONTAINING FLAMMABLE MATERIAL. NO WALLS, FENCES, OR ANY OTHER OBSTRUCTIONS WILL BE PLACED WITHIN 3 FEET OF THE SIDES OR BACK OF THE PAD, OR WITHIN 10 FEET OF THE FRONT OF THE PAD (SEE DETAIL "C"). THE AREA IN FRONT OF THE PAD MUST HAVE 10 FEET OF CLEAR, LEVEL WORKING AREA FOR MAINTENANCE OF THE TRANSFORMER. THE PAD MAY NOT BE PLACED IN LINE WITH AN AIR INTAKE WITHIN 32 VERTICAL FEET OF THE SURFACE PAD, ALSO VERTICALLY. IT MUST NOT BE PLACED WITHIN 12 FEET OF A DOOR OR WINDOW. VAULTS SHALL BE LOCATED WITHIN 15 FEET OF A GRAVELED OR PAVED SURFACE SUITABLE FOR INCIDENTAL HEAVY TRUCK ACCESS.
 - BARRIERS: IF THE TRANSFORMER PAD IS TO BE LOCATED IN AREAS SUBJECT TO VEHICULAR TRAFFIC, (PARKING LOTS, DRIVEWAYS, ETC) CONTACT KAYSVILLE POWER FOR PROTECTIVE BARRIER REQUIREMENTS.
 - IF THE TRANSFORMER WILL NOT COVER THE CABLE OPENINGS ON THESE STANDARD PADS, SEAL THE SIDES OF THE CABLE OPENING TO FIT THE TRANSFORMER USING SAKRETE OR COMPARABLE.

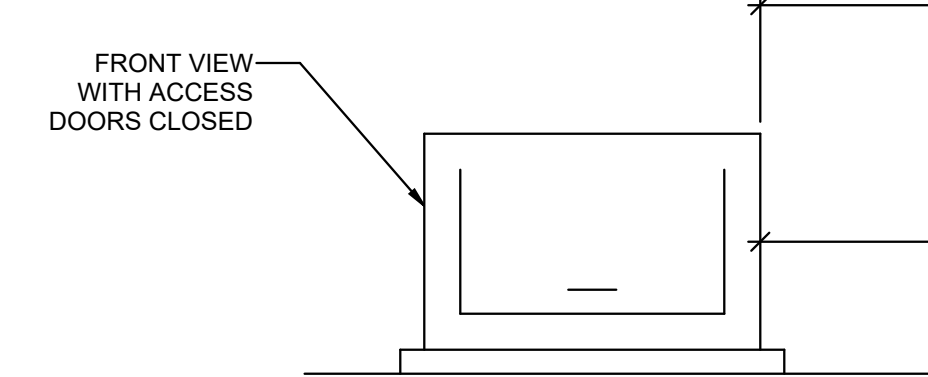


MINIMUM DISTANCE REQUIRED FROM PAD:

x = 10 FT. CLEAR AREA IN FRONT OF ANY EQUIPMENT ACCESS DOOR OR OPENING TO ALLOW THE USE OF HOT STICKS. (SEE DIMENSIONS IN DETAIL "C"). LOCATE PADMOUNTED EQUIPMENT WITH ACCESS DOORS AWAY FROM BUILDING WALLS OR OTHER BARRIERS TO ALLOW SAFE WORKING PRACTICES. IF THE EQUIPMENT ACCESS SIDE MUST FACE A WALL, ALLOW 10 FEET FOR WORKING CLEARANCE. NO VEGETATION OR TRIP HAZARDS IN THIS WORK SPACE ARE PERMITTED.)

y = 8 FT FROM ANY STRUCTURE OR ROOF OVERHANG CONSISTING OF COMBUSTIBLE MATERIAL. 3 FT TO NON-COMBUSTIBLE STRUCTURES HAVING NO OPENINGS CLOSER THAN 10 FT.

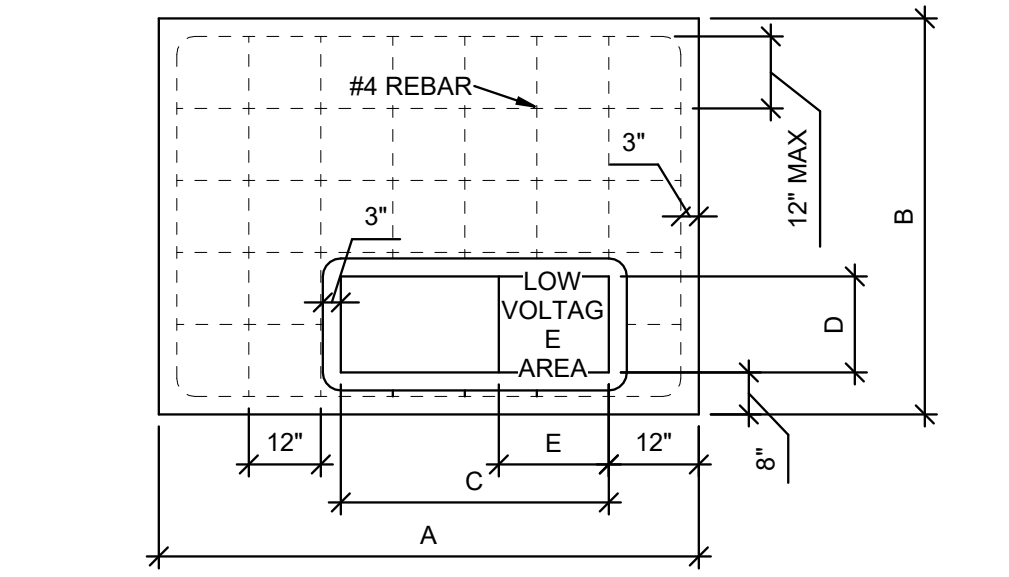
z = 3 FT CLEAR AREA ON NON-ACCESS SIDES OF THE EQUIPMENT TO ALLOW WORK SPACE. (SEE DIMENSIONS IN DETAILS "C"). 6" FROM ANY METALLIC OBJECT INCLUDING THE METERING EQUIPMENT, AND 8" FROM ANY COMBUSTIBLE SURFACE.



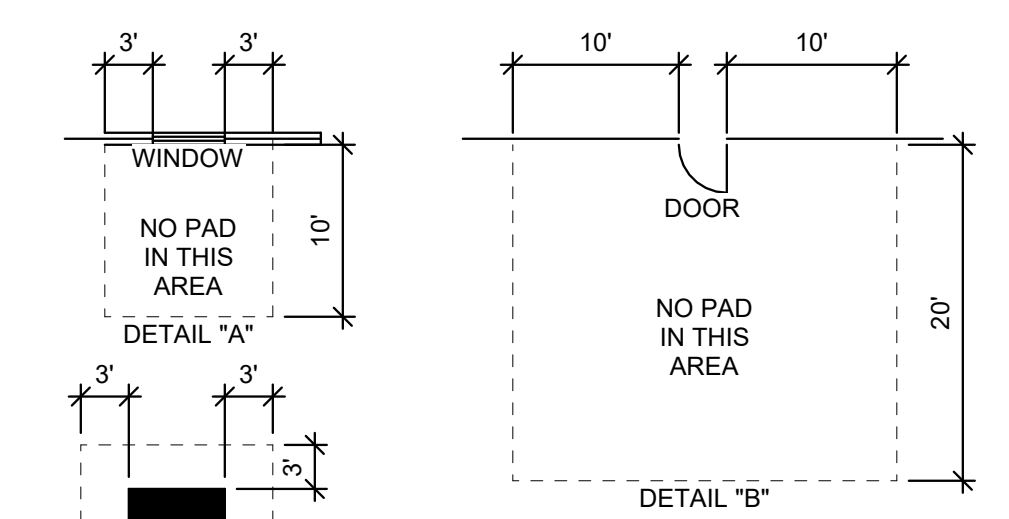
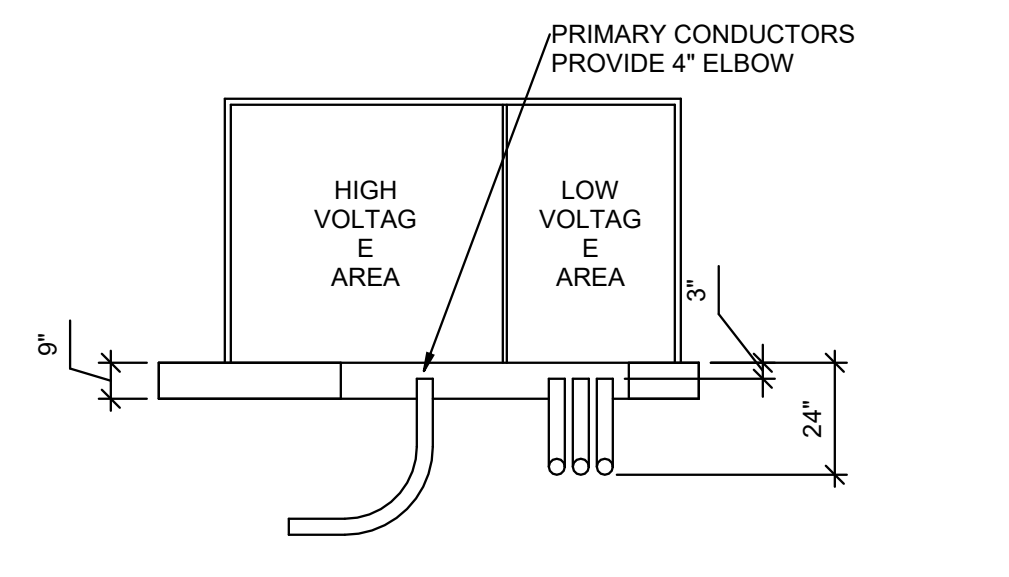
C

C1 TRANSFORMER CLEARANCE DETAIL

SCALE: NTS



- CONTRACTOR GENERAL NOTES:
- SITE PREPARATION: ALL SOIL BENEATH THE PAD SITE MUST BE COMPACTED AND LEVEL PRIOR TO SETTING OR POURING THE PAD TO PREVENT SETTLING.
 - CONCRETE: STEEL REINFORCEMENT SHALL BE #4 BARS. PLACED ACCORDING TO THE DRAWING. THE PAD MUST BE POURED AT LEAST SEVEN FULL DAYS PRIOR TO SETTING THE TRANSFORMER. THE FINISHED SURFACE MUST BE COMPLETELY FLAT AND LEVEL. SEE STANDARD 73 036 FOR CONCRETE SPECIFICATIONS.
 - PREFABRICATION: THE PAD MAY EITHER BE CONSTRUCTED ON THE SITE OR PREFABRICATED ACCORDING TO SPECIFICATIONS.
 - CONDUIT WINDOW LAYOUT: LOW VOLTAGE CONDUITS SHALL BE FORMED AS TIGHTLY AS POSSIBLE AGAINST THE RIGHT SIDE OF THE OPENING AND SHALL IN NO CASE EXTEND FURTHER THAN 20" FROM THE RIGHT SIDE OF THE CONDUIT WINDOW ON THE SMALL PAD OR 30" ON THE LARGE PAD. NO MORE THAN 8 CONDUITS WILL BE USED ON THE LOW VOLTAGE SIDE (NOT INCLUDING THE METERING CONDUIT). DO NOT PUT ANY CONCRETE IN OR UNDER THE CONDUIT WINDOW. USE SOIL TO SEPARATE CONDUITS. BELL ENDS ARE REQUIRED FOR ALL METAL CONDUIT, BUT NOT FOR PLASTIC CONDUIT.
 - CLEARANCES: THE FRONT OF THE PAD SHOULD ALWAYS FACE AWAY FROM ADJACENT STRUCTURES AND BE FREE OF OBSTRUCTIONS. AT LEAST 3 FEET MUST SEPARATE THE EDGES OF THE PAD FROM ANY ADJACENT STRUCTURE. THE EDGES OF THE PAD MUST BE AT LEAST 10 FEET FROM ANY COMBUSTIBLE STRUCTURE. IF AN ADJACENT STRUCTURE HAS ANY OVERHANG OR EAVE WITHIN 27 VERTICAL FEET OF THE TOP OF THE PAD, CLEARANCES MUST BE MEASURED FROM THE OUTSIDE OF THE OVERHANG. THE PAD MUST NOT BE PLACED IN AN AREA 10 FEET IN LINE WITH OR 3 FEET TO EITHER SIDE OF ANY WINDOW IN AN ADJACENT STRUCTURE (SEE DETAIL "A"). CLEARANCE FOR A DOOR MUST BE 20 FEET IN LINE WITH IT AND 10 FEET ON THE SIDES (SEE DETAIL "B"). PADS MUST NOT BE PLACED WITHIN 15 FEET OF ANY VALVE OR WITHIN 20 FEET OF ANY PLUMBING OR STORAGE FACILITY CONTAINING FLAMMABLE MATERIAL. NO WALLS, FENCES, OR ANY OTHER OBSTRUCTIONS WILL BE PLACED WITHIN 3 FEET OF THE SIDES OR BACK OF THE PAD, OR WITHIN 10 FEET OF THE FRONT OF THE PAD (SEE DETAIL "C"). THE AREA IN FRONT OF THE PAD MUST HAVE 10 FEET OF CLEAR, LEVEL WORKING AREA FOR MAINTENANCE OF THE TRANSFORMER. THE PAD MAY NOT BE PLACED IN LINE WITH AN AIR INTAKE WITHIN 32 VERTICAL FEET OF THE SURFACE PAD. ALSO VERTICALLY, IT MUST NOT BE PLACED WITHIN 12 FEET OF A DOOR OR WINDOW.
 - BARRIERS: IF THE TRANSFORMER PAD IS TO BE LOCATED IN AREAS SUBJECT TO VEHICULAR TRAFFIC, (PARKING LOTS, DRIVEWAYS, ETC) CONTACT UTAH POWER & LIGHT FOR PROTECTIVE BARRIER REQUIREMENTS.
 - IF THE TRANSFORMER WILL NOT COVER THE CABLE OPENINGS ON THESE STANDARD PADS, SEAL THE SIDES OF THE CABLE OPENING TO FIT THE TRANSFORMER USING SAKRETE OR COMPARABLE.



TRANSFORMER PAD DIMENSION CHART

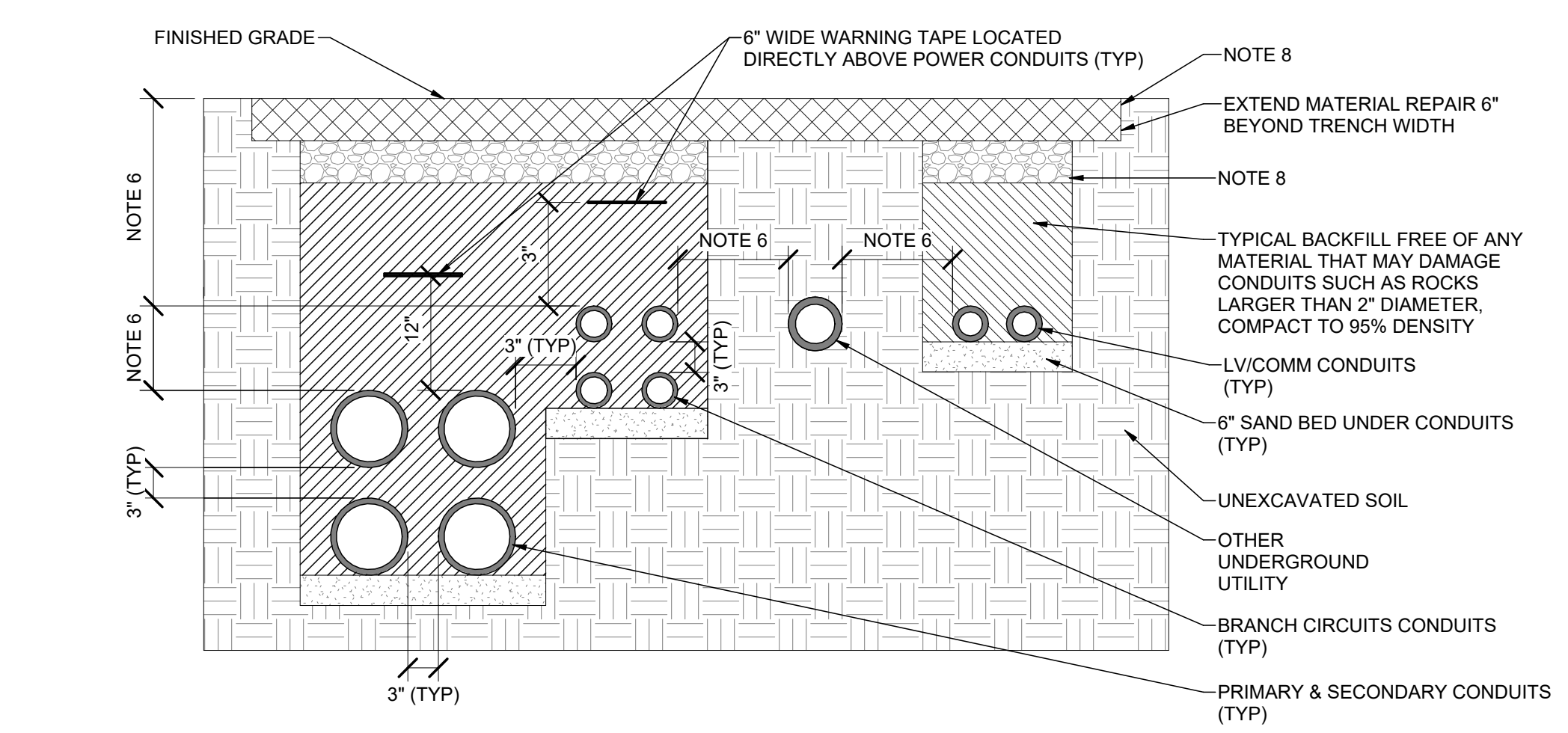
TRANSFORMER RATING	A	B	C	D	E
75-500 KVA	96"	78"	48"	15"	20"
750-2500 KVA	100"	105"	60"	16"	30"

A

A1 TRANSFORMER PAD DETAIL

SCALE: NTS

B



- NOTES:
- INSTALL CONDUITS PER LOCAL UTILITY REQUIREMENTS AND NEC.
 - ALL BENDS SHALL BE LARGE RADIUS.
 - PROVIDE WIDE SWEEP FIBERGLASS ELBOWS FOR UTILITY POWER CONDUITS.
 - ALL ABOVEGROUND CONDUIT IN AREAS WHERE DAMAGE MAY OCCUR, ALL STUBUPS AND THE FIRST 10' UNDERGROUND SHALL BE PVC WRAPPED RMC. ALL OTHER UNDERGROUND CONDUIT SHALL BE PVC SCH 40.
 - PROVIDE 200-LB POLYPROPYLENE PULL ROPE WITH MEASUREMENT MARKS IN EMPTY CONDUITS.
 - TYPICAL DEPTHS UNLESS NOTED OTHERWISE: PRIMARY AND SECONDARY POWER = 36" MIN DEPTH; BRANCH CIRCUITS & LV/COMM = 24" MIN DEPTH.
 - MAINTAIN UTILITY SEPARATION AS SHOWN BELOW UNLESS NOTED OTHERWISE. FOR ALL OTHER UTILITIES NOT SHOWN, MAINTAIN A MINIMUM 18" CLEARANCE. UTILIZE VERTICAL MEASUREMENT WHEN CROSSING UTILITIES.
 - REPAIR EXISTING ASPHALT OR SURFACE REFER TO CIVIL SPECIFICATIONS, WHERE SPECIFICATIONS DO NOT DEFINE CONTRACTOR SHALL REPAIR TO MATCH EXISTING CONDITIONS OR REQUIREMENTS BELOW, WHICHEVER IS GREATER.

- TURF/PLANTER/LANDSCAPE REPAIR
- TURF/PLANTER/LANDSCAPE
 - 12" TOPSOIL
- SANITARY SEWER & NON-WATER
- 60" HORIZONTALLY
 - 18" VERTICALLY
- WATERS
- 36" HORIZONTALLY
 - 18" VERTICALLY
- STORM DRAIN
- 60" HORIZONTALLY
 - 18" VERTICALLY
- LV/COMM CONDUITS (TYP)
- 36" HORIZONTALLY
 - 18" VERTICALLY
- 6" SAND BED UNDER CONDUITS (TYP)
- UNEXCAVATED SOIL
- 0-600V - 34" VERTICALLY
 - 600V - 30" VERTICALLY
 - 600V - 22,000V - 30" VERTICALLY
 - 22,000V - 40,000V - 36" VERTICALLY
- OTHER UNDERGROUND UTILITY
- TELECOM & LOW VOLTAGE
- 12" HORIZONTALLY
 - 12" VERTICALLY

- CONCRETE REPAIR
- 5" CEMENT CONCRETE
 - 4" UNTREATED BASE COURSE
 - COMPACT TO 95% MODIFIED PROCTOR
 - 8" PIT RUN GRAVE COMPACT TO 95% MODIFIED PROCTOR
 - 12" AGGREGATE BASE COURSE
 - UNDER ROADS
 - COMPACT TO 95% MODIFIED PROCTOR
- ASPHALT REPAIR
- 3" STANDARD ASPHALT
 - 4" UNTREATED BASE COURSE
 - COMPACT TO 95% MODIFIED PROCTOR
 - 8" PIT RUN GRAVE COMPACT TO 95% MODIFIED PROCTOR

A3 TYPICAL POWER AND TELECOM CONDUIT DIRECT BURY DETAIL

SCALE: 1/8" = 1'-0"

E

D

C

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SERVICE LOAD CALCULATIONS									
480Y/277 V SERVICE CALCULATION									
NEW CONSTRUCTION AREA FT ²		16,000							
ELECTRICAL SERVICE VOLTAGE (V)		480							
ELECTRICAL SERVICE PHASE (PH)		3							
NEC DEMAND									
DESCRIPTION	KW	VA/FP	HP	PF	KVA	FACTOR	LOAD (KVA)	LOAD (A)	NOTES
A. LIGHTING LOAD		1.5			24.0	125%	30	38	1
B. MOTOR & HVAC LOAD					99.5	100%	99	120	3
C. MISCELLANEOUS LOAD	0.5				3.0	100%	3	10	
D. LARGEST MOTOR			30		29.1	25%	7	9	4
E. WELDERS					904.6	100%	904	1087	
F. WELDING EQUIPMENT					537.0	100%	537	646	
G. EXTRACTION SYSTEM					166.3	100%	166	200	
H. 208/120V LOAD					107.2	100%	107	129	
TOTAL							1859	2230	
BUILDING SERVICE SIZE (A)							2500		
SERVICE SPARE CAPACITY (%)								11%	

208Y/120 V SERVICE CALCULATION									
NEW CONSTRUCTION AREA FT ²		16,000							
ELECTRICAL SERVICE VOLTAGE (V)		208							
ELECTRICAL SERVICE PHASE (PH)		3							
NEC DEMAND									
DESCRIPTION	KW	VA/FP	HP	PF	KVA	FACTOR	LOAD (KVA)	LOAD (A)	NOTES
A. RECEPTACLE LOAD FIRST 10 KVA					10.0	100%	10	28	2
B. RECEPTACLE LOAD REMAINING >10 KVA					32.0	50%	16	44	2
C. MOTOR & HVAC LOAD					29.7	100%	30	82	3
D. SPECIFIC APPLIANCES OR LOADS					18.0	100%	18	44	
E. WELDING EQUIPMENT					19.6	100%	20	54	
F. MISCELLANEOUS LOAD					16.0	100%	16	44	
TOTAL							107	288	
BUILDING SERVICE SIZE (A)							400		
SERVICE SPARE CAPACITY (%)								26%	

NOTES:
 1. LIGHTING LOADS BASED ON NEC TABLE 220.12 VALUE FOR SCHOOLS OR ACTUAL LOAD WHICHEVER IS GREATER
 2. LOAD BASED ON ACTUAL CALCULATED LOAD OR 1 VA/S.F. WHICHEVER IS GREATER. RECEPTACLE DEMAND FACTOR BASED ON NEC TABLE 220.44.
 3. MOTOR LOAD CALCULATED AS PER NEC 220.50, 430.24, 430.25, 430.26, AND 440.6.
 4. LARGEST MOTOR LOAD AS PER NEC 220.50, 430.24.

EQUIPMENT NAMEPLATE SCHEDULE			
EQUIPMENT ID SCHEME	FIRST DIGIT - BUILDING LEVEL (0, 1, 2, ETC) SECOND DIGIT - PANEL TYPE L - (120/208) H - (277/480) THIRD DIGIT - M - MECHANICAL L - LIGHTING W - WELDING EQUIP G - GENERAL POWER FOURTH DIGIT - SEQUENCE # (1,2,3...) OR DISTRIBUTION		
LABEL FORMAT	[NAME] [SYSTEM] [VOLTAGE] [FED FROM] [SOURCE(S)]		
LABEL EXAMPLE	PANEL "4L1" STANDBY POWER 120/208V FED FROM BUS-A / XFMR-4TA		
OTHER	EXCEPTIONS: MDP - MAIN DISTRIBUTION BOARD		
COLOR SCHEME			
SYSTEM	EQUIPMENT	NAMEPLATE COLOR	BACKGROUND
NORMAL POWER	ALL GEAR NOT INCLUDED BELOW	WHITE	BLACK

BRANCH CIRCUIT CONDUCTOR AND CONDUIT SIZING TABLE			
CIRCUIT AMPACITY/VOLTAGE	CIRCUIT LENGTH	CONDUCTOR SIZE (PHASE, NEUTRAL AND GR)	CONDUIT SIZE
20A/120V	0' - 60'	#12 AWG	0.75" Ø
20A/120V	60' - 95'	#10 AWG	1" Ø
20A/120V	95' - 150'	#8 AWG	1.125" Ø
20A/120V	150' - 240'	#6 AWG	1.25" Ø
20A/277V	0' - 140'	#12 AWG	0.75" Ø
20A/277V	140' - 220'	#10 AWG	0.75" Ø
20A/277V	220' - 350'	#8 AWG	1" Ø
20A/277V	350' - 550'	#6 AWG	1.25" Ø

NOTES:
 1. WIRE SIZING IS BASED ON COPPER CONDUCTORS SUPPLYING A 20A, 120V CIRCUIT AT THE INDICATED VOLTAGE, ASSUMED TO BE 80% LOADED (16A), WITH MAXIMUM VOLTAGE DROP OF 3% AT THE LOAD.
 2. DOWN-SIZED WIRE AT DEVICE LOAD AS REQUIRED AND TERMINATE CONDUCTORS IN A SAFE AND CODE COMPLIANT MANNER.
 3. CONDUIT SIZE IS BASED ON A MAXIMUM OF 3 CIRCUITS PER CONDUIT, EACH WITH A SEPARATE NEUTRAL CONDUCTOR.

ALUMINUM CONDUCTOR AND CONDUIT SCHEDULE								
SCHEDULE NUMBER								
SUBSCRIPT (NOTE 5)								
SYM	AMP	CONDUIT SIZE	CONDUCTOR (NOTE 1)	IG	SE	NOTES		
130	2	3	2/0	4	1/0	4	2,7	
130	2	4	2/0	4	1/0	4	2,7	
150	2	3	3/0	4	1/0	4	2,7	
150	2	4	3/0	4	1/0	4	2,7	
175	2	3	4/0	4	1/0	2	2,7	
175	2	4	4/0	4	1/0	2	2,7	
200	2	3	250	4	1/0	2	2,7	
200	2	4	250	4	1/0	2	2,7	
230	2	3	300	2	1/0	1/0	2,7	
230	2	4	300	2	1/0	1/0	2,7	
250	3	3	350	2	2/0	1/0	2,7	
250	3	4	350	2	2/0	1/0	2,7	
310	3	3	500	1	3/0	1/0	2,7	
310	3	4	500	1	3/0	1/0	2,7	
380	2	EA 2.50	3	250	1	4/0	3/0	2,7
380	2	EA 3	4	250	1	4/0	3/0	2,7
400	2	EA 2.50	3	250	1/0	4/0	3/0	2,7
400	2	EA 2.50	4	250	1/0	4/0	3/0	2,7
500	2	EA 3	3	350	1/0	300	3/0	2,4,7
500	2	EA 3	4	350	1/0	300	3/0	2,4,7
620	2	EA 3	3	500	3/0	300	3/0	2,4,7
620	2	EA 4	4	500	3/0	300	3/0	2,4,7
750	3	EA 3	3	350	3/0	300	4/0	2,4,7
750	3	EA 3	4	350	3/0	300	4/0	2,4,7
810	3	EA 3	3	400	4/0	300	250	2,4,7
810	3	EA 4	4	400	4/0	300	250	2,4,7
1000	4	EA 3	3	350	4/0	300	250	4,7
1000	4	EA 3	4	350	4/0	300	250	4,7
1140	4	EA 4	4	500	250	300	250	4,7
1240	4	EA 4	3	500	350	300	250	4,7
1240	4	EA 4	4	500	350	300	250	4,7
1620	6	EA 4	4	400	400	350	250	4,7
2170	7	EA 4	4	500	400	400	500	4,7
2695	7	EA 4	4	750	600	750	750	4,7
3080	8	EA 4	4	750	600	750	750	4,7
4235	11	EA 4	4	750	800	750	750	4,7
1200	5	EA 4	-	-	-	-	-	6
3000	10	EA 6	-	-	-	-	-	6
-	-	10	EA 4	-	-	-	-	6

GENERAL SHEET NOTES							
1	PROVIDE NEMA 3R ENCLOSURES FOR EQUIPMENT LOCATED OUTDOORS. REFER TO PLANS FOR EQUIPMENT LOCATIONS.						
2	REFER TO PLANS FOR CONSTRAINTS ON PHYSICAL DIMENSIONS AND CLEARANCE REQUIREMENTS OF EQUIPMENT. PROVIDE EQUIPMENT DIMENSIONS THAT FALL WITHIN THE CONSTRAINTS OF EACH SPECIFIC LOCATION.						
3	ALL EQUIPMENT SHALL BE CONSTRUCTED AND BRACED FOR THE SEISMIC CONDITIONS OF THE PROJECT. REFER TO ELECTRICAL SPECIFICATIONS FOR REQUIREMENTS.						
4	PROVIDE PERFORMANCE TESTING FOR GROUND-FAULT PROTECTION SYSTEMS ON SITE WITH A WRITTEN RECORD OF THIS TEST SUBMITTED TO THE AUTHORITY HAVING JURISDICTION PER NEC 230.95(C).						
SHEET KEYNOTES							
1	CONTRACTOR SHALL PROVIDE 4" CONDUIT WITH PULL STRING. UTILITY TO INSTALL CABLING. CONTRACTOR SHALL PROVIDE WIDE SWEEP FIBERGLASS ELBOWS.						
2	CONTRACTOR TO INSTALL TRANSFORMER PAD. UTILITY TO PROVIDE TRANSFORMER.						
3	PROVIDE (2) 3" CONDUITS AND 1" CONDUIT TO OUTSIDE OF SWITCHBOARD PAD FOR FUTURE POWER FACTOR CORRECTING CAPACITORS.						
4	KCP WILL FURNISH METER BASE WITH CABLING. CONTRACTOR TO INSTALL THE METER BASE AND CONDUITS. KCP TO CONNECT AND TERMINATE ALL WIRING.						
5	EQUIPMENT ASSOCIATED WITH BID PACKAGE 1.						

COPPER CONDUCTOR AND CONDUIT SCHEDULE										
SCHEDULE NUMBER										
SUBSCRIPT (NOTE 5)										
SYM	AMP	HH	CONDUIT SIZE	CONDUCTOR (NOTE 1)	IG/HH	SE	NOTES			
1	20	-	75	2	12	12	8	2		
2	20	-	75	3	12	12	8	2,3		
3	20	24	75	4	12	12	8	2,3		
4	30	-	75	2	10	10	8	2		
5	30	-	75	3	10	10	8	2		
6	30	32	75	4	10	10	8	2		
7	40	-	1	2	8	10	8	6	2	
8	40	-	1	3	8	10	8	6	2	
9	40	44	1	4	8	10	8	6	2	
10	55	-	1	2	6	10	8	4	2	
11	55	-	1	3	6	10	8	4	2	
12	55	60	1.25	4	6	10	8	4	2	
13	70	-	1	2	4	8	4	2	2	
14	70	-	1.25	3	4	8	4	2	2	
15	70	76	1.25	4	4	8	4	2	2	
16	85	-	1.25	2	3	8	3	2	2	
17	85	-	1.25	3	3	8	3	2	2	
18	85	82	1.25	4	3	8	3	2	2	
19	95	-	1.25	3	2	8	2	2	2	
20	95	104	1.50	4	2	8	2	2	2	
21	130	-	1.50	3	1	6	2	2	2	
22	130	116	1.50	4	1	6	2	2	2	
23	150	-	2	3	1/0	6	2	10	2	
24	150	136	2	4	1/0	6	2	10	2	
25	175	-	2	3	2/0	6	2	20	2	
26	175	156	2	4	2/0	6	2	20	2	
27	200	-	2	3	3/0	6	2	20	2	
28	200	180	2.50	4	3/0	6	2	20	2	
29	230	-	2.50	3	4/0	4	2	20	2	
30	230	208	2.50	4	4/0	4	2	20	2	
31	255	-	2.50	3	250	4	1	20	2	
32	255	232	2.50	4	250	4	1	20	2	
33	310	-	3	3	350	3	1/0	30	2	
34	310	280	3	4	350	3	1/0	30	2	
35	380	-	3.50	3	500	3	3/0	30	2	
36	380	344	4	4	500	3	3/0	30	2	
37	400	-	2	EA 2	3	3/0	3	30	2	
38	400	360	2	EA 2.50	4	3/0	3	30	2	
39	510	-	2	EA 2.50	3	250	1	4/0	30	2
40	510	464	2	EA 3	4	250	1	4/0	30	2
41	620	-	2	EA 3	3	350	1/0	4/0	30	2,4
42	620	560	2	EA 3	4	350	1/0	4/0	30	2,4
43	760	-	2	EA 3.50	3	500	1/0	4/0	30	2,4
44	760	688	2	EA 4	4	500	1/0	4/0	30	2,4
45	855	-	3	EA 3	3	300	2/0	4/0	30	2,4
46	855	788	3	EA 3	4	300	2/0	4/0	30	2,4
47	1000	-	3	EA 3.50	4	2/0	4/0	30	4	
48	1000	912	3	EA 3.50	4	400	2/0	4/0	30	4
49	1140	-	3	EA 4	3	500	3/0	4/0	30	4
50	1140	1032	3	EA 4	4	500	3/0	4/0	30	4
51	1240	-	4	EA 3	3	350	3/0	4/0	30	4
52	1240	1120	4	EA 3	4	350	3/0	4/0	30	4
53	1675	1520	5	EA 4	4	400	4/0	4/0	4	
54	2010	1824	6	EA 4	4	400	250	250	4	
55	2680	2408	7	EA 4	4	500	350	350	4	
56	3040	2752	8	EA 4	4	500	500	500	4	
57	4180	3784	11	EA 4	4	500	500	500	4	
58	1200	-	5	EA 4	-	-	-	-	6	
59	3000	-	10	EA 6	-	-	-	-	6	
60	-	-	10	EA 4	-	-	-	-	6	

CONDUCTOR AND CONDUIT SCHEDULE NOTES
 1. CONDUCTORS SHOWN ARE SHOWN FOR EACH CONDUIT WITH MODIFICATIONS AS NOTED IN NOTE 5. ALL CONDUCTORS SHOWN ARE THWN UNLESS OTHERWISE NOTED.
 2. PROVIDE EQUIPMENT GROUND CONDUCTORS PER TABLE 250-122 WHEN CIRCUIT BREAKERS ARE SIZED GREATER THAN AMPERE RATING SHOWN IN TABLE.
 3. PROVIDE #10 NEUTRALS FOR MULTIWIRE BRANCH CIRCUITS SERVING COMPUTERS.
 4. GROUND (G) CONDUCTOR MAY BE DELETED ON SERVICE ENTRANCE CONDUCTORS.
 5. SYMBOL SUBSCRIPTS:
 "2N": INCLUDE TWO NEUTRAL CONDUCTORS SIZED AS SCHEDULED FOR PHASE AND NEUTRAL CONDUCTORS WHERE THE CONDUCTOR IS #1/0 OR LARGER. INCLUDE A SINGLE 200% RATED CONDUCTOR THAT IS TWICE THE AMPACITY OF THE SCHEDULED PHASE AND NEUTRAL CONDUCTOR WHERE THE CONDUCTOR IS BELOW #1/0 IN SIZE.
 "CI": PROVIDE CIRCUIT INTEGRITY CABLE. TYPE TWO-HOUR FIRE RESISTIVE CABLES IN CONDUIT OR PROVIDE FEEDER ENCASED IN CONCRETE.
 "FG": FULL SIZE GROUND. SIZE EQUIPMENT GROUNDING CONDUCTOR TO BE SAME SIZE AS THE PHASE CONDUCTORS.
 "HH": NEUTRAL CURRENTS EXIST DUE TO HIGH HARMONIC "NONLINEAR" LOADS. CURRENT CARRYING CONDUCTORS DERATED ACCORDINGLY. PROVIDE THE IGH/H SIZE FOR THE EQUIPMENT GROUNDING CONDUCTOR.
 "IG": INCLUDE IG (INSULATED/ISOLATED GROUND CONDUCTOR) SCHEDULED ALONG WITH THE GROUND OF EQUIPMENT GROUND CONDUCTOR.
 "MC": PROVIDE FEEDER IN METAL-CLAD CABLE. TYPE MC IN PLACE OF SINGLE CONDUCTORS IN CONDUIT.
 "SE": SUBSTITUTE "SE" CONDUCTOR FOR "G" CONDUCTOR SHOWN, WHICH IS SIZED FOR THE GROUNDING OF THE SECONDARY OF THE SEPARATELY DERIVED SYSTEM.
 "SER": PROVIDE SERVICE-ENTRANCE CABLE. TYPE SE OR SER IN PLACE OF SINGLE CONDUCTORS IN CONDUIT.
 6. RACEWAY ONLY. CONDUCTORS PROVIDED BY UTILITY.
 7. ALUMINUM CONDUCTORS NOT TO BE USED FOR CONNECTION TO MOTORS OR MOTOR DRIVEN EQUIPMENT.

CONDUCTOR AND CONDUIT SCHEDULE NOTES
 1. CONDUCTORS SHOWN ARE SHOWN FOR EACH CONDUIT WITH MODIFICATIONS AS NOTED IN NOTE 5. ALL CONDUCTORS SHOWN ARE THWN UNLESS OTHERWISE NOTED.
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